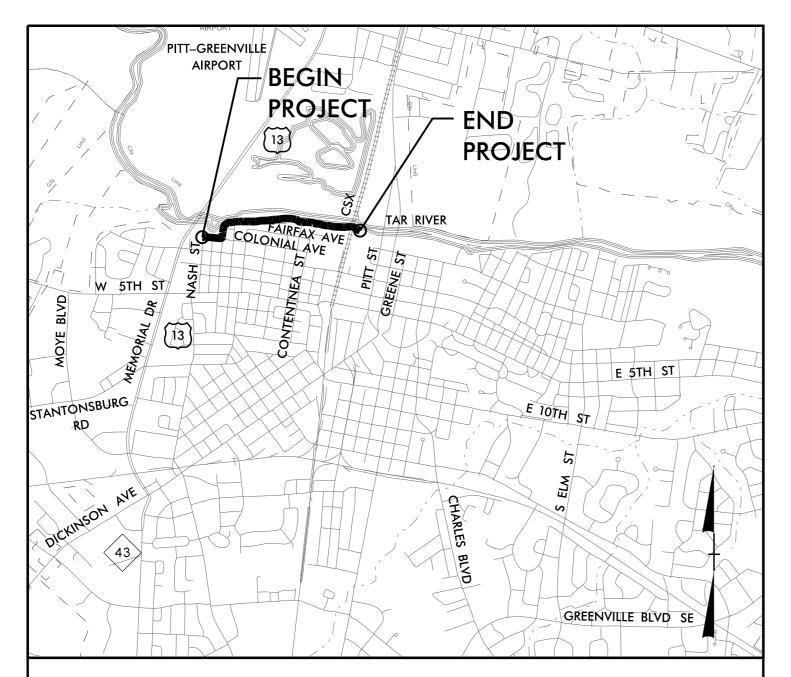
PROJECT REFERENCE NO. SHEET NO. EB-5539

GRAPHIC SCALE

PLANS

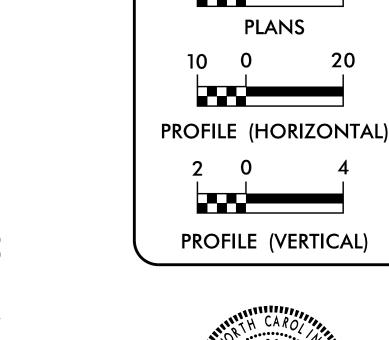


CITY OF GREENVILLE

SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

PHASE 3A - FROM NASH STREET TO THE WESTERN TERMINUS OF THE EXISTING SOUTH TAR RIVER GREENWAY NEAR PITT STREET PHASE 3B – FROM MOYE BOULEVARD

TO NASH STREET (FUTURE CONSTRUCTION)





-L2- STA 52+54.52 VICINITY MAP -L3-STA 98+75.00=END PEDESTRIAN BRIDGE -L4- STA 108+00.00 / **_L2**_ STA 53 + 23.58 -L1- STA 40+00.00=END PROJECT EB-5539, PHASE 3A -L2- STA 50+00.00 -L4- STA 126+16.33 -L2-STA 66+00.66=-L3- STA 76+00.00 TAR RIVER FAIRFAX TRAILHEAD W. CONLEY ST S FAIRFAX AVE S PROJECT EB-5539, PHASE 3B -L1- STA 10+08.86 OWNER: CITY OF GREENVILLE 1500 BEATTY STREET GREENVILLE, NC 27834 LYNN RAYNOR, P.E. 252.329.4467 lraynor@greenvillenc.gov ALL ROADS ARE CITY OF GREENVILLE **ENGINEER**: STREETS UNLESS OTHERWISE NOTED KIMLEY-HORN AND ASSOCIATES, INC. PHASE 3B PHASE 3B | PHASE 3A 3001 WESTON PARKWAY CARY, NC 27513 **FUTURE CONSTRUCTION** JEFFREY W. MOORE, P.E. (FUTURE 919.677.2000

PHASE)

BEGIN PEDESTRIAN BRIDGE

GEOTECHNICAL:

fmundy@stewart-eng.com

jeff.moore@kimley-horn.com

421 FAYETTEVILLE STREET

SURVEYOR:

STEWART

SUITE 400 RALEIGH, NC 27601 FRANK MUNDY, PLS 919.380.8750

FALCON ENGINEERING, INC. 1210 TRINITY ROAD SUITE 110 RALEIGH, NC 27607 CHRISTOPHER V. NORVILLE, P.E 919.871.0800 cnorville@falconengineering.com

> ANN E. WALL CITY MANAGER



ALL REFERENCES TO WORK OUTSIDE OF

PHASE 3A (-L2- Sta. 64+68.00 TO -L4- 126+16.33) ARE NOT APPLICABLE TO THIS PROJECT.



BEIGN PROJECT EB-5539, PHASE 3A

-L2-STA 64+68.00

Find yourself in good company

INDEX OF SHEETS				
SHEET NO. DESCRIPTION				
1	TITLE SHEET			
2	CONVENTIONAL SYMBOLS			
C-1 TO C-16	MULTI-USE PATH AND DRAINAGE PLANS			
CD-1 TO CD-6	PROJECT NOTES AND CONSTRUCTION DETAILS			
L-1 TO L-3	FAIRFAX AVENUE TRAILHEAD PLAN AND DETAILS			
EC-1A TO EC-22	GRADING AND EROSION CONTROL PLANS			
S-1 TO S-24B	STRUCTURE PLANS AND DETAILS			
X-1 TO X-23	MULTI-USE PATH AND ROADWAY CROSS SECTIONS			

ALLEN THOMAS MAYOR

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO EB-5539 2

*S.U.E. = Subsurface Utility Engineering

BOUNDARIES AND PROPERTY:

CONVENTIONAL PLAN SHEET SYMBOLS

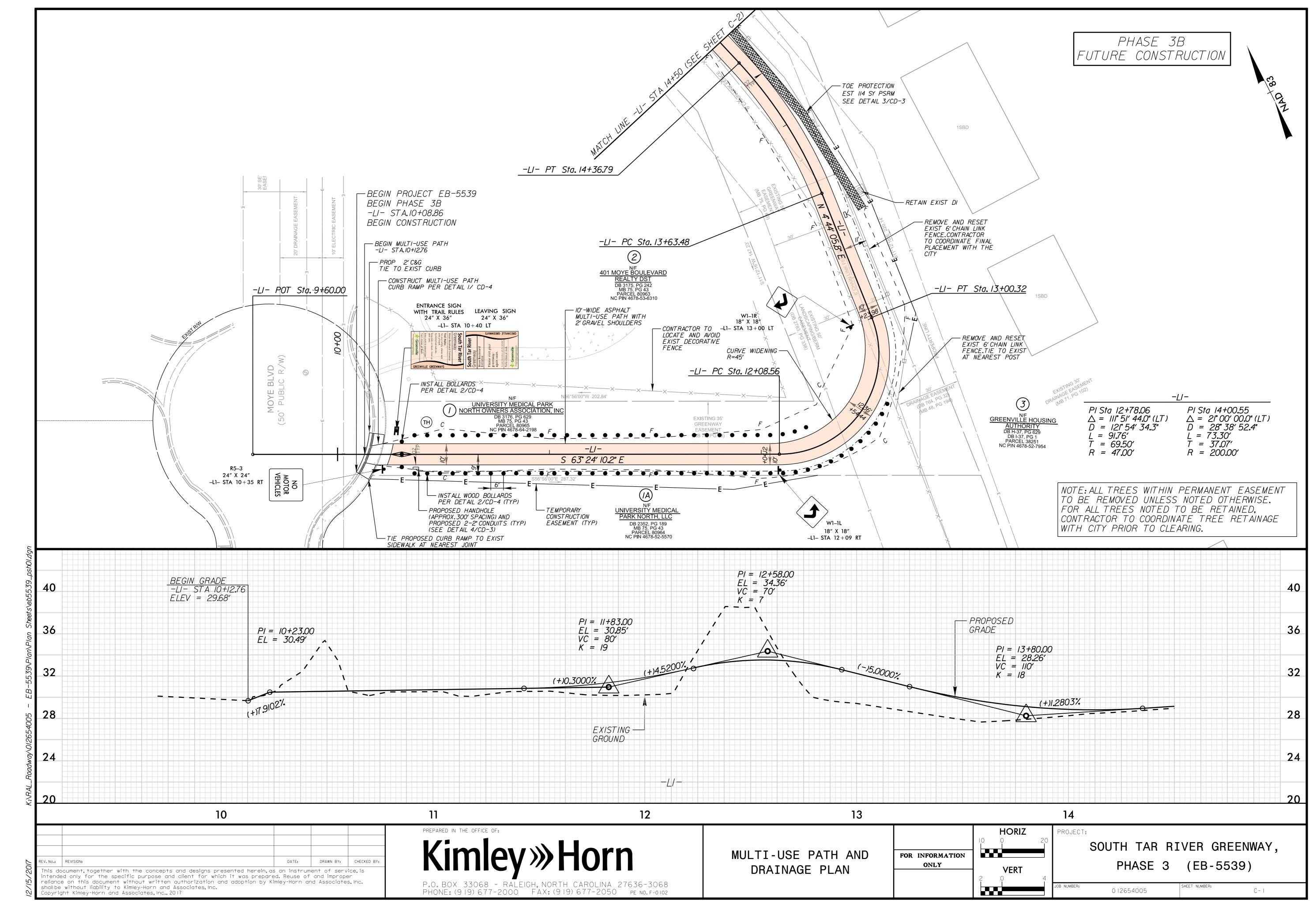
State Line ————————————————————————————————————			
County Line —		RAILROADS:	
Township Line ————————————————————————————————————		Standard Gauge ————	CSX TRANSPORTATION
City Line		RR Signal Milepost —————	⊙ MILEPOST 35
Reservation Line		Switch ————	SWITCH
Property Line —		RR Abandoned ————	
Existing Iron Pin	<u></u>	RR Dismantled	
Property Corner	×	RIGHT OF WAY:	
Property Monument	 ECM	Baseline Control Point	•
Parcel/Sequence Number ————————————————————————————————————	— (23)	Existing Right of Way Marker ————	\triangle
Existing Fence Line	xxx	Existing Right of Way Line	
Proposed Woven Wire Fence		Proposed Right of Way Line ———	$\frac{R}{W}$
Proposed Chain Link Fence		Proposed Right of Way Line with	
Proposed Barbed Wire Fence		Iron Pin and Cap Marker	W
Existing Wetland Boundary		Proposed Right of Way Line with Concrete or Granite R/W Marker	
Proposed Wetland Boundary ————	wlb	Proposed Control of Access Line with	
Existing Endangered Animal Boundary ———	EAB	Concrete C/A Marker	
Existing Endangered Plant Boundary ———		Existing Control of Access	(C)
Known Soil Contamination: Area or Site ——		Proposed Control of Access ————	
Potential Soil Contamination: Area or Site —		Existing Easement Line ————	—— E ———
BUILDINGS AND OTHER CULT		Proposed Temporary Construction Easement –	——Е——
Gas Pump Vent or U/G Tank Cap	- o	Proposed Temporary Drainage Easement ——	TDE
Sign ————————————————————————————————————	_	Proposed Permanent Drainage Easement ——	PDE
Well ————		Proposed Permanent Drainage / Utility Easement	DUE
Small Mine ————————————————————————————————————		Proposed Permanent Utility Easement ———	PUE
Foundation —		Proposed Temporary Utility Easement ———	TUE
Area Outline ————————————————————————————————————	_	Proposed Aerial Utility Easement ————	AUE
		Proposed Permanent Easement with	•
Cemetery	_	Iron Pin and Cap Marker	♦
Building ————————————————————————————————————		ROADS AND RELATED FEATURE	'S:
		Existing Edge of Pavement	
Church —		Existing Curb	
Dam —		Proposed Slope Stakes Cut	<u>C</u>
HYDROLOGY:		Proposed Slope Stakes Fill ————	<u>F</u>
Stream or Body of Water ——————		Proposed Curb Ramp	CR
Hydro, Pool or Reservoir ——————		Existing Metal Guardrail ————	
Jurisdictional Stream		Proposed Guardrail ————	<u> </u>
Buffer Zone 1	BZ 1 ———	Existing Cable Guiderail	
Buffer Zone 2 ———————————————————————————————————	BZ 2 ———	Proposed Cable Guiderail	
Flow Arrow ———————————————————————————————————		Equality Symbol	lacktriangle
Disappearing Stream ————————————————————————————————————		Pavement Removal ————	
Spring ————		VEGETATION:	
Wetland ——————		Single Tree	슌
Proposed Lateral, Tail, Head Ditch ————	< FLOW	Single Shrub	¢
False Sump ——————	$\qquad \qquad \bigoplus$	Hedge ————	

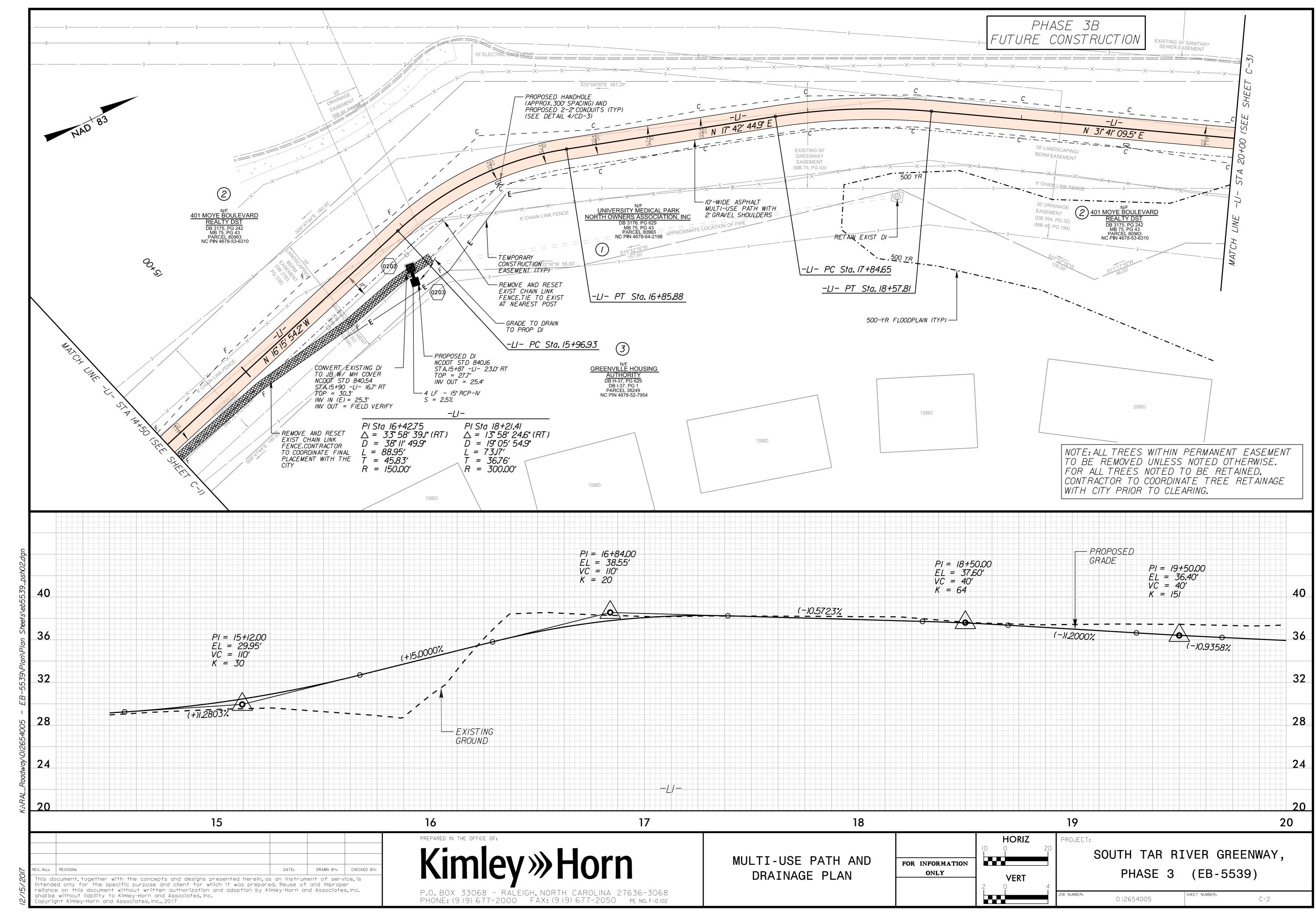
Orchard —	상 상 상 상
Vineyard ————	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall –) CONC WW (
MINOR: Head and End Wall	CONC HW
Pipe Culvert	
Footbridge >	
Drainage Box: Catch Basin, DI or JB ———————————————————————————————————	СВ
Tavea Blich Collei	_
Storm Sewer Manhole ————————————————————————————————————	(S)
Storm Sewer —	
UTILITIES:	
POWER:	
Existing Power Pole ———	•
Proposed Power Pole —	φ
Existing Joint Use Pole	-
Proposed Joint Use Pole	- 6-
Power Manhole ————	P
Power Line Tower ————	\boxtimes
Power Transformer ————	M
U/G Power Cable Hand Hole	
H_Frame Pole —————	•—•
Recorded U/G Power Line ————	P
Designated U/G Power Line (S.U.E.*)	P
TELEPHONE:	
Existing Telephone Pole	-
Proposed Telephone Pole	- 0-
Telephone Manhole	(1)
Telephone Booth	3
Telephone Pedestal ————————————————————————————————————	□
Telephone Cell Tower ————————————————————————————————————	∨• У
U/G Telephone Cable Hand Hole ————————————————————————————————————	_
Designated U/G Telephone Cable (S.U.E.*) — Recorded U/G Telephone Conduit ———	
Designated U/G Telephone Conduit (S.U.E.*)	
Recorded U/G Fiber Optics Cable ———	
Designated U/G Fiber Optics Cable (S.U.E.*)	ro .

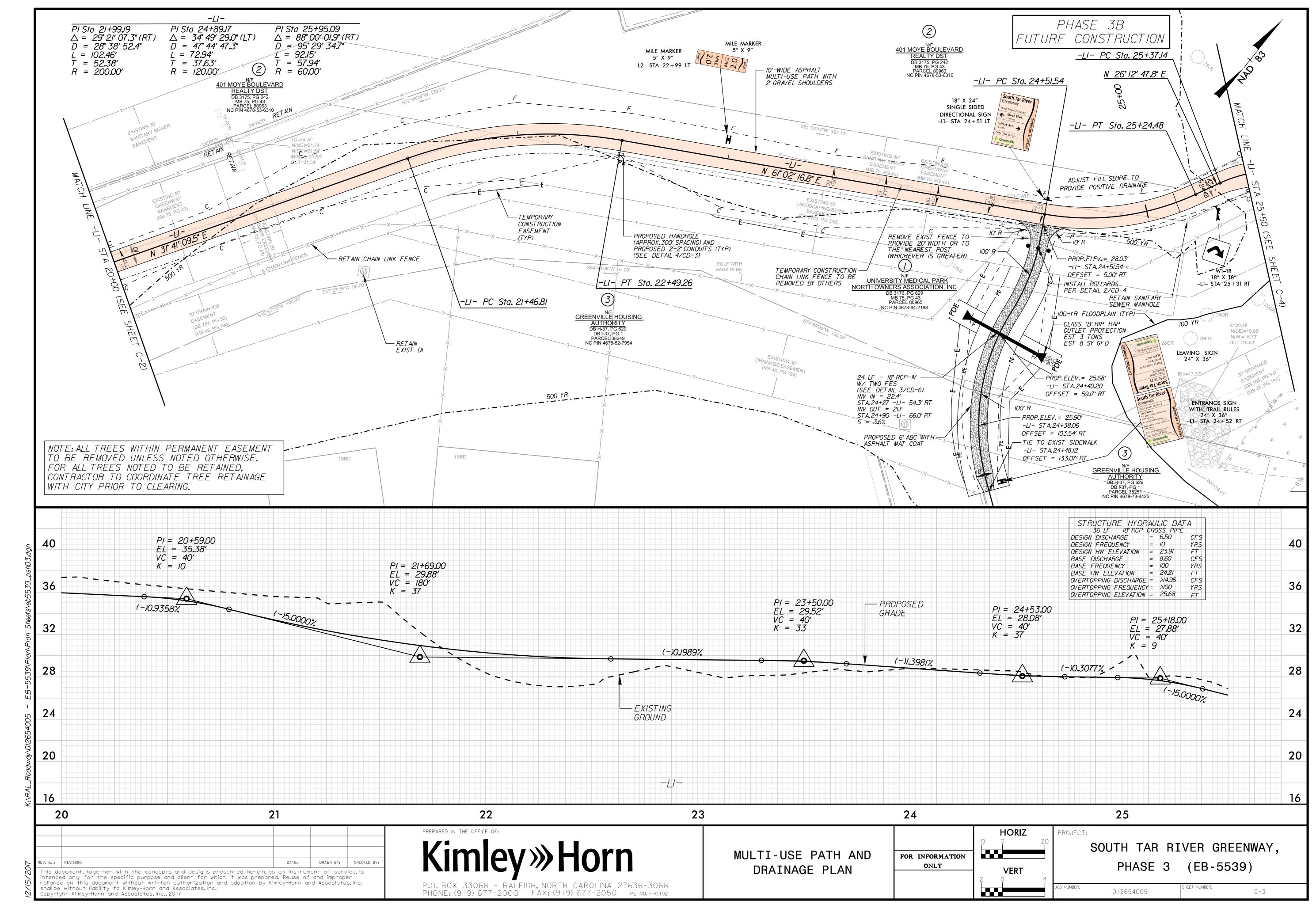
WATER:	
Water Manhole	W
Water Meter —	0
Water Valve —	\otimes
Water Hydrant —	•\$
Recorded U/G Water Line ————	
Designated U/G Water Line (S.U.E.*)	
Above Ground Water Line	
TV:	
TV Satellite Dish —	K
TV Pedestal —	
TV Tower —	\otimes
U/G TV Cable Hand Hole ————	
Recorded U/G TV Cable —	
Designated U/G TV Cable (S.U.E.*)	
Recorded U/G Fiber Optic Cable ————————————————————————————————————	
Designated U/G Fiber Optic Cable (S.U.E.*)—	- — — TV FO— —
CAS.	
GAS:	^
Gas Valve	♦
Gas Meter ———————————————————————————————————	·
Recorded U/G Gas Line —————	
Designated U/G Gas Line (S.U.E.*)———	
Above Ground Gas Line	A/G Gds
SANITARY SEWER:	
Sanitary Sewer Manhole	(
Sanitary Sewer Cleanout ——————	-
U/G Sanitary Sewer Line ————	
Above Ground Sanitary Sewer ————	
Recorded SS Forced Main Line————	FSS
Designated SS Forced Main Line (S.U.E.*) —	— — — FSS —
MISCELLANEOUS:	
Utility Pole ——————	•
Utility Pole with Base —————	$\overline{\cdot}$
Utility Located Object —————	\odot
Utility Traffic Signal Box —	S
Utility Unknown U/G Line	
U/G Tank; Water, Gas, Oil —————	
Underground Storage Tank, Approx. Loc. ——	(UST)
A/G Tank; Water, Gas, Oil	
Geoenvironmental Boring	₩
U/G Test Hole (S.U.E.*)	•
Abandoned According to Utility Records —	•
End of Information ————————————————————————————————————	E.O.I.
	L.O.I.

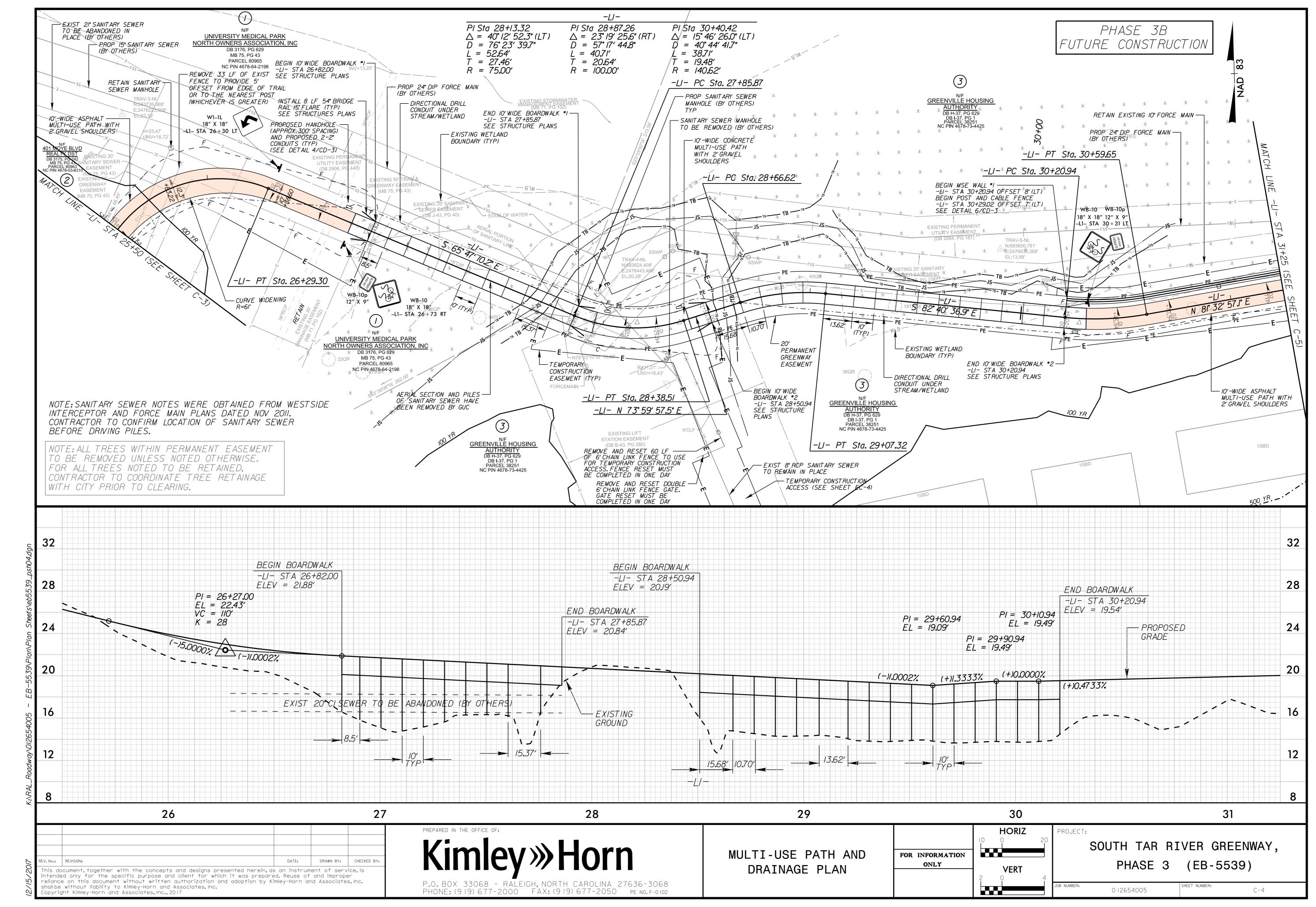
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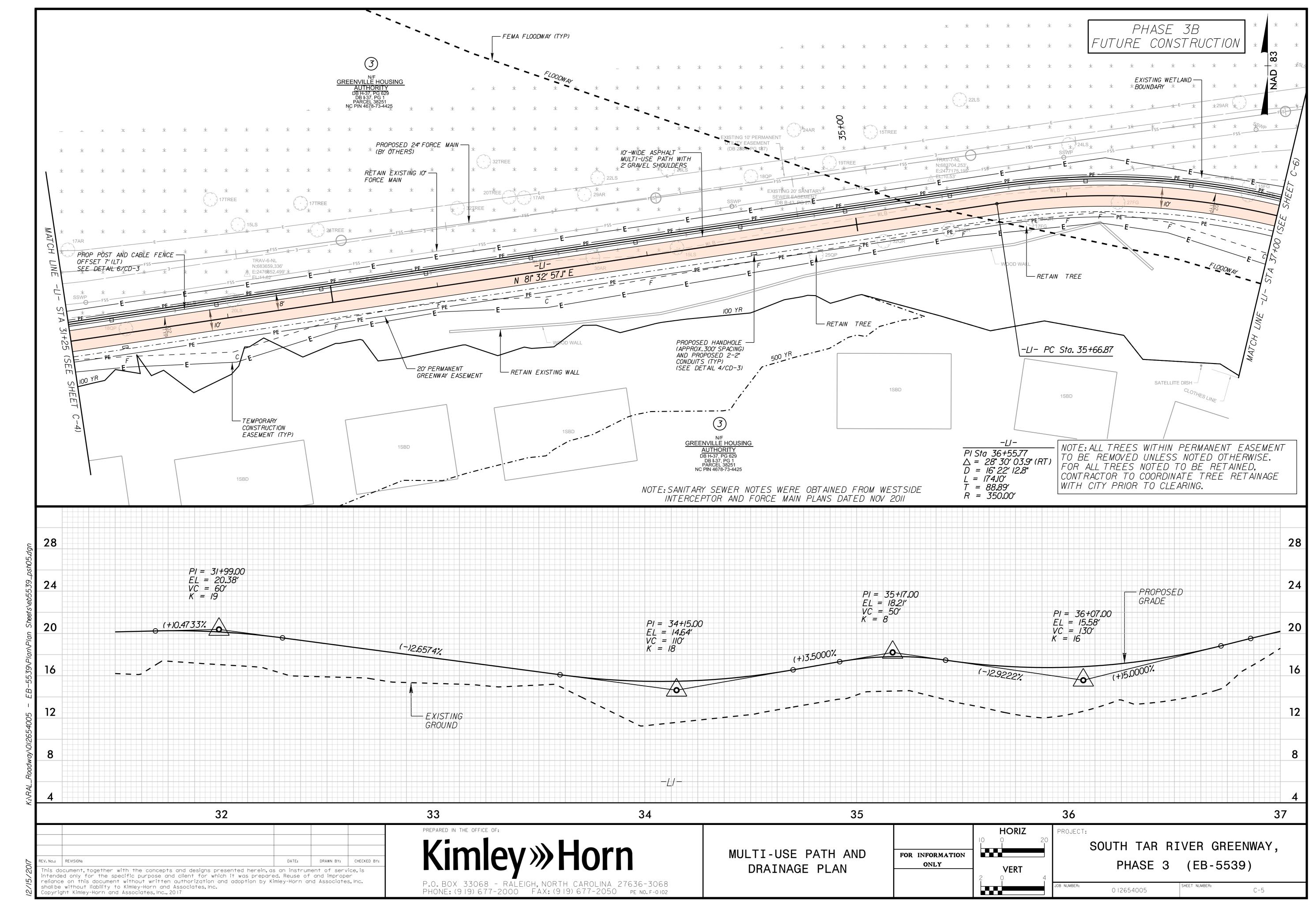
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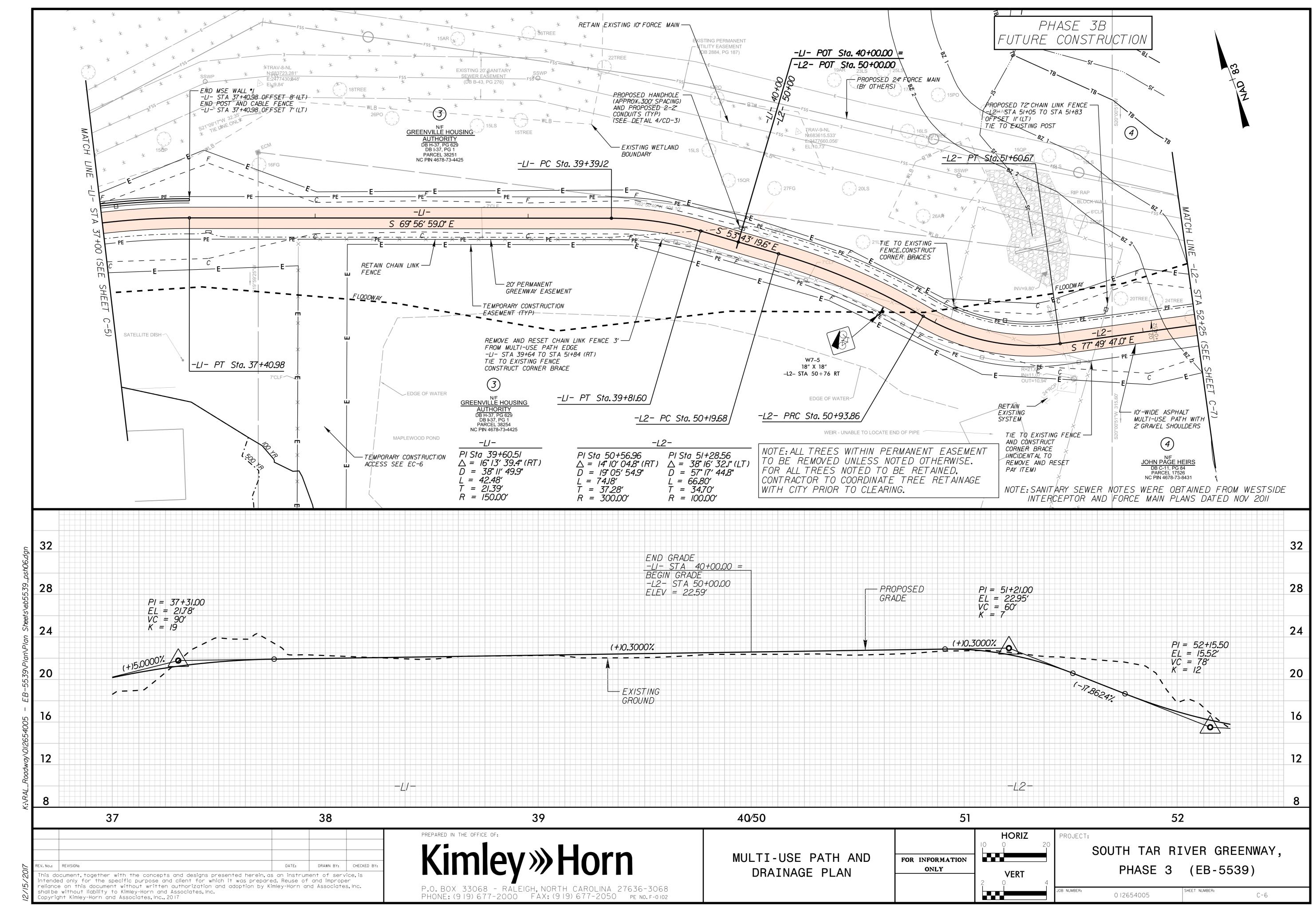


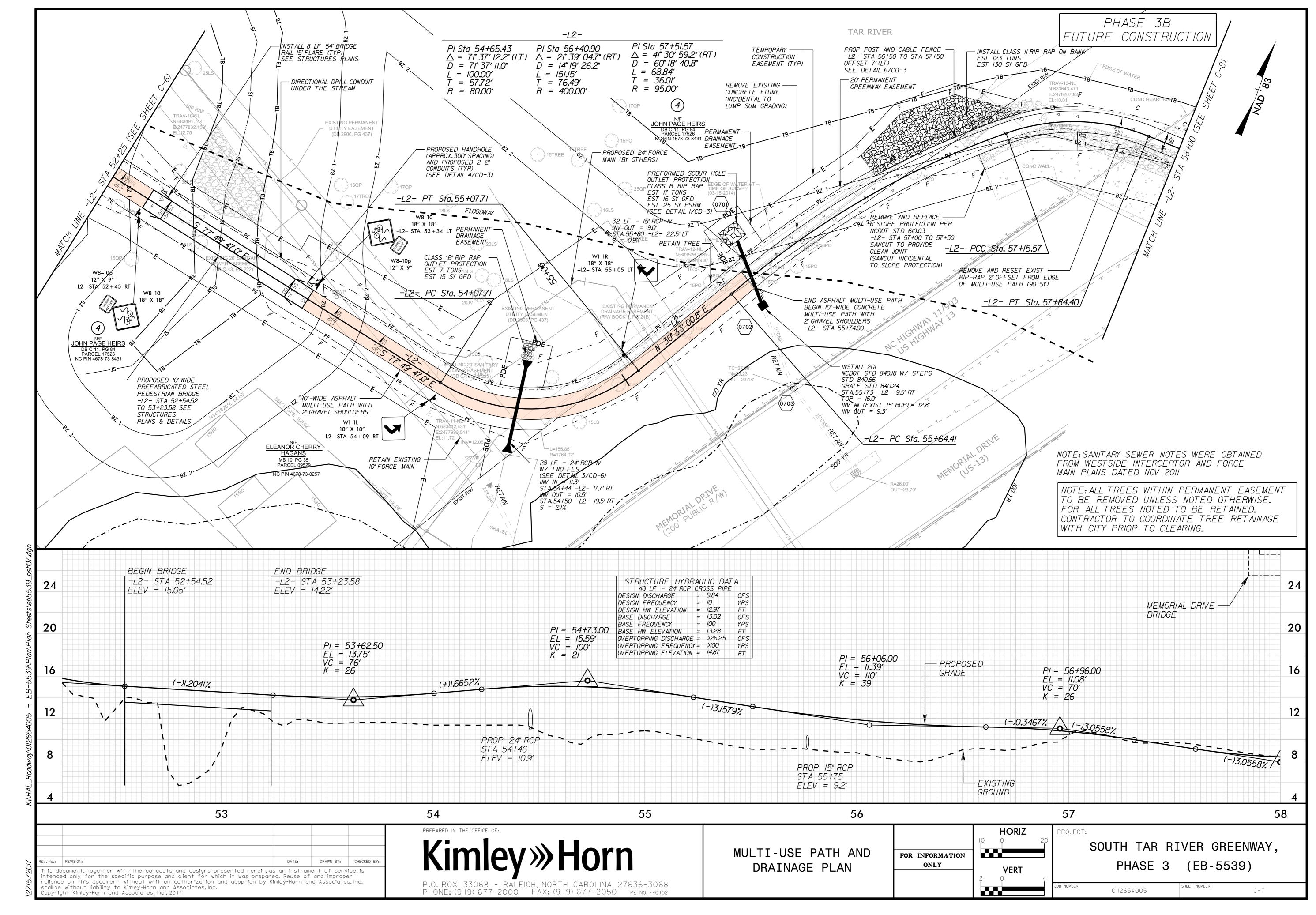


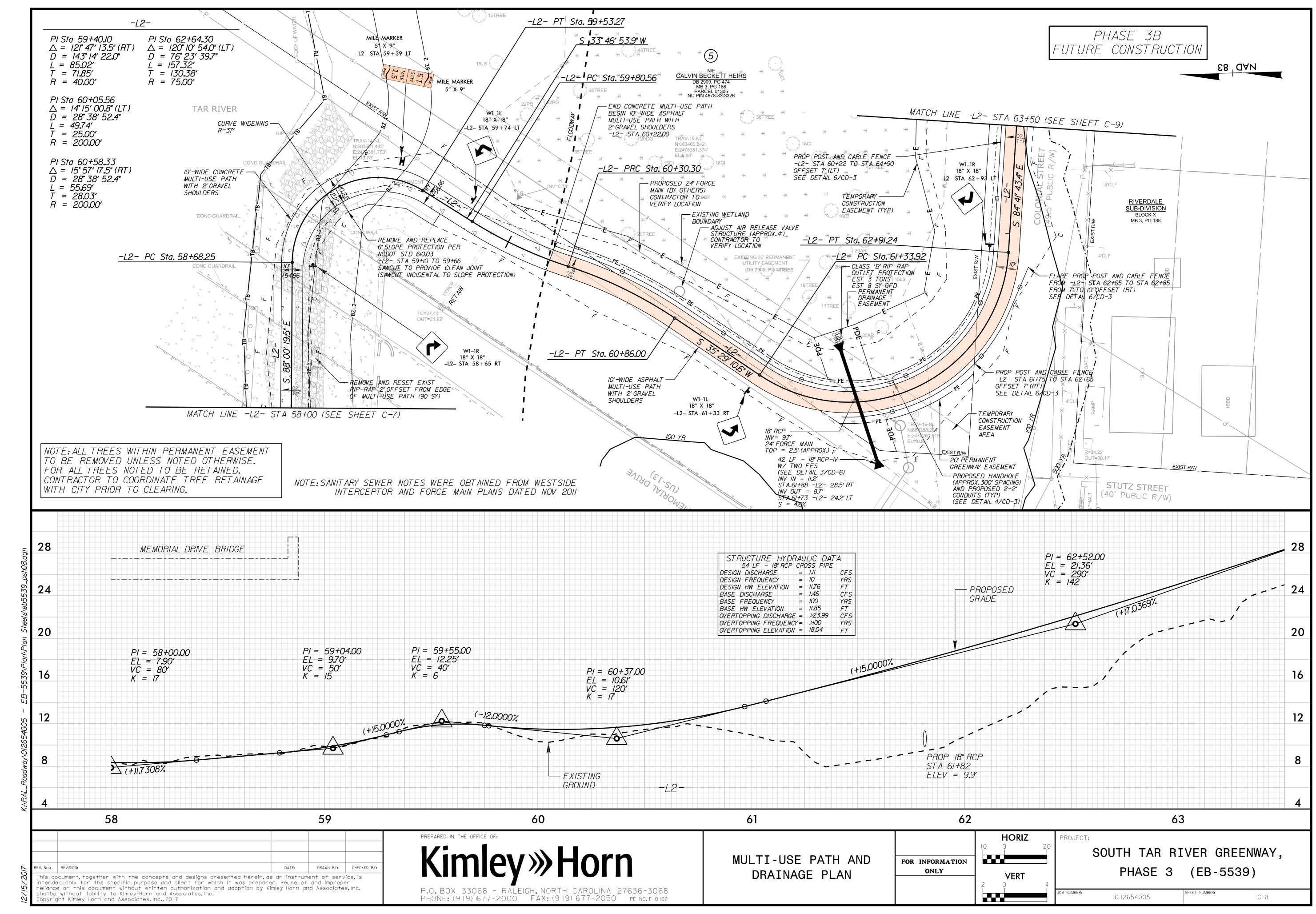


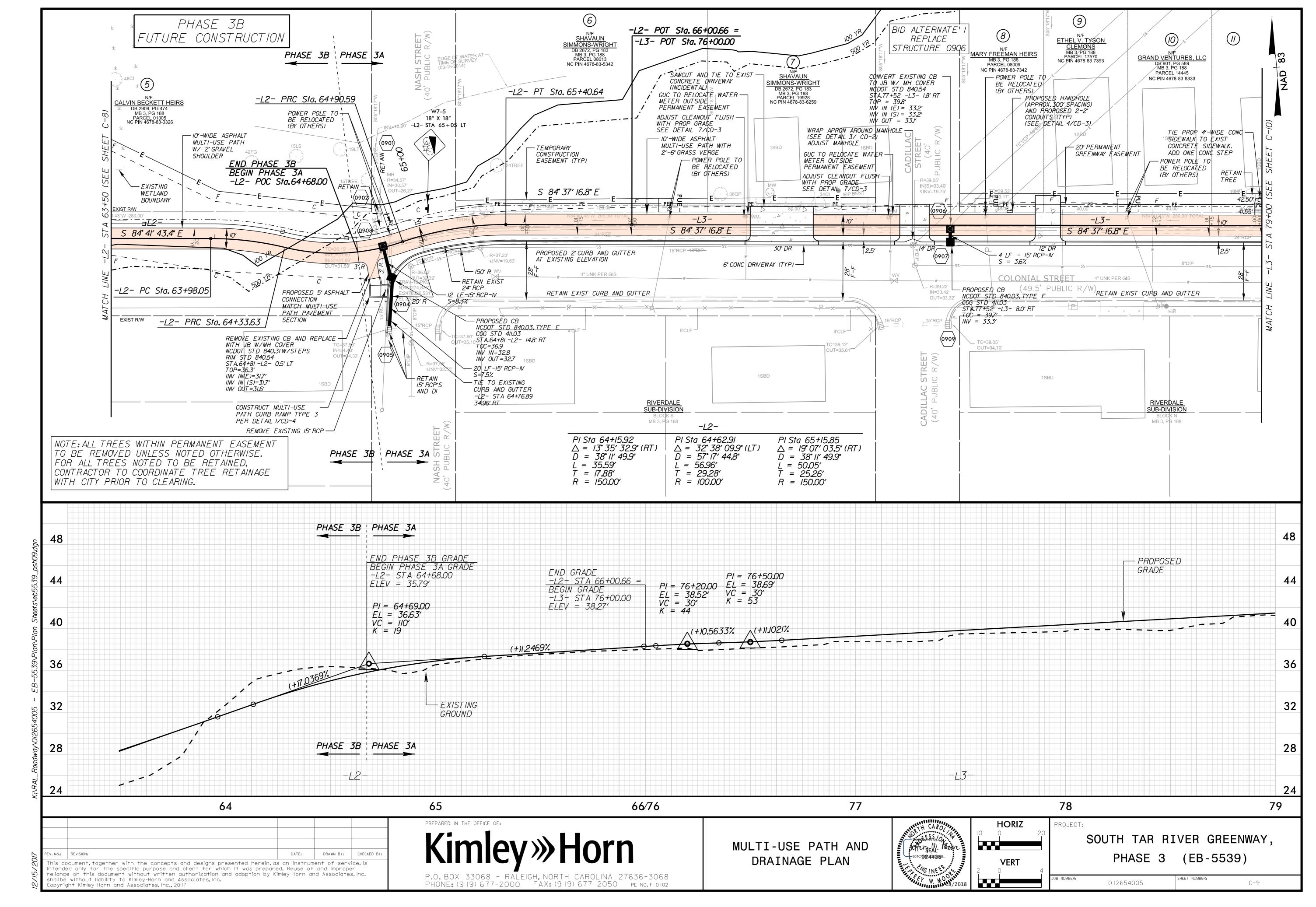


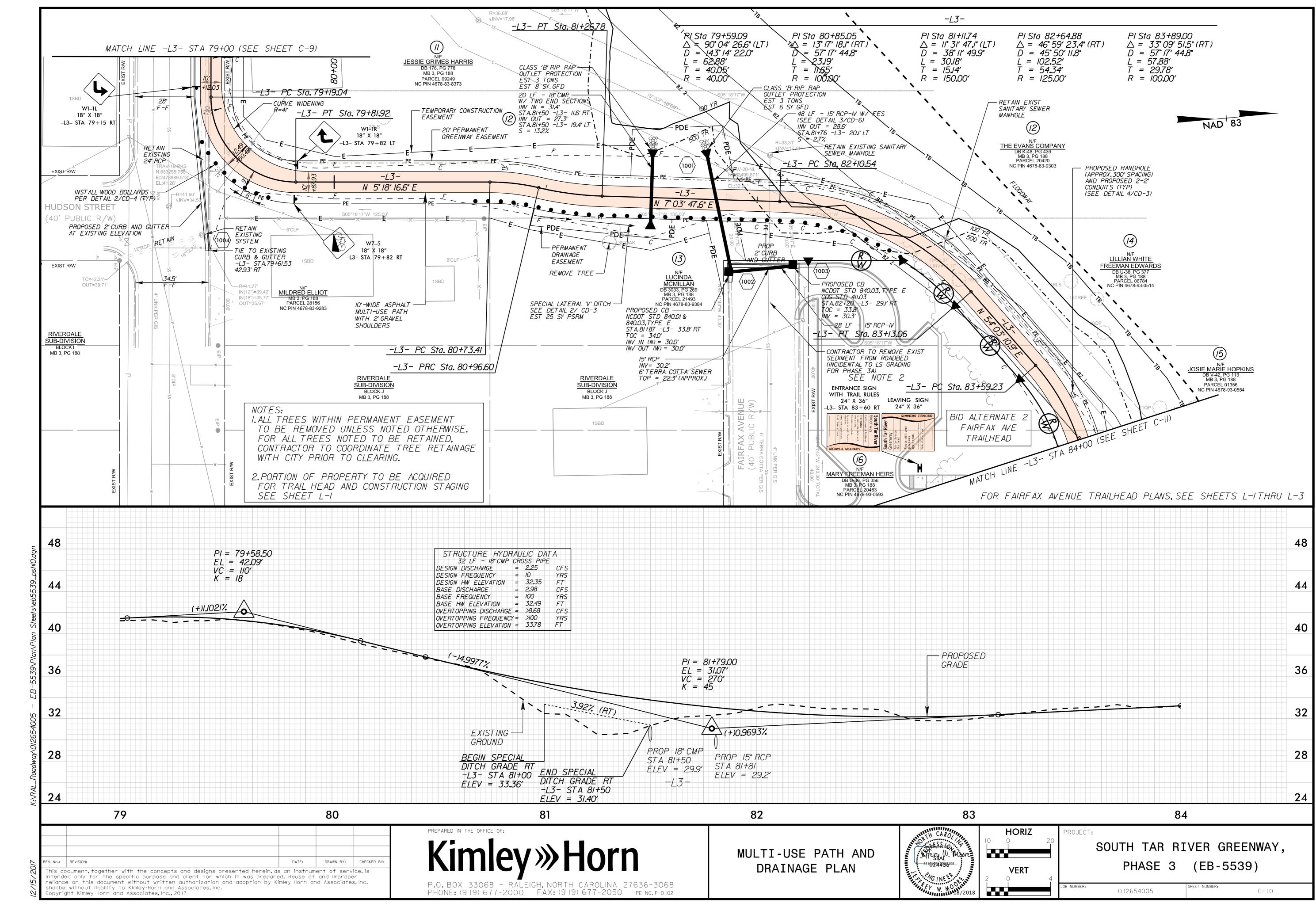


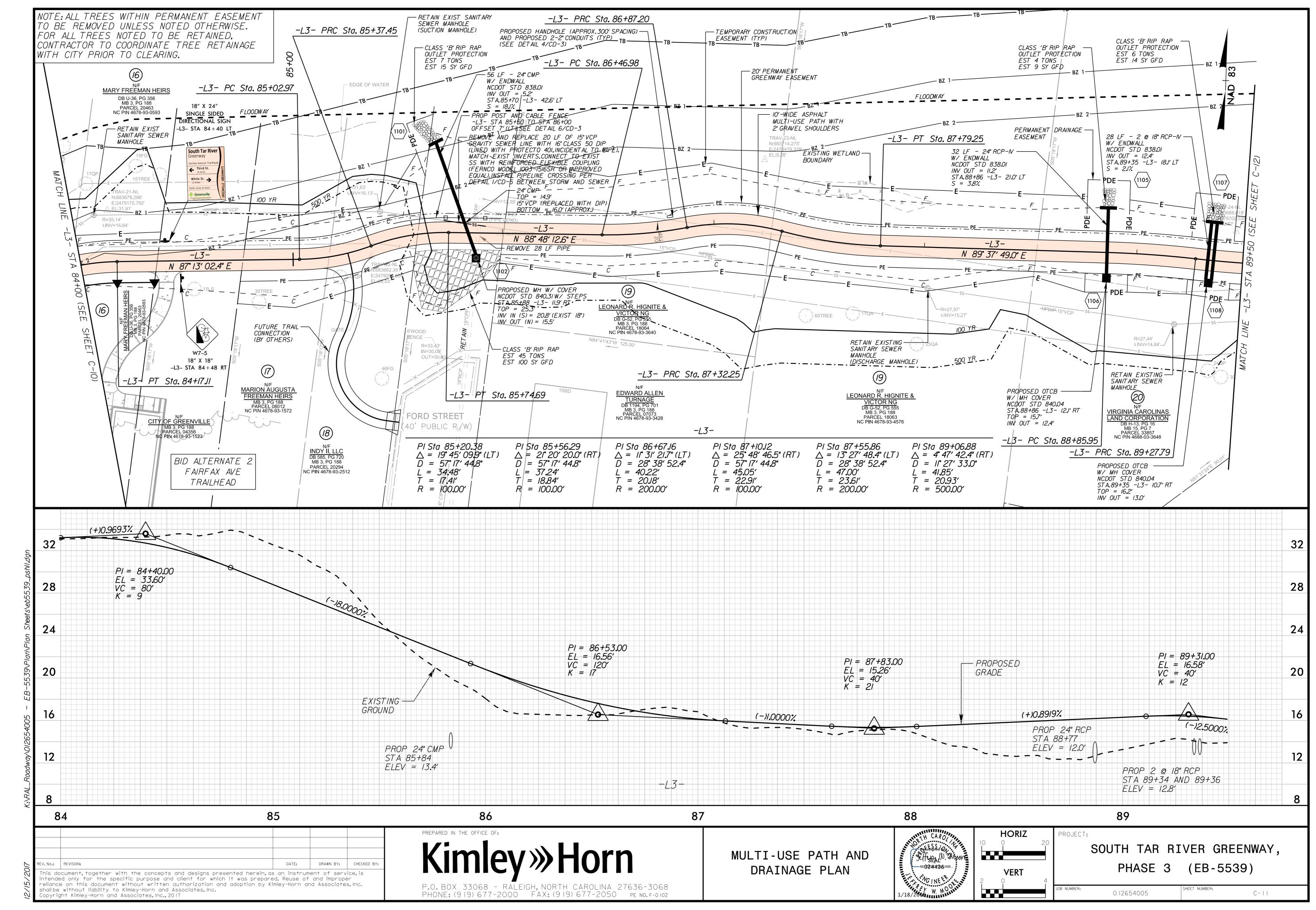


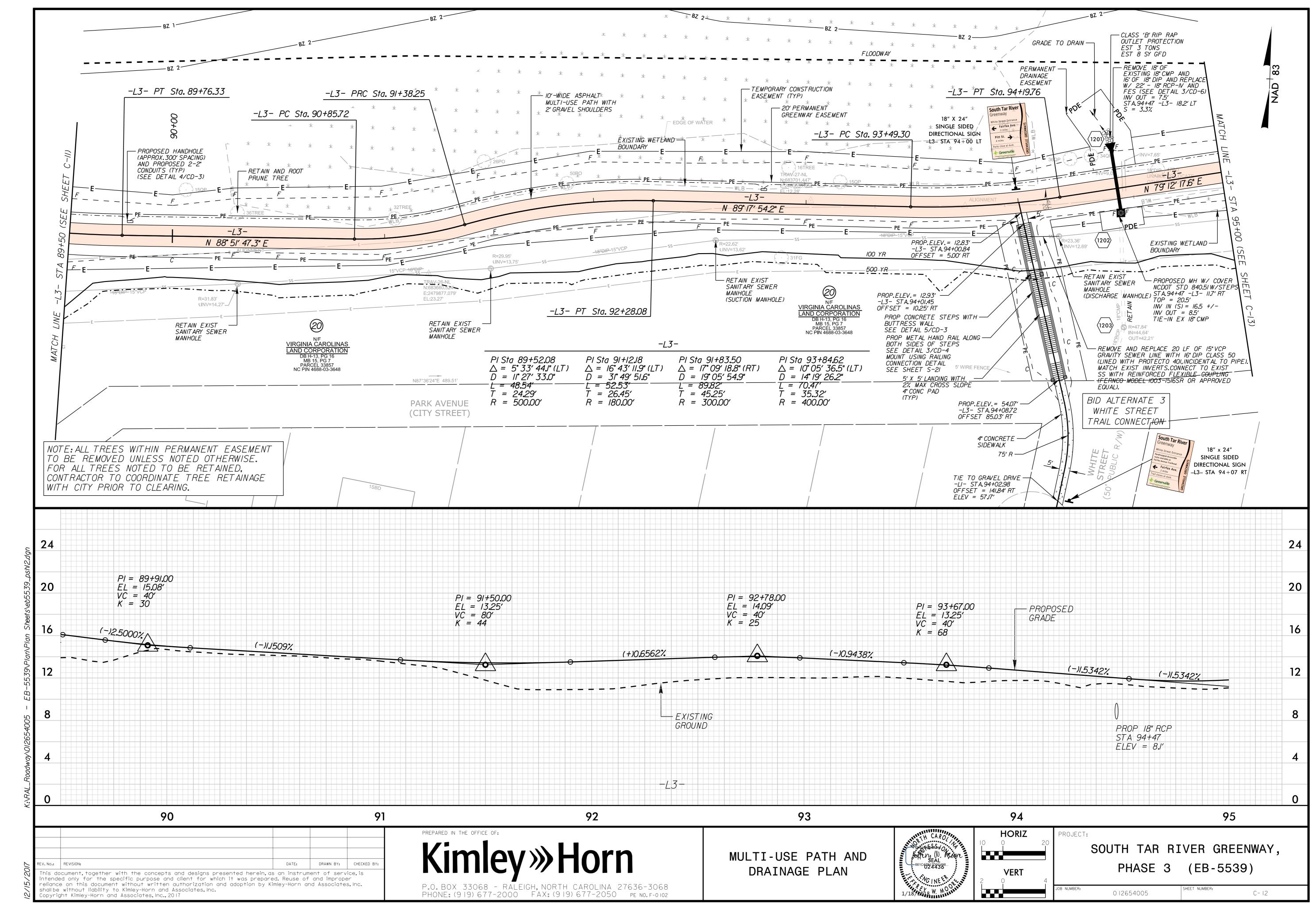


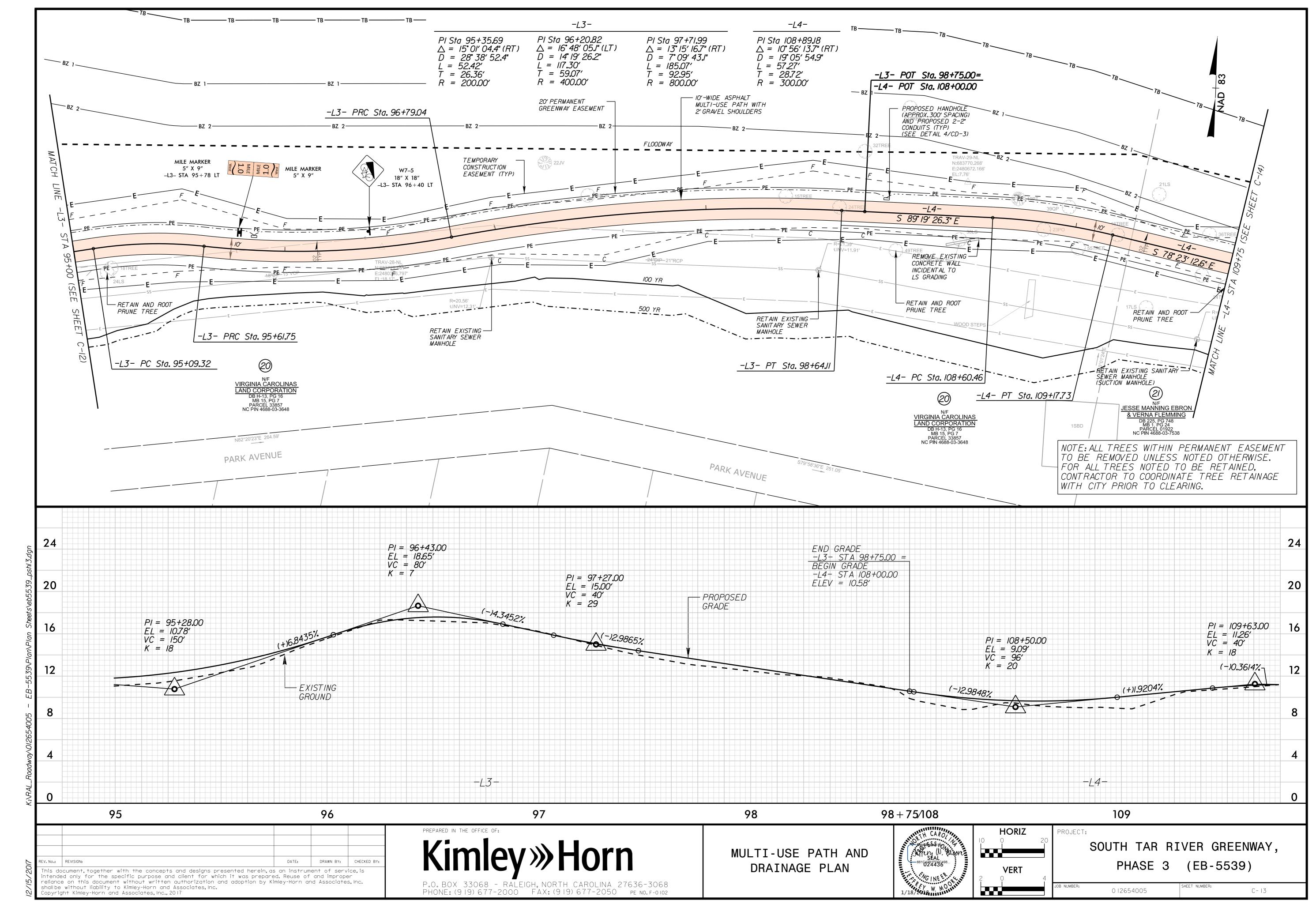


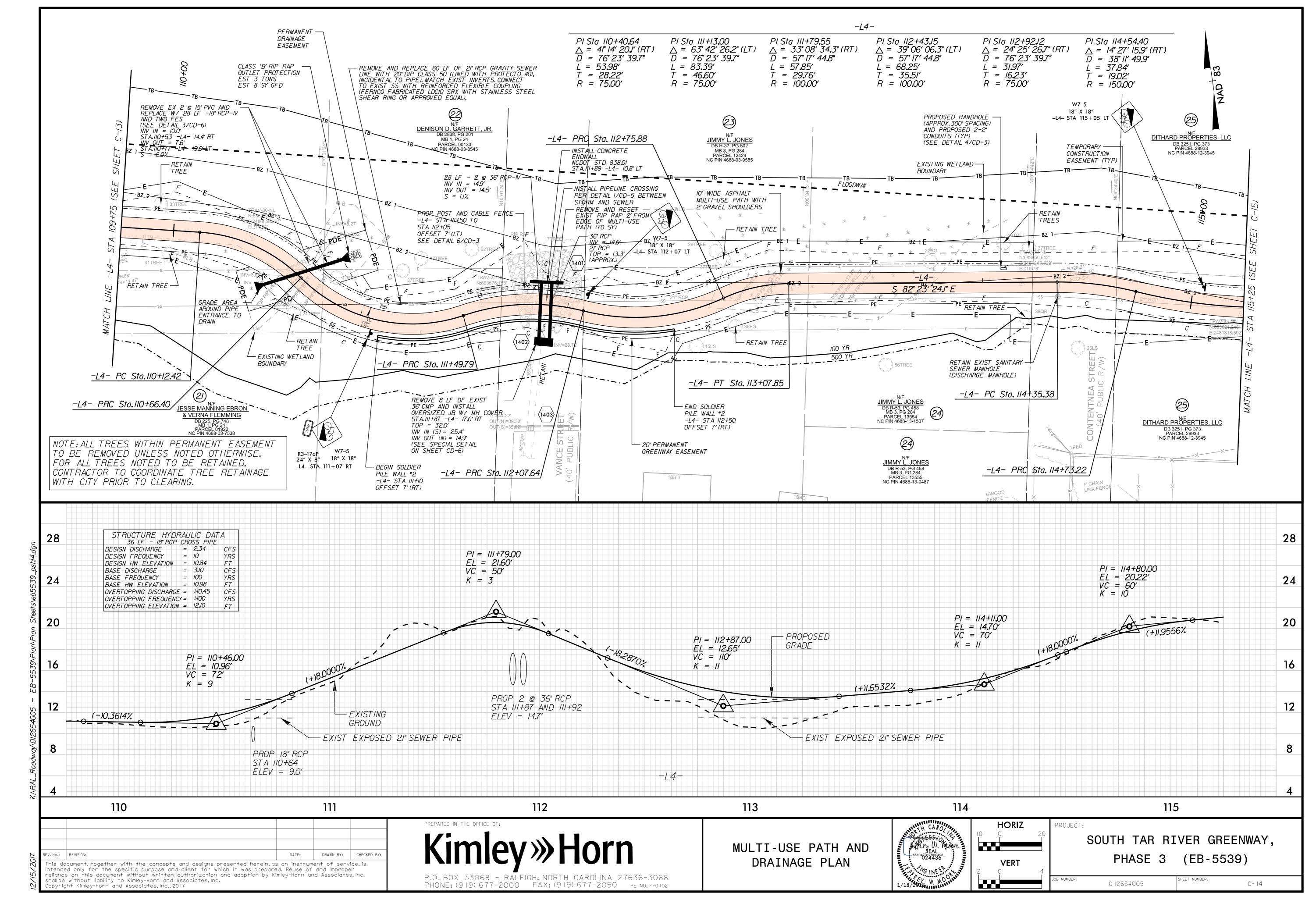


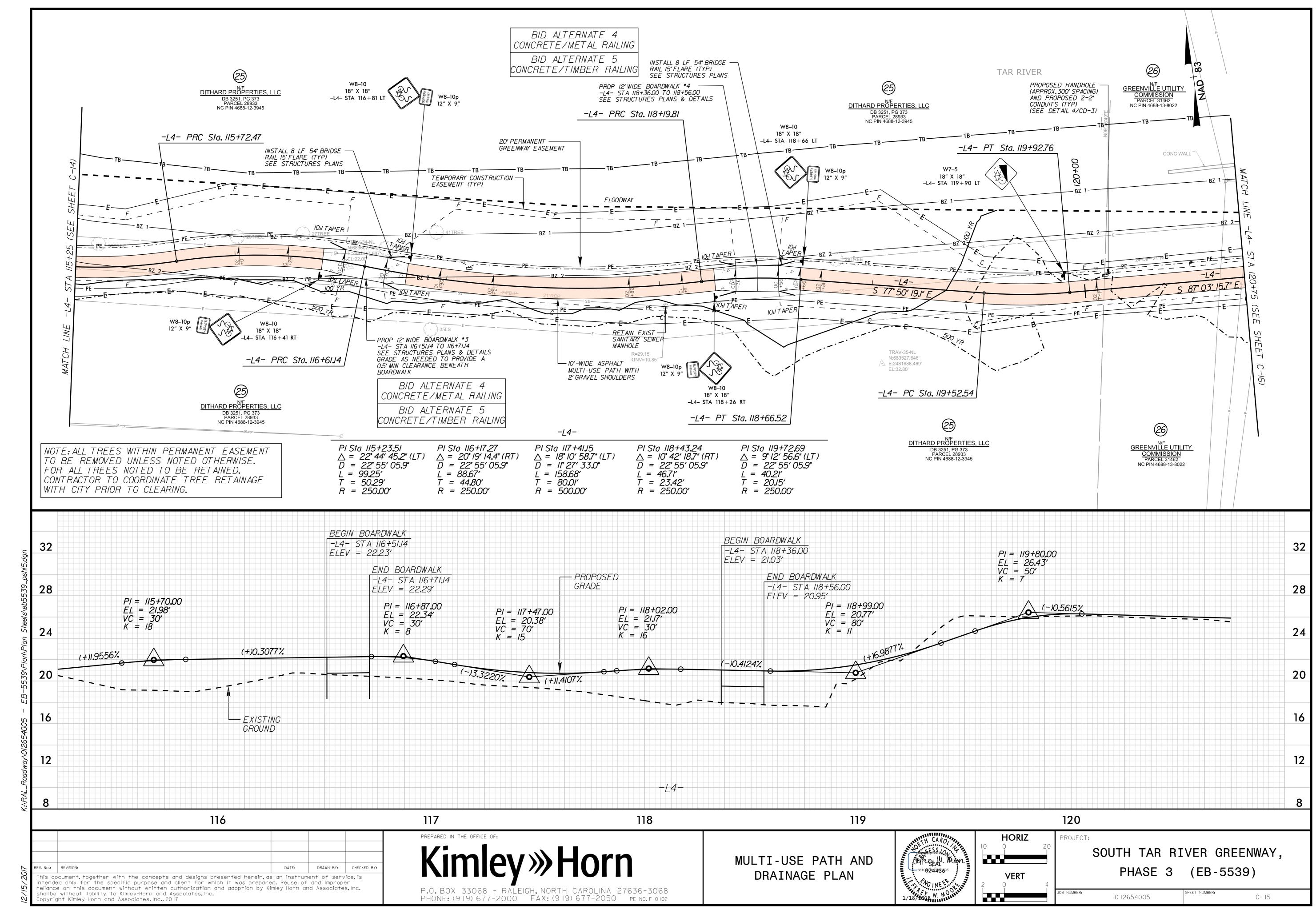


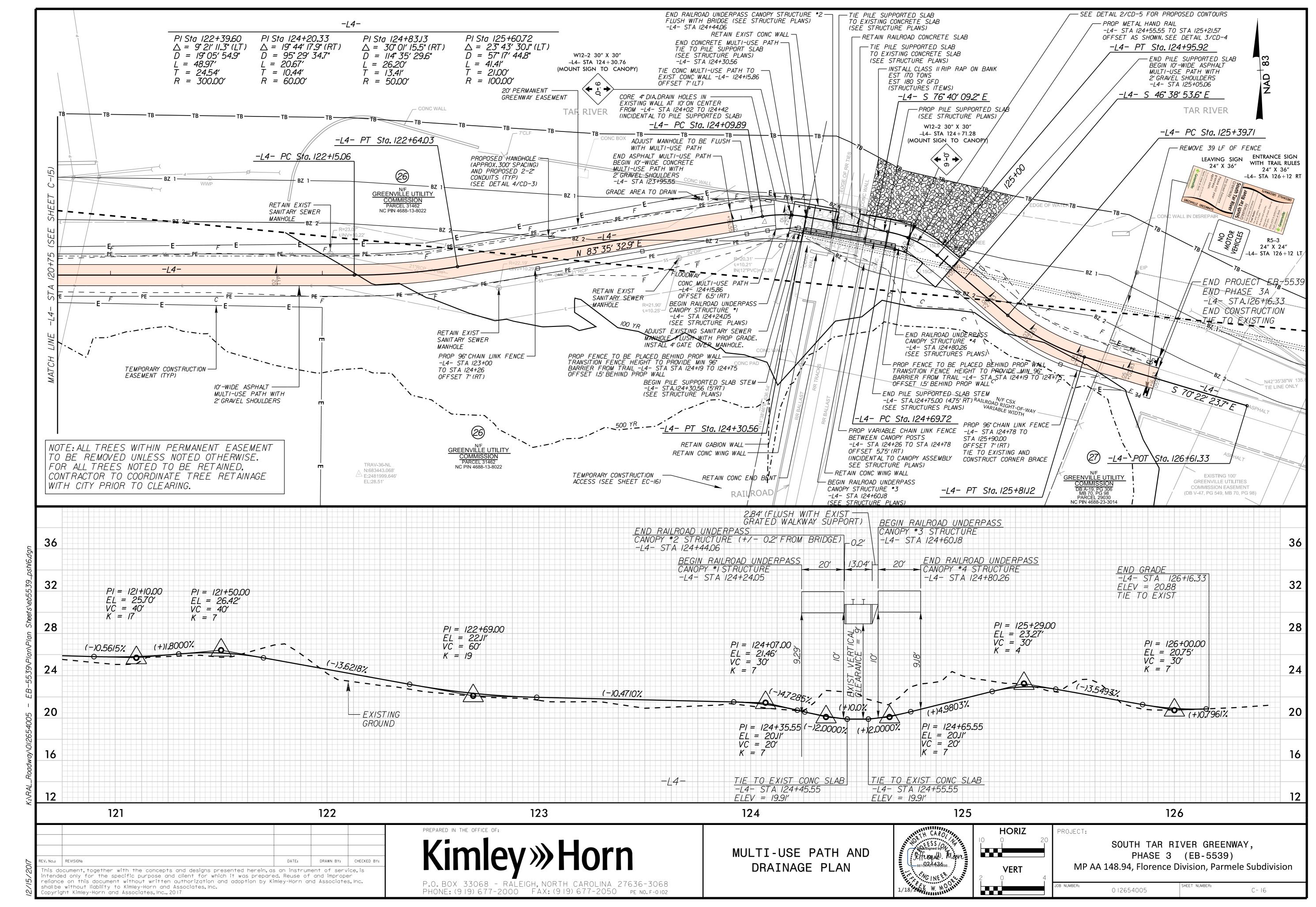












LIST OF ROADWAY STANDARD DRAWINGS THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" HIGHWAY DESIGN BRANCH-N.C. DEPARTMENT OF TRANSPORTATION-RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS: DESCRIPTION 200.02 METHOD OF CLEARING - METHOD II 225.02 GUIDE FOR GRADING SUBGRADE - SECONDARY AND LOCAL 300.01 METHOD OF PIPE INSTALLATION 610.03 GUIDE FOR PAVING SHOULDERS UNDER BRIDGES - METHOD III 815.03 PIPE UNDERDRAIN AND BLIND DRAIN 838.01 CONCRETE ENDWALL FOR SINGLE AND DOUBLE PIPE CULVERTS 840.00 CONCRETE BASE PAD FOR DRAINAGE STRUCTURES 840.01 BRICK CATCH BASIN - 12" THRU 54" PIPE

CONCRETE CATCH BASIN - 12" THRU 54" PIPE 840.02 840.03 FRAME, GRATES AND HOOD - FOR USE ON STANDARD CATCH BASIN 840.04 CONCRETE OPEN THROAT CATCH BASIN - 12" THRU 48" PIPE

840.05 BRICK OPEN THROAT CATCH BASIN - 12" THRU 48" PIPE

840.14 CONCRETE DROP INLET 840.15 BRICK DROP INLET 840.16

DROP INLET FRAME AND GRATES 840.18 CONCRETE GRATED DROP INLET TYPE 'B' - 12" THRU 36" PIPE

840.24 FRAMES AND NARROW SLOT SAG GRATES 840.31 CONCRETE JUNCTION BOX - 12" THRU 36" PIPE 840.32 BRICK JUNCTION BOX - 12" THRU 66" PIPE 840.34 TRAFFIC BEARING JUNCTION BOX

840.45 PRECAST DRAINAGE STRUCTURE BRICK MANHOLE - 12" THRU 36" PIPE 840.51 PRECAST MANHOLE - 4', 5' AND 6' DIAMETER 840.52 840.54 MANHOLE FRAME AND COVER

DRAINAGE STRUCTURE STEPS 840.66 846.01 CONCRETE CURB, GUTTER AND CURB AND GUTTER 848.01 CONCRETE SIDEWALK

848.02 DRIVEWAY TURNOUT - RADIUS TYPE 848.05 CURB RAMP (EXISTING CURB) 848.06 CURB RAMP (PROPOSED CURB)

866.01 CHAIN LINK FENCE - 4', 5' AND 6' HIGH FENCE 876.02 GUIDE FOR RIP RAP AT PIPE OUTLETS

904.50 MOUNTING OF TYPE 'D', 'E', AND 'F' SIGNS ON 'U' CHANNEL POSTS 1101.01 WORK ZONE ADVANCE WARNING SIGNS

1101.02 TEMPORARY LANE CLOSURES 1101.04 TEMPORARY SHOULDER CLOSURES 1101.11 TRAFFIC CONTROL DESIGN TABLES 1110.01 STATIONARY WORK ZONE SIGNS 1110.02 PORTABLE WORK ZONE SIGNS

1205.01 PAVEMENT MARKINGS - LNE TYPES AND OFFSETS 1130.01

1135.01 1145.01 BARRICADES - TYPE III 1150.01 FLAGGING DEVICES 1180.01 SKINNY DRUM 1605.01 TEMPORARY SILT FENCE

1606.01 SPECIAL SEDIMENT CONTROL FENCE 1607.01 GRAVEL CONSTRUCTION ENTRANCE 1630.06 SPECIAL STILLING BASIN 1631.01 MATTING INSTALLATION

1632.03 ROCK INLET SEDIMENT TRAP TYPE 'C' TEMPORARY ROCK SILT CHECK TYPE 'A' 1633.01

SITE NOTES

ALL RADII DIMENSIONS ARE MEASURED TO EDGE OF PAVEMENT, TO CENTERLINE CENTER TO CENTER ON STRIPES, AND/OR TO FACE OF CURB, UNLESS OTHERWISE NOTED.

PROVIDE CONSTRUCTION JOINTS IN CONCRETE WALKWAYS EVERY 10' MAXIMUM WHEN WALKS ARE 10' WIDE. MATCH WIDTH UNDER 10'.

CONCRETE PADS AND WALKWAYS: UNLESS OTHERWISE NOTED, ARE 3000 PSI, OF 6" THICKNESS WITH NO. 4 BARS 12" O.C. ALL SIGNS AND PAVEMENT MARKINGS ARE TO MEET MUTCD (MANUAL ON UNIFORM TRAFFIC

CONTROL DEVICES) AND NCDOT STANDARDS.

CONTRACTOR TO GRADE FILL SLOPES AROUND EXISTING MANHOLES TO AVOID PONDING

ALL RC PIPES SHALL BE CLASS III OR CLASS IV AS SHOWN ON PLANS.

UTILITY NOTES

CONTRACTOR SHALL NOTIFY "NORTH CAROLINA ONE CALL" (TELEPHONE 1-800-632-4949) AT LEAST 72 HOURS PRIOR TO BEGINNING CONSTRUCTION OR EXCAVATION TO HAVE EXISTING UTILITIES LOCATED. CONTRACTOR TO CONTACT LOCAL UTILITIES THAT PROVIDE THEIR OWN LOCATOR SERVICES INDEPENDENT OF "NORTH CAROLINA ONE CALL."

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION OF ALL EXISTING UTILITIES IN COORDINATION WITH THE APPROPRIATE UTILITY, AGENCY, OR COMPANY.

ALL UTILITIES THAT ARE LOCATED WITHIN LIMITS OF DISTURBANCE SHALL BE SET SO THAT TOPS/RIMS ARE FLUSH WITH FINISHED GRADE UNLESS OTHERWISE NOTED.

LOCATIONS AND SIZES OF EXISTING PUBLIC AND PRIVATE UTILITIES SHOWN ON THESE PLANS ARE FROM CITY AND UTILITY COMPANY RECORDS ONLY. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR FIELD LOCATING ALL UTILITIES AND FOR DAMAGES RESULTING FROM FAILURE TO DO SO.

CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING WITH THE CITY AND GREENVILLE UTILITIES COMMISSION FOR ANY ADDITIONAL INFORMATION ON EXISTING WATER AND SEWER UTILITIES.

LOCATIONS OF EXISTING UTILITIES ARE APPROXIMATE. CONTRACTOR SHALL BE RESPONSIBLE FOR FIELD VERIFYING EXACT LOCATION, ORIENTATION, AND ELEVATION OF EXISTING UTILITIES PRIOR TO BEGINNING CONSTRUCTION OR ORDERING MATERIALS.

CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY SHOULD ANY FIELD CONDITIONS BE ENCOUNTERED THAT VARY FROM THE INFORMATION PROVIDED IN THE CONTRACT DOCUMENTS.

DRAWN BY: CHECKED BY: his document, together with the concepts and designs presented herein, as an instrument of service, is tended only for the specific purpose and client for which it was prepared. Reuse of and improper eliance on this document without written authorization and adoption by Kimley-Horn and Associates, Inc. hallbe without liability to Kimley-Horn and Associates, Inc.

yright Kimley-Horn and Associates, Inc., 2017

GENERAL NOTES

2012 SPECIFICATIONS

STATEMENT OF CONSTRUCTION

THE PROJECT GENERALLY CONSISTS OF THE CONSTRUCTION OF A 10' WIDE MULTI-USE PATH, BRIDGE AND BOARDWALKS. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH NCDOT STANDARD SPECIFICATIONS, DATED 2012.

GRADE LINE:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED OR FUTURE SURFACE AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

THE MULTI-USE PATH IS TO BE SUPERELEVATED AS SHOWN ON PLAN SHEETS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE: GREENVILLE UTILITIES COMMISSION (GUC)

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON THE PLANS.

CURB RAMPS

CURB RAMPS ARE SHOWN ON THE PLANS AT APPROXIMATE LOCATIONS. CONSTRUCT ALL CURB RAMPS IN ACCORDANCE WITH NCDOT STD 848.05, 848.06 AND DETAILS SHOWN IN PLANS.

CONTRACTOR TO ADAPT STANDARD DETAIL TO CONSTRUCT 8' MIN. DROP CURB AT INTERSECTION WITH TRANSITION TO 10' WIDTH OF MULTI-USE PATH.

SAFETY:

PEDESTRIAN AND BICYCLE SAFETY MUST BE MAINTAINED AT ALL TIMES BY ADEQUATE PROJECT LIMITS, FENCING, AND SIGNAGE.

EROSION CONTROL:

THE FINAL SIZE AND LOCATION OF ALL EROSION CONTROL DEVISES MAY BE ADJUSTED BY THE ENGINEER IN THE FIELD. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO PLACING EROSION CONTROL MEASURES.

TRAFFIC CONTROL:

TRAFFIC CONTROL GENERAL NOTES, DETAILS, AND A LIST OF STANDARDS ARE INCLUDED IN THIS PLAN SET. ALL TRAFFIC CONTROL DEVICES AND OPERATIONS SHALL CONFORM TO NCDOT 2012 STANDARD DRAWINGS FOR TRAFFIC CONTROL.

EXISTING SURVEYS:

PREPARED IN THE OFFICE OF:

SURVEYS HAVE BEEN PROVIDED BY STEWART

TREE PRESERVATION:

REFER TO CONTRACT SPECIAL PROVISIONS AND NOTES ON SHEET EC-1 FOR TREE PRESERVATION REQUIREMENTS. CONTRACTOR TO STAKE THE PATH ALIGNMENT AND PAVEMENT EDGES PRIOR TO TREE CLEARING TO PROVIDE CITY REPRESENTATIVES AN OPPORTUNITY TO REVIEW TREES TO BE CLEARED. CONTRACTOR AND CLEARING CONTRACTOR TO WALK THE STAKED ALIGNMENT WITH CITY REPRESENTATIVES PRIOR TO TREE CLEARING.

SHEET NOTES

EXISTING UTILITIES AND STRUCTURES SHOWN, BOTH UNDERGROUND AND ABOVE, ARE BASED ON A FIELD SURVEY PERFORMED BY STEWART AND THE BEST AVAILABLE RECORD DRAWINGS.

ALL DEMOLITION, AND ANY SUBSEQUENT CONSTRUCTION, SHALL BE PERFORMED IN ACCORDANCE WITH THESE PLANS AND SPECIFICATIONS SET FORTH AND APPROVED BY THE CITY OF GREENVILLE. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL REQUIRED PERMITS. ALL TREE PROTECTION FENCING SHALL REMAIN IN PLACE DURING CONSTRUCTION.

TRAFFIC CONTROLS FOR ANY WORK WITHIN THE PUBLIC RIGHT OF WAY SHALL BE PERFORMED IN COMPLIANCE WITH STANDARDS OF THE NCDOT STD DRAWINGS AND THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) ENGINEER AND/OR OWNER DISCLAIM ANY ROLE IN THE CONSTRUCTION MEANS AND/OR METHODS ASSOCIATED WITH THE PROJECT AS SET FORTH IN THESE PLANS.

CONTRACTOR SHALL MAINTAIN AN "AS BUILT" SET OF DRAWINGS TO RECORD THE ACTUAL LOCATION OF ALL PIPING PRIOR TO CONCEALMENT. DRAWINGS SHALL BE GIVEN TO THE ENGINEER AT REGULAR INTERVALS THROUGHOUT THE PROJECT FOR RECORD KEEPING.

THE CONTRACTOR SHALL, AT ALL TIMES, KEEP THE PREMISES FREE FROM ACCUMULATIONS OF WASTE MATERIALS OR RUBBISH CAUSED BY HIM, HIS EMPLOYEES OR HIS WORK. ALL DEBRIS SHALL BE REMOVED FROM THE PROJECT SITE ON A DAILY BASIS IF DEPARTURES FROM THE DRAWINGS OR SPECIFICATIONS ARE DEEMED NECESSARY BY THE CONTRACTOR, DETAILS OF SUCH DEPARTURES AND REASONS THEREOF SHALL BE SUBMITTED TO THE OWNER FOR REVIEW. NO DEPARTURES FROM THE CONTRACT DOCUMENTS SHALL BE MADE WITHOUT THE EXPRESS WRITTEN PERMISSION OF THE OWNER.

THE CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING THE ACTUAL AND EXACT LOCATION, SIZE AND MATERIAL COMPOSITION OF ANY EXISTING WATER OR SEWER SERVICE PROPOSED FOR CONNECTION OR USE ON THIS PROJECT

ALL SIGNS SHALL BE MOUNTED WITH SIGN EDGE AND TO BE LOCATED A MINIMUM OF 3 FEET FROM EDGE OF TRAIL OR AS DIRECTED BY THE ENGINEER.

DEMOLITION NOTES

THE CONTRACTOR SHALL NOTIFY THE CITY PRIOR TO STARTING WORK.

THE CONTRACTOR SHALL NOT MAKE ANY LANE CLOSURES OR CHANGES TO THE EXISTING TRAVEL PATTERNS ON ANY PUBLIC STREET WITHOUT PRIOR APPROVAL FROM THE CITY AND/OR NCDOT.

THE CONTRACTOR IS RESPONSIBLE FOR CONFORMING TO ALL LOCAL. STATE AND FEDERAL REQUIREMENTS REGARDING REMOVAL AND DISPOSAL OF MATERIALS AND DEBRIS.

ALL DEMOLITION WORK WILL BE COORDINATED BY CONTRACTOR.

RELOCATION OF EXISTING UTILITIES TO BE COORDINATED WITH THE LOCAL UTILITY PROVIDER(S).

CLEANOUTS LOCATED IN AREAS OF DEMOLITION OR SUBSEQUENT CONSTRUCTION THAT ARE TO REMAIN, SHALL BE PROTECTED FROM DAMAGE AND RAISED TO FLUSH WITH NEW GRADE.

ALL ITEMS DESIGNATED TO BE REMOVED SHALL BE DISPOSED OF LEGALLY OFFSITE UNLESS OTHERWISE NOTED ON THIS PLAN.

ALL TREES AND VEGETATION NOTED TO BE REMOVED SHALL BE GROUND INTO MULCH, AND STOCKPILED FOR FUTURE USE. TREE PROTECTION MEASURES SHALL BE PROVIDED DURING CONSTRUCTION PER APPLICABLE PERMITS AND THE BID DOCUMENTS.

ALL PAVEMENT MARKINGS USED FOR PROJECT SHALL BE NCDOT STANDARD THERMOPLASTIC PAVEMENT MARKINGS.

ALL WORK MUST BE COMPLETED WITHIN EASEMENTS AND CONSTRUCTION LIMITS SHOWN.

REMOVE EXISTING CONCRETE (WHERE REQUIRED) TO FIRST COLD JOINT OR SAW CUT TO OBTAIN A CLEAN EDGE FOR NEW CONSTRUCTION. SAW CUT EXISTING ASPHALT DRIVE AT LIMITS OF NEW CURBING TO OBTAIN A CLEAN EDGE. CITY SHALL APPROVE ALL SAWCUT LIMITS PRIOR TO REMOVAL TO ENSURE THAT CITY STANDARDS ARE MET.

CONTRACTOR SHALL RESTORE THE LAY-DOWN AND STAGING AREA TO ORIGINAL CONDITIONS AND TO THE SATISFACTION OF THE OWNER, PRIOR TO DEMOBILIZATION AT THE CONCLUSION OF THE PROJECT.

CLEAN SOILS SHALL BE UTILIZED FOR BACKFILL COMPACTION OF THESE SOILS PERFORMED IN ACCORDANCE WITH SPECIFICATIONS AND DRAWINGS.

ALL GRAVEL TO BE REMOVED (SURFACE OR SUBSURFACE) SHALL BE STOCKPILED AND REUSED ON SITE WHERE POSSIBLE IF IT CONFORMS TO SPECIFICATIONS AND DRAWINGS.

ALL ITEMS DESIGNATED TO BE REMOVED SHALL BE REMOVED COMPLETELY, INCLUDING ALL SUBGRADE MATERIALS DIRECTLY ASSOCIATED WITH ITEMS TO BE REMOVED.

GUC UTILITY NOTES

MAINTAIN 36" MIN COVER OVER ALL EXISTING SANITARY SEWER LINES.

STANDARDS AND SPECIFICATIONS - ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH GUC SANITARY SEWER REGULATIONS AND MANUAL FOR THE CONSTRUCTION OF WATER AND WASTEWATER SYSTEMS, LATEST VERSION

PRECONSTRUCTION CONFERENCE - A PRECONSTRUCTION CONFERENCE WITH THE GUC IS REQUIRED BEFORE BEGINNING ANY WATER OR SEWER UTILITY CONSTRUCTION.

CONTACT THE NATIONAL "CALL BEFORE YOU DIG" 811 TO HAVE FACITILIES LOCATED BEFORE BEGINNING ANY EXCAVATION.

SEWER STATEMENT - SEWER LINES UNDER CONSTUCTION SHALL BE PLUGGED WITH A MECHANICAL PLUG AT THE FIRST MANHOLE UPSTREAM FROM THE POINT OF CONNECTION. PLUG SHALL BE PLACES IN THE OUTLET CONNECTION AND SECURED WITH STEEL CABLE. PLUG SHALL REMAIN IN PLACE UNTIL ACCEPTANCE OF LINES BY GUC. BEFORE ANY CONSTRUCTION IS TO BEGIN ON THE SITE, CONTRACTOR SHALL PROTECT ALL EXISTING SEWER MANHOLES WITH IRON FENCE POST AND ORANGE TREE PROTECTION FENCING. WATER, STONE, DIRT, OR ANY OTHER DEBRIS SHALL NOT BE ALLOWED TO ENTER THE SANITARY SEWER SYSTEM DURING FLUSHING OPERATION OR AT ANY OTHER TIME. CONSTRUCTION TAKING PLACE IN THE VICINITY OF ANY EXISTING OWASA SEWER LINES OR MANHOLES SHALL NOT CAUSE ANY INFLOW OF SURFACE WATER OR DEBRIS TO ENTER THE SANITARY SEWER SYSTERM. EXISTING MANHOLES LOCATED IN CONSTRUCTION SITES ARE TO BE KEPT CLEAR AND ACCESSIBLE TO GUC PERSONNEL AT ALL TIMES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGES INCURRED TO THE SANITARY SEWER SYSTEM AND ANY FINES IMPOSED BY THE STATE OF NORTH CAROLINA DIVISION OF WATER QUALITY DUE TO SEWER SPILLS OR OVERFLOWS.

EXISTING WATER AND/OR SEWER LINES ENCOUNTERED DURING CONSTRUCTION MUST BE SUPPORTED IN A MANNER ACCEPTABLE TO GUC.

A BYPASS PUMPING PLAN SHALL BE PROVIDED TO THE ENGINEER AND GUC PERSONNEL FOR REVIEW AND APPROVIAL PRIOR TO ANY MODIFICATIONS TO THE EXISTING SANITARY SEWER. THE CONTRACTOR SHALL PROVIDE A BYPASS PUMPING MEETING THE REQUIREMENTS (INCLUDING FLOW AND REDUNDANCY) OF THE PLANS AND SPECIFICATIONS. PUMPS SHALL BE SIZED TO HANDLE THE FLOW REQUIREMENTS PER THE PLANS AND AS APPROVED BY GUC AND/OR ENGINEER.

BYPASS PUMPING NOTES

- 1. ALL PROPOSED BYPASS PUMPING OPERATIONS SHALL MEET THE CONDITIONS AND REQUIREMENTS OF THE GREENVILLE UTILITIES COMMISSION (GUC) SANITARY SEWER USE REGULATIONS AND MANUAL FOR THE CONSTRUCTION OF WATER AND WASTEWATER SYSTEM AND CONTRACT DOCUMENTS.
- CONTRACTOR SHALL OPERATE AND MAINTAIN BYPASS PUMPING WITHOUT RELIANCE ON GUC. THE BYPASS SHALL ONLY BE IN OPPERATION DURING NORMAL WORKING HOURS (8 AM TO 5 PM).
- THE BYPASS SHALL BE PERFORMED DURING DRY WEATHER CONDITIONS ONLY. DO NOT BEGIN BYPASS
- OPERATION IF RAIN IS FORECASTED WITHIN THE TIME PERIOD IN WHICH THE WORK IS EXPECTED TO BE COMPLETED.
- CONTRACTOR SHALL PLAN WORK AS TO MINIMIZE BYPASS PUMPING DURATIONS. ALL BYPASS PUMPING EQUIPMENT SHALL BE SOUND ATTENUATED AND SHALL BE DESIGNED TO HANDLE THE
- FLOWS SHOWN IN THE TABLE BELOW. BYPASSES SHALL BE EQUIPPED WITH CELLULAR PHONE BASED TELEMETRY SYSTEMS PROVIDING REMOTE
- ALARM REPORTING TO CONTRACTOR AND BYPASS SUBCONTRACTOR. ALL BYPASS SYSTEMS SHALL HAVE 100% REDUNDANCY WITH EQUIVALENT BACKUP PUMPS AND PIPING.
- ALL BYPASS PIPING SHALL BE FUSIBLE PIPE. NO STRUCTURES OR PIPE SHALL BE LOCATED LESS THAN 12 INCHES ABOVE THE STREAM ORDINARY HIGH WATER LINE.
- ALL BYPASS PUMPS AND PIPING SHALL BE INSPECTED BY THE CONTRACTOR A MINIMUM OF ONCE PER DAY FOR PROPER OPERATION
- 10. CONTRACTOR SHALL PROVIDE A SEPARATE DETAILED BYPASS PUMPING PLAN. THE BYPASS PUMPING PLAN
- SHALL INCLUDE THE FOLLOWING AT A MINIMUM: SKETCHES OF THE PROPOSED LAYOUT WITH PIPING SIZES
- PUMP CURVES AND DATA SHEETS FOR EACH PROPOSED PUMP
- TELEMETRY SYSTEM DATA SHEETS DETAILED SCHEDULE FOR BYPASS PUMPING
- WASTEWATER OPERATING AND CONTROL ELEVATIONS

AND THE WORK IS APPROVED BY THE ENGINEER AND/OR GUC.

- PUMP OPERATION SEQUENCE
- SOUND LEVELS FOR EACH PUMP AT 20 TO 25 FEET
- PUMP FAILURE RESPONSE PLAN SPILL RESPONSE PLAN
- AT LEAST FOUR (4) 24/7 EMERGENCY RESPONSE CONTACT NAMES AND PHONE NUMBERS.
- A LIST OF PERSONS/COMPANIES ON STANDBY TO HANDLE SPILLS RUNTIME FOR EACH PUMP AT FULL LOAD WITHOUT REFUELING
- 11. BYPASS PUMPING SHALL REMAIN ENTIRELY IN PLACE AND FULLY OPERATIONAL UNTIL ALL STORM AND SANITARY SEWER INSTALLATION IN THE BYPASSED SECTION HAVE BEEN COMPLETED

PROPOSED SHEET C-11 ANTICIPATED BYPASS CONDITIONS			PROPOSED SHEET C-	12 ANTICIPATED BYPASS CONDITIONS
SUCTION POINT	SUCTION MANHOLE ON SHEET C-11		SUCTION POINT	SUCTION MANHOLE ON SHEET C-12
DISCHARGE POINT	DISCHARGE MANHOLE ON SHEET C-11		DISCHARGE POINT	DISCHARGE MANHOLE ON SHEET C-12
SUCTION STATIC LIFT	18'		SUCTION STATIC LIFT	10'
DISCHARGE STATIC LIFT	N/A		DISCHARGE STATIC LIFT	N/A
LENGTH OF BYPASS	APPROX. 275 LF		LENGTH OF BYPASS	APPROX. 175 LF

╛		PROPOSED SHEET C-12 ANTICIPATED BYPASS CONDITIONS					
		SUCTION POINT	SUCTION MANHOLE ON SHEET C-12				
1		DISCHARGE POINT	DISCHARGE MANHOLE ON SHEET C-12				
		SUCTION STATIC LIFT	10'				
	DISCHARGE STATIC LIFT		N/A				
		LENGTH OF BYPASS	APPROX. 175 LF				
	PEAK FLOW RATE		1550 GPM				
5	TO DDODOSED BYDASS SETUD						

PROJECT:

12	S	SUCTION POINT	SUCTION MANHOLE ON SHEET C-13
C-12 DISCHARGE POINT D		SCHARGE POINT	DISCHARGE MANHOLE ON SHEET C-14
	SUC	TION STATIC LIFT	16'
	DISC	HARGE STATIC LIFT	N/A
	LEN	NGTH OF BYPASS	APPROX. 475 LF
	PE	EAK FLOW RATE	1550 GPM

PROPOSED SHEET C-14 ANTICIPATED BYPASS CONDITIONS

NOTES: CONTRACTOR SHALL VERIFY FLOW CONDITIONS PRIOR TO PROPOSED BYPASS SETUP.

1550 GPM

LIST OF ROADWAY AND GENERAL NOTES

. Dogusighed/pg/ Siffing W. Alson 661**627414306**498.

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SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

CD-I 0 12654005

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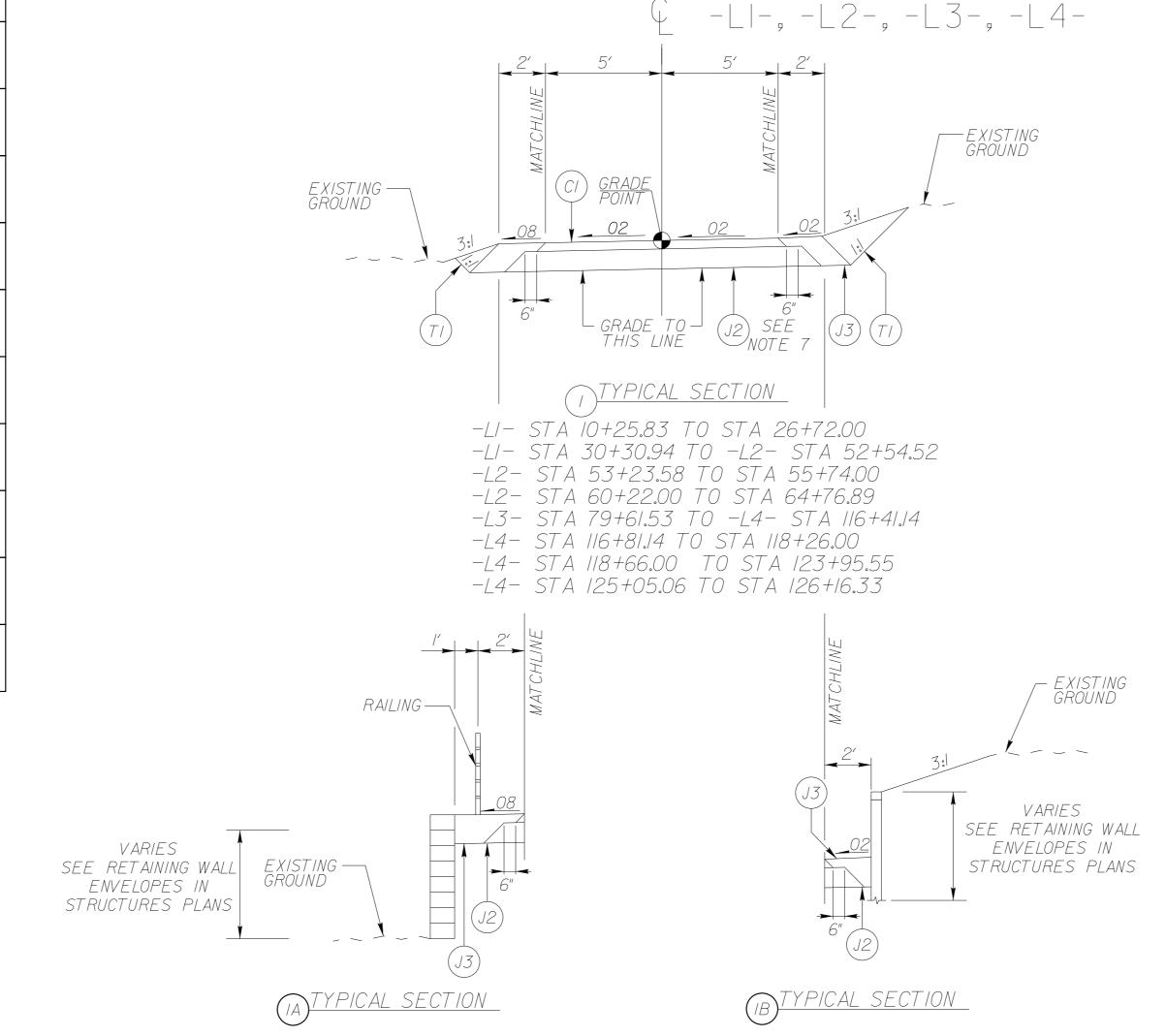
STANDARD DRAWINGS

PEAK FLOW RATE

	PAVEMENT SCHEDULE
Α	PROPOSED 6" REINFORCED CONCRETE SIDEWALK (UNLESS NOTED OTHERWISE)
C/	PROP. APPROX.1.5" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ.YD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS. PER SQ.YD. IN EACH OF 2 LAYERS
DI	PROP. APPROX. 4" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 119.0B, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
JI	PROPOSED 4" AGGREGATE BASE COURSE
J2	PROPOSED 6" AGGREGATE BASE COURSE
J3	PROPOSED AGGREGATE BASE COURSE (SHOULDER)
R	PROPOSED 2'-0" CONCRETE CURB AND GUTTER PER COG STD 411.01
TI	PROPOSED COMPACTED EARTH MATERIAL
<i>T2</i>	PROPOSED GRASS VERGE
U	EXISTING PAVEMENT

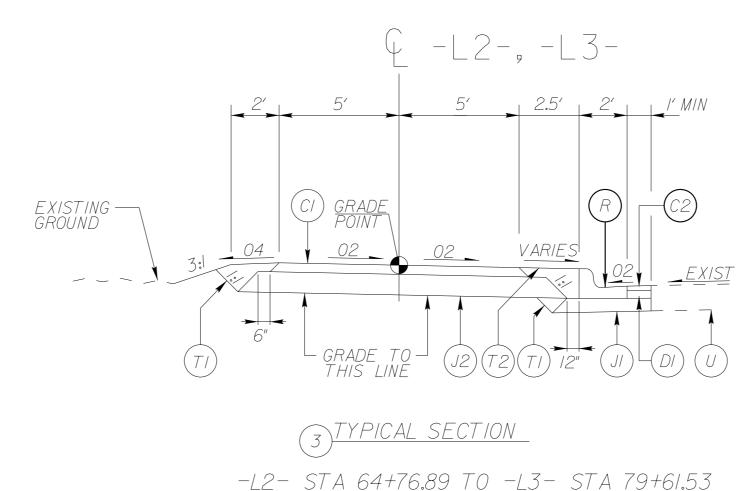
NOTES:

- 1. SEE PLANS FOR SPECIFIC MULTI-USE PATH LOCATIONS.
- 2. PAVEMENT EDGE SLOPES ARE 1:1 UNLESS OTHERWISE NOTED.
- 3. SEE PLAN SHEETS FOR MULTI-USE PATH SUPERELEVATION TRANSITIONS. (2% MAX SUPERELEVATION)
- 4. SLOPE SHOULDERS FOR POSITIVE DRAINAGE.
- 5. SIDE SLOPES VARY. SEE CROSS SECTIONS.
- 6. SEE STRUCTURE PLANS FOR PEDESTRIAN BRIDGE, BOARDWALK, CONCRETE SLAB ON PILES, AND RETAINING WALLS
- 7. AT THE DIRECTION OF THE ENGINEER OR FIELD INSPECTOR, THE AGGREGATE BASE COURSE MAY BE INCREASED TO 10".
- 8. SEE STRUCTURES PLANS FOR RETAINING WALL TYPES AND **DETAILS**



-LI- STA 30+20.94 TO STA 37+40.98 (LT)

-L4- STA III+10.00 TO STA II2+50.00 (RT)



(-L2-, -L4-#4 BARS @ 12" O.C. - EXISTING GROUND EXISTING — GROUND BEND - #4 BARS @ 12" O.C. (2) TYPICAL SECTION

- -LI- STA 27+95.87 TO STA 28+40.94
- -L2- STA 55+74.00 TO STA 60+22.00
- -L4- STA 123+95.55 TO STA 124+15.86
- -L4- STA 124+15.86 TO STA 124+30.56 (10" REINFORCED CONC)

PAY ITEM NOTES (SIGNING): A. REMOVE AND DISPOSE OF EXISTING "U" CHANNEL POST.

B. ERECT EXISTING SIGN ON STEEL "U" CHANNEL POST. C. REMOVE AND STOCKPILE EXISTING SIGN(S).

PAVEMENT MARKING LINES & SYMBOLS

- TA THERMOPLASTIC (2" WHITE, 90 MILS) WHITE EDGE LINE
- TH THERMOPLASTIC (4" YELLOW, 120 MILS) YELLOW SINGLE CENTER LINE T2 - THERMOPLASTIC (24" WHITE, 120 MILS) WHITE STOP BAR
- UC THERMOPLASTIC STRAIGHT ARROW (90 MIL)

SIGNING AND PAVEMENT MARKING NOTES:

- 1. ALL PAVEMENT MARKINGS SHALL BE THERMOPLASTIC.
- 2. CONTRACTOR SHALL TIE PROPOSED MARKINGS TO EXISTING MARKINGS. 3. CONTRACTOR SHALL MILL ANY EXISTING MARKINGS AND/OR SYMBOLS IN CONFLICT WITH PROPOSED.
- 4. DO NOT PLACE PAVEMENT MARKINGS WITHIN CROSSWALKS UNLESS NOTED.
- 5. CONTRACTOR TO STOCKPILE ANY REMOVED SIGNS AND PARKING SIGNS FOR CITY OF GREENVILLE TO USE AT THEIR DISCRETION.
- 6. RETAIN ALL OTHER EXISTING SIGNS NOT SHOWN ON SIGNING PLAN UNLESS OTHERWISE DIRECTED BY CITY.

REV. No.:	REVISION:	DATE:	DRAWN BY:	CHECKED BY:

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TYPICAL SECTIONS



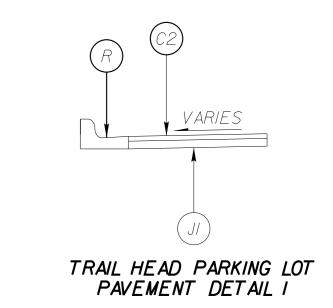
MULTI-USE PATH

DETAIL 2 - SIGN PLACEMENT

4' MIN

5' MAX

PROJECT: SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)



CURB TRANSITION DETAIL 3 WARP CURB AROUND MANHOLE WITHIN GUTTER LINE

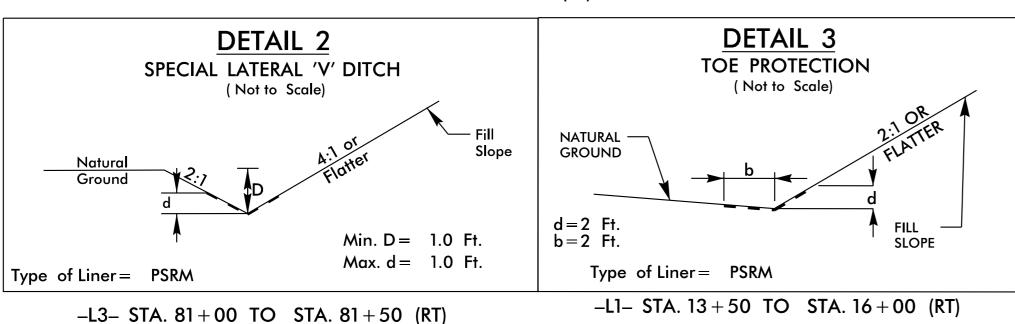
- MULTI-USE PATH -∠ ROADWAY ¬

CD-2

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-L2-STA. 55+80 (LT)

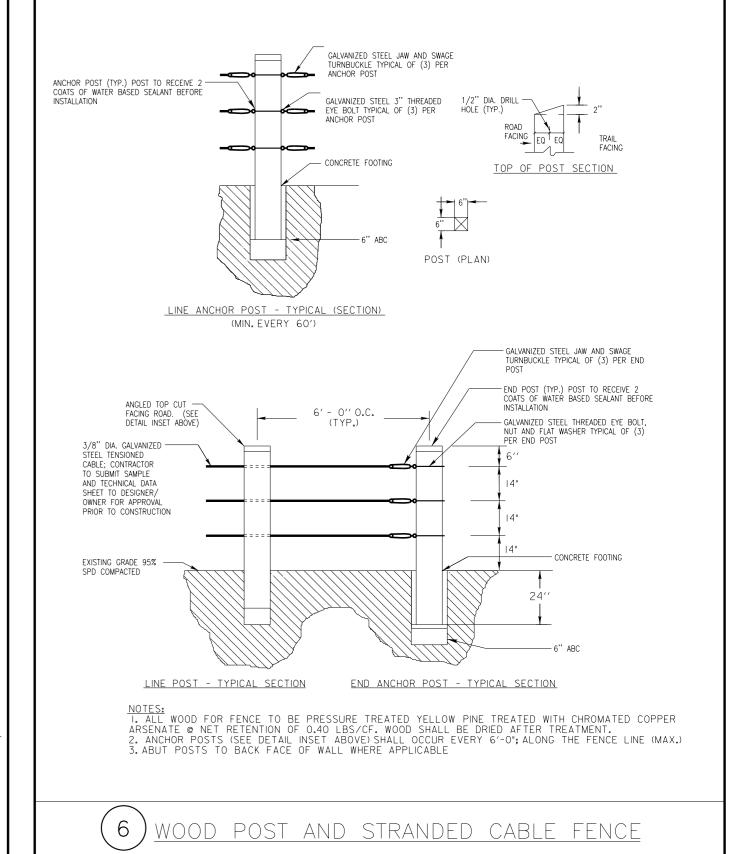


-METAL HAND RAIL (PAID FOR SEPARATELY) -CONCRETE/ASPHALT (PAID FOR SEPARATELY) ٠٠.. -CONC BUTTRESS WALL LENGTH \sim SLOPE TREADS 1/8" \rightarrow - EXPANSION JOINT (TYP) I.TREADS AND RISER DIMENSIONS MAY VARY SLIGHTLY TO ACCOMMODATE FIELD CONDITIONS 2.CONCRETE SHALL BE CLASS A, 3000 PSI AIR ENTRAINED. 3.ONE AND TWO RISER STEPS WILL NOT HAVE BUTTRESS WALL UNLESS DIRECTED BY THE ENGINEER 4. LANDING REQUIRED AT EVERY VERTICAL CHANGE OF 12' (MIN)

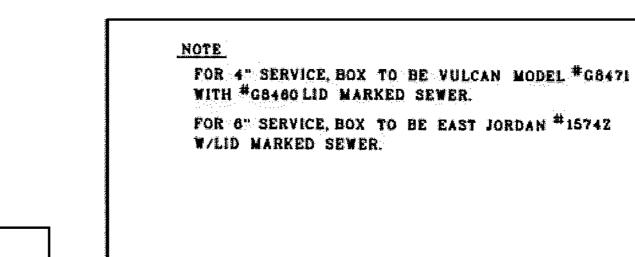
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— BOARDWALK

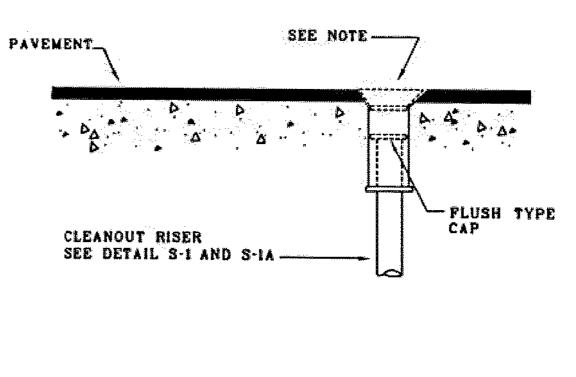
CONCRETE STEPS AND BUTTRESS WALL



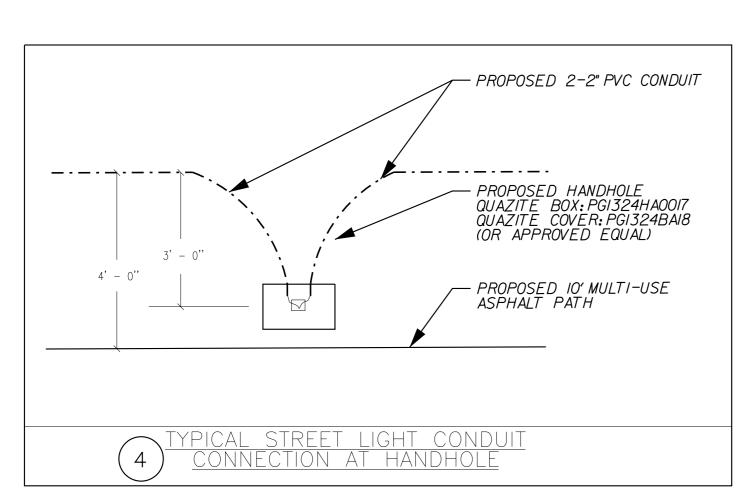
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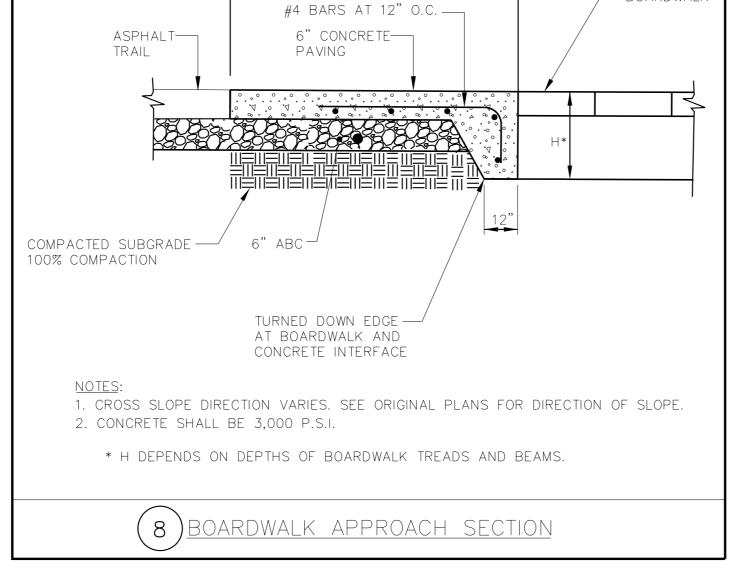
CLEANOUT DETAIL FOR PAVED AREAS GREENVILLE UTILITIES COMMISSION (GUC)



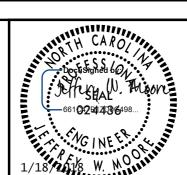
CLEANOUT DETAIL FOR PAVED AREAS		GREENVILLE UTILITIES		NO.	DATE	DESCRIPTION	
		GREENVILLE, N.C.		:		A THE STATE OF THE	
Sheet	DWG. NO. S-3	DWG N. WOOTEN	WATER RESOURCES ENGINEERING				
1 OF 1	DATE 3-12-99	APP H. COREY	GREENVILLE, N.C.	Greenville Utilities	: 		



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CONSTRUCTION DETAILS



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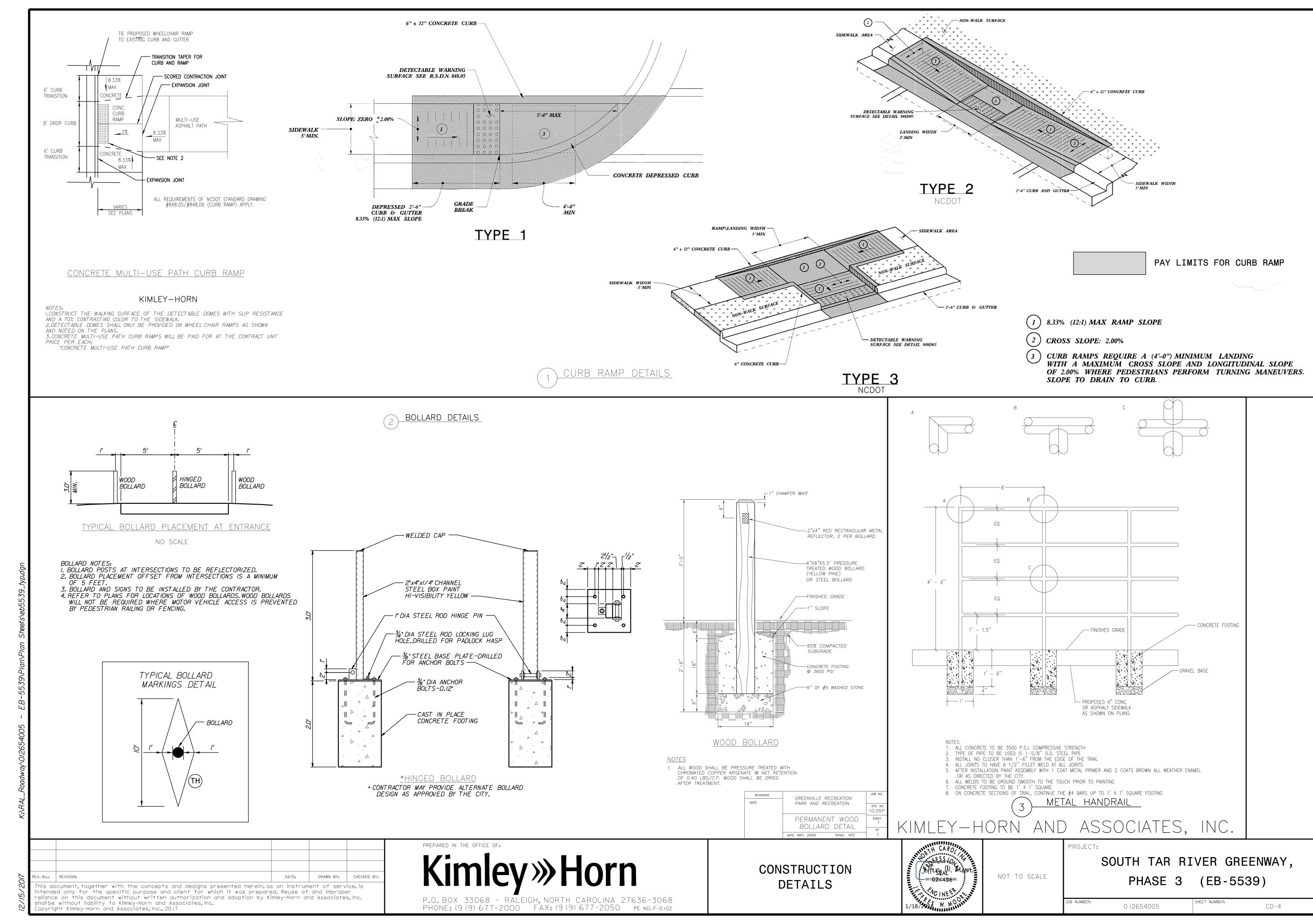
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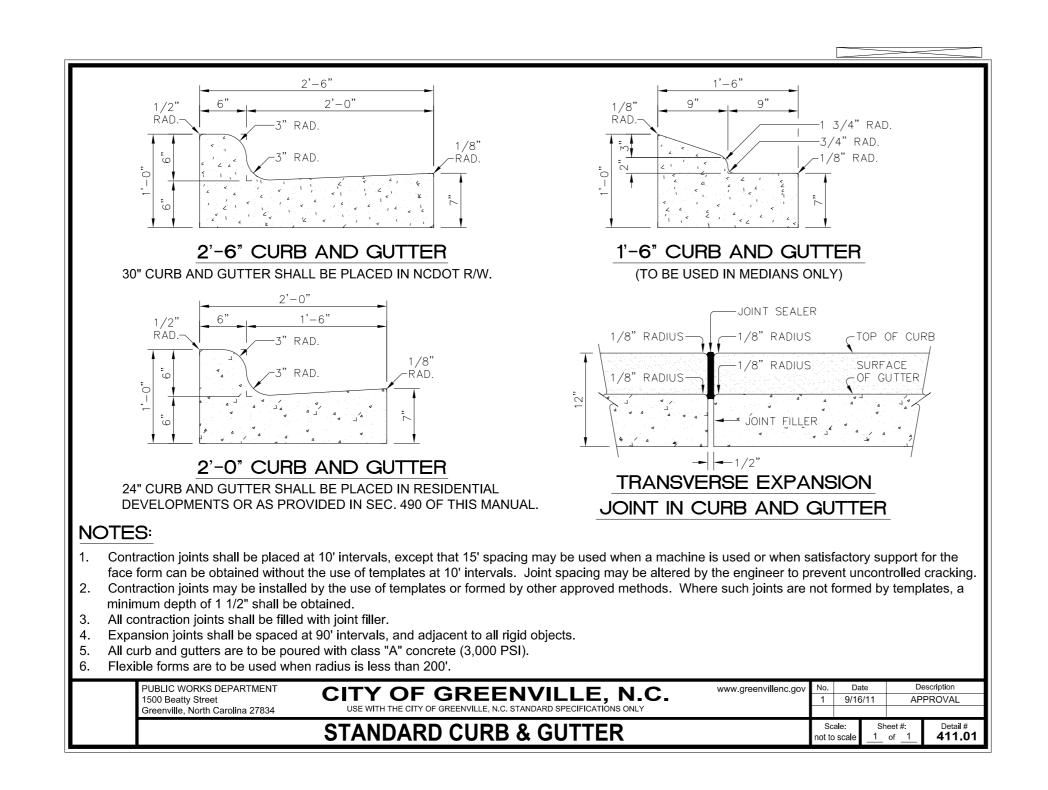
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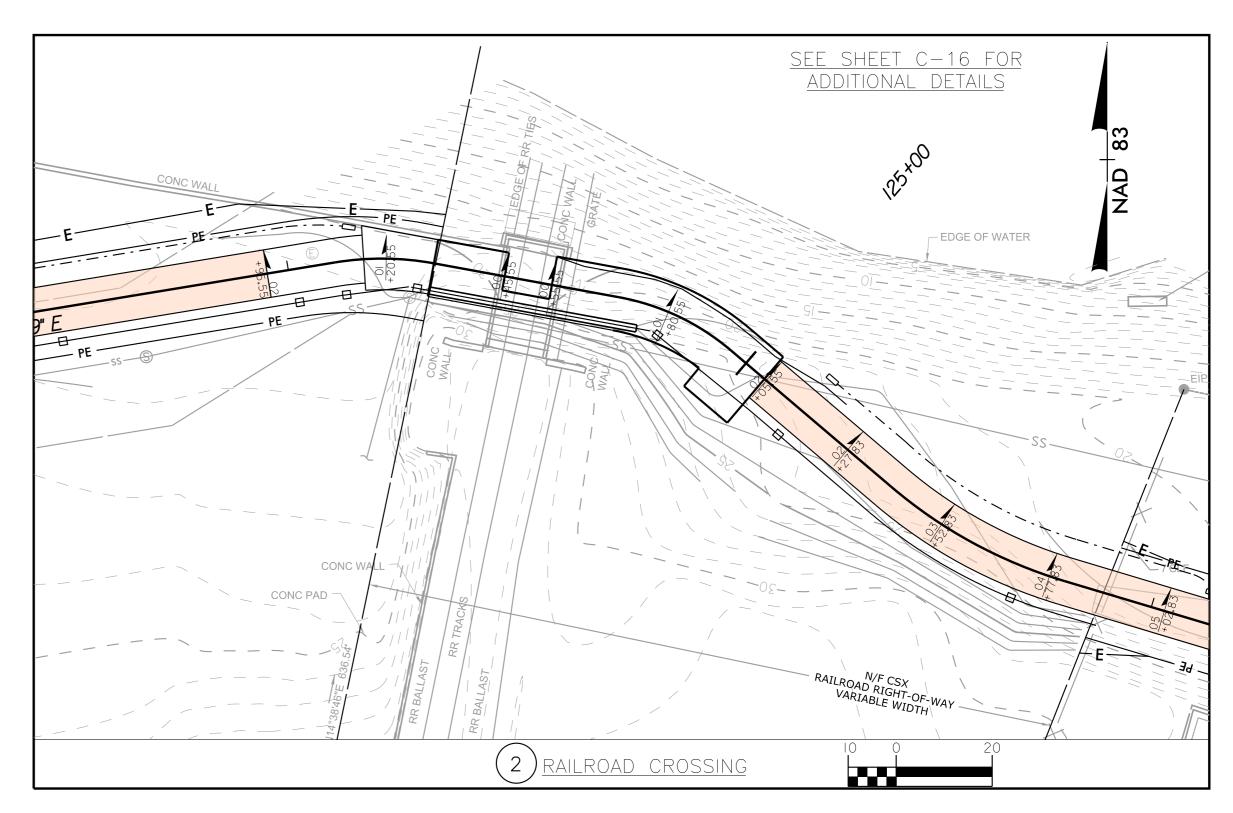
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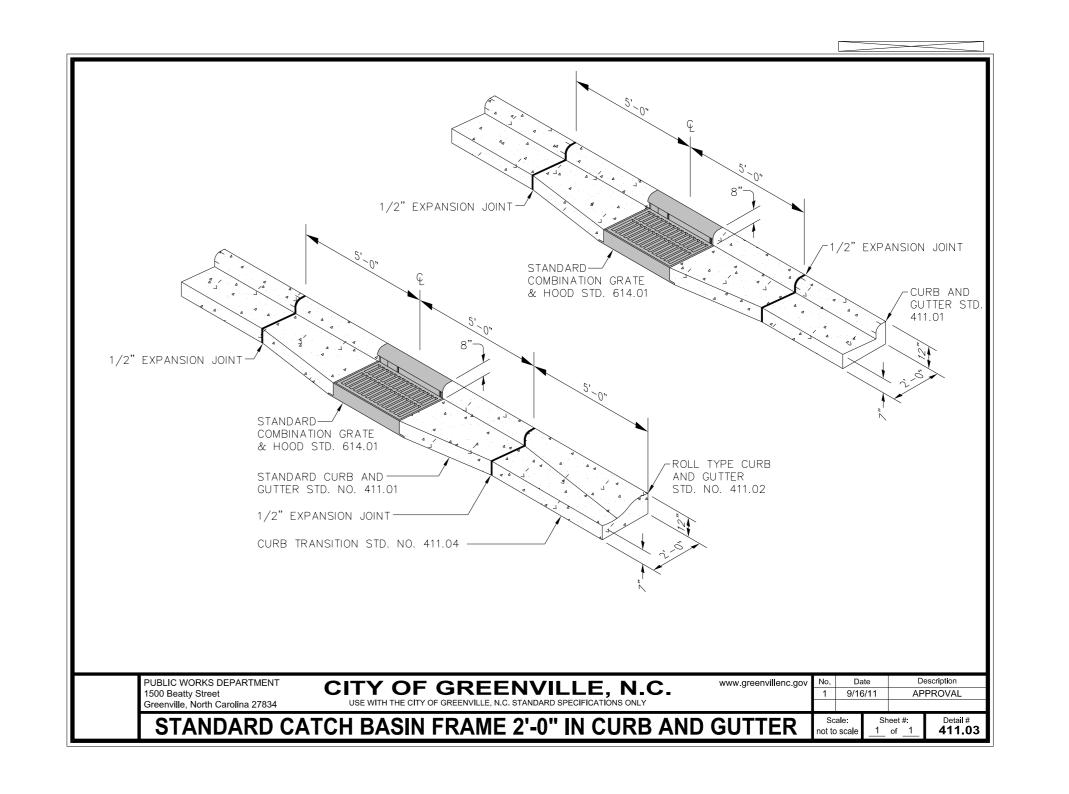








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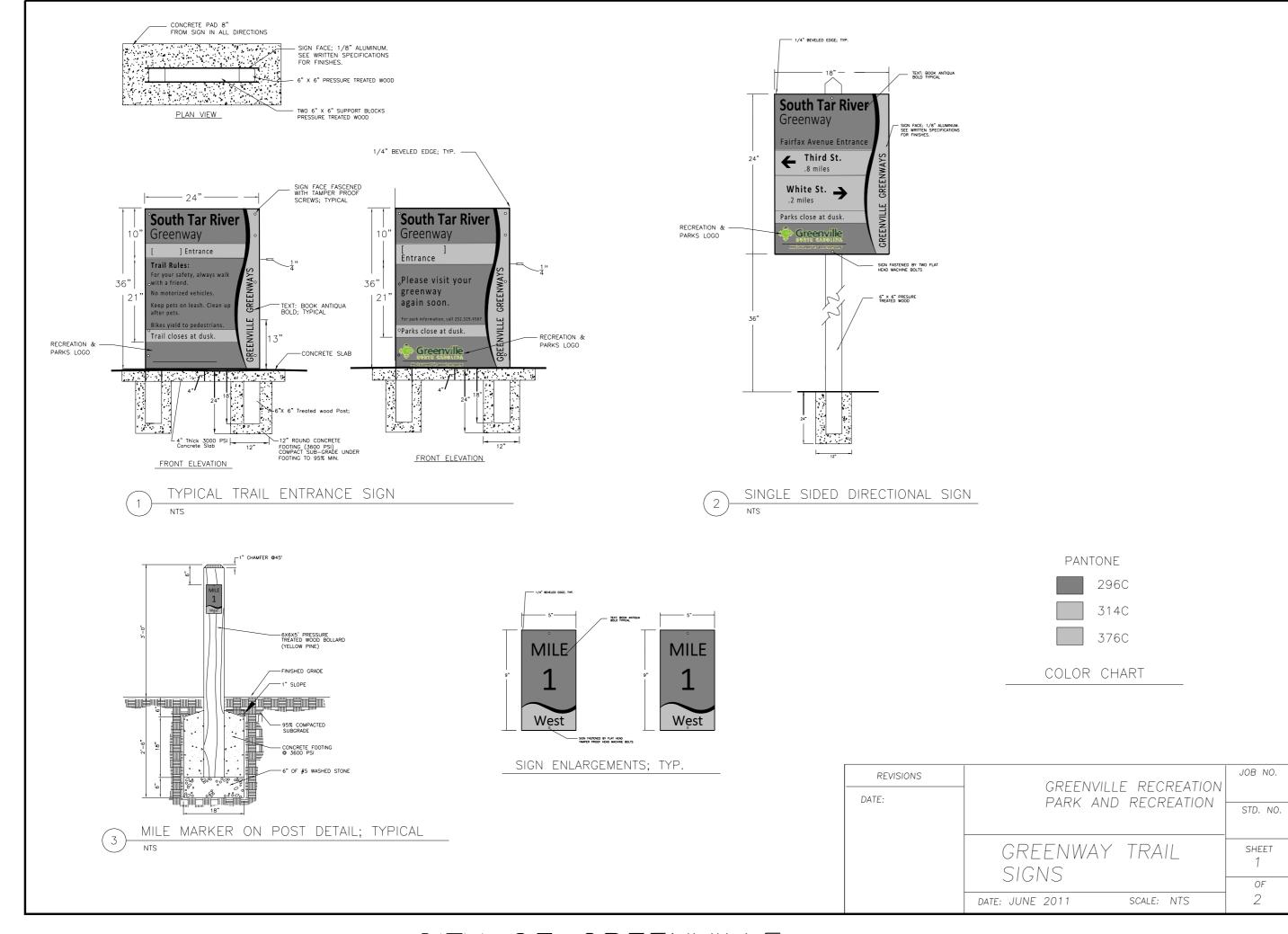
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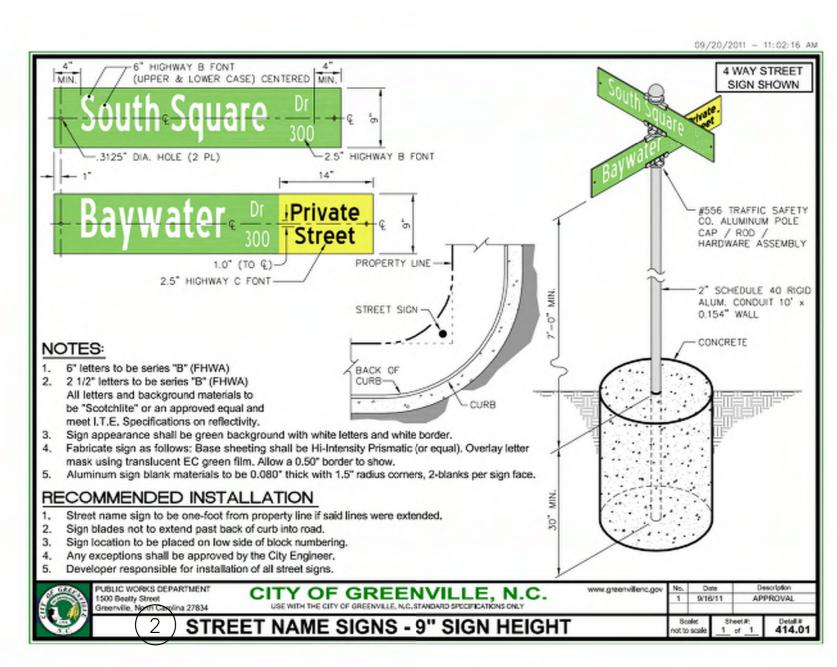
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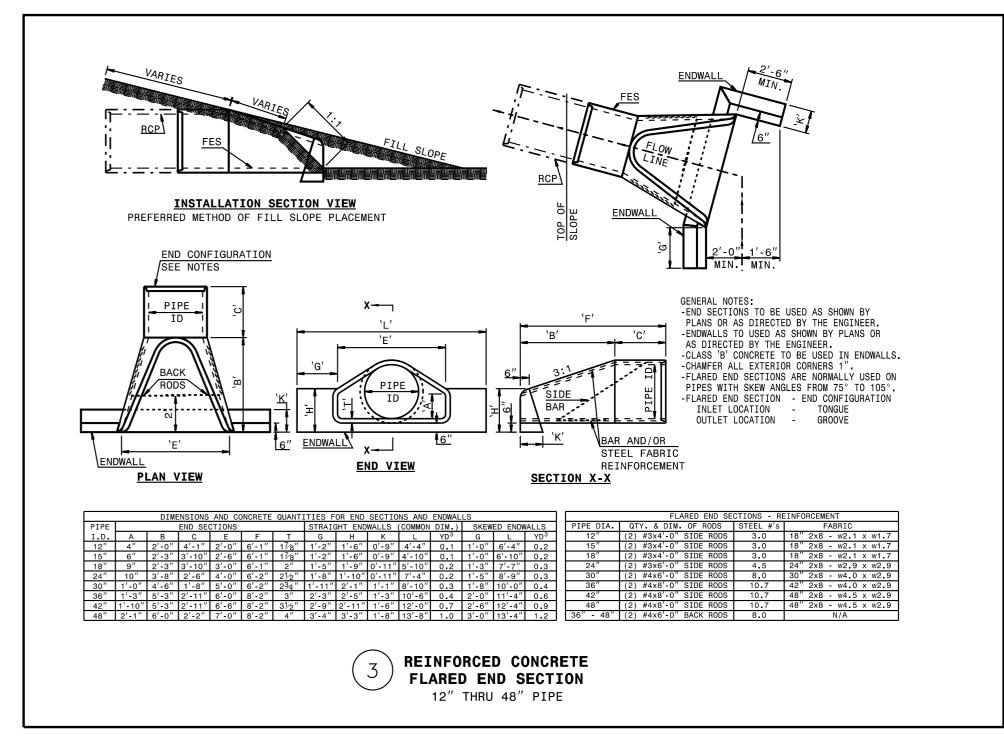
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CITY OF GREENVILLE

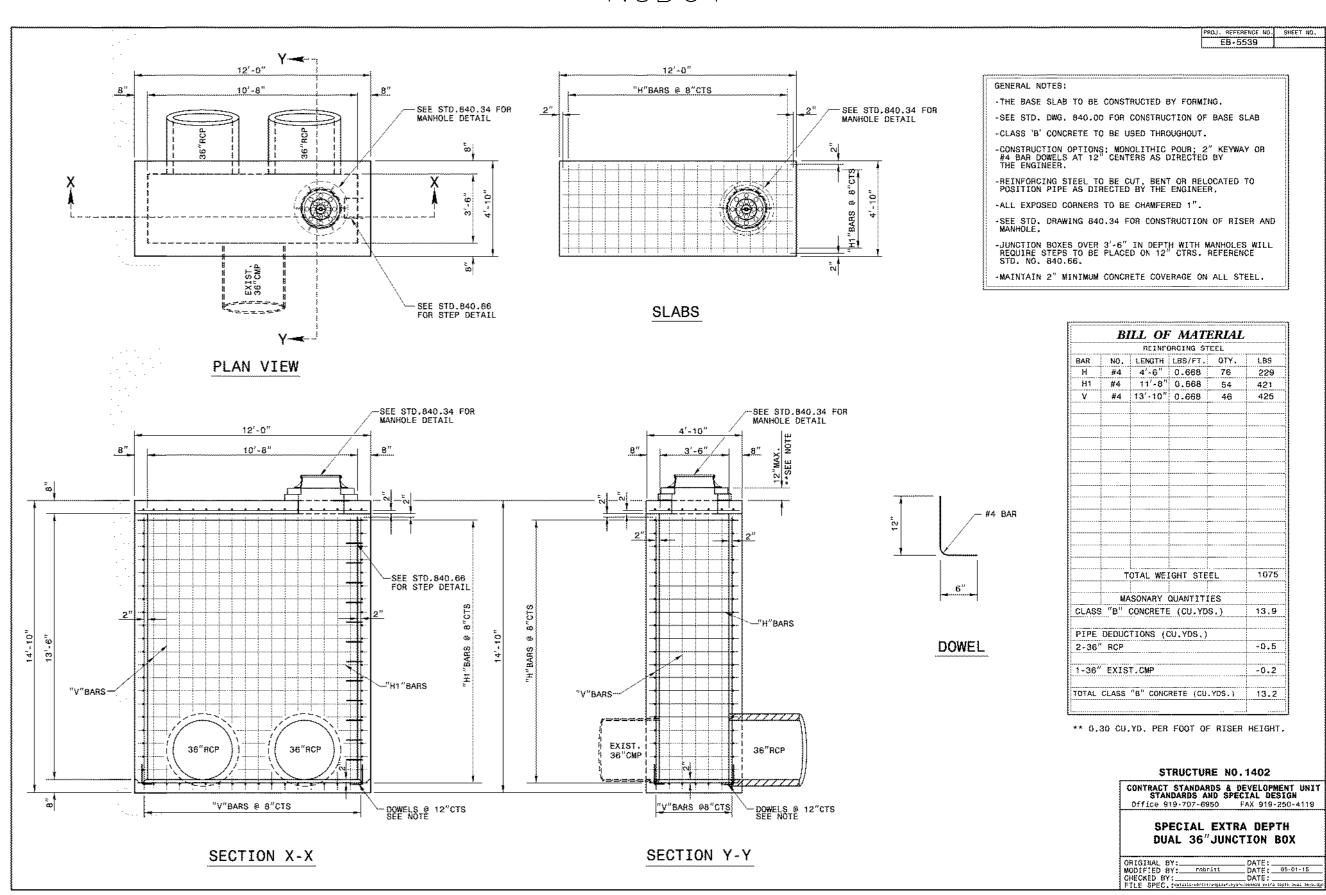


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NCDOT



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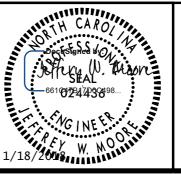
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CONSTRUCTION **DETAILS**

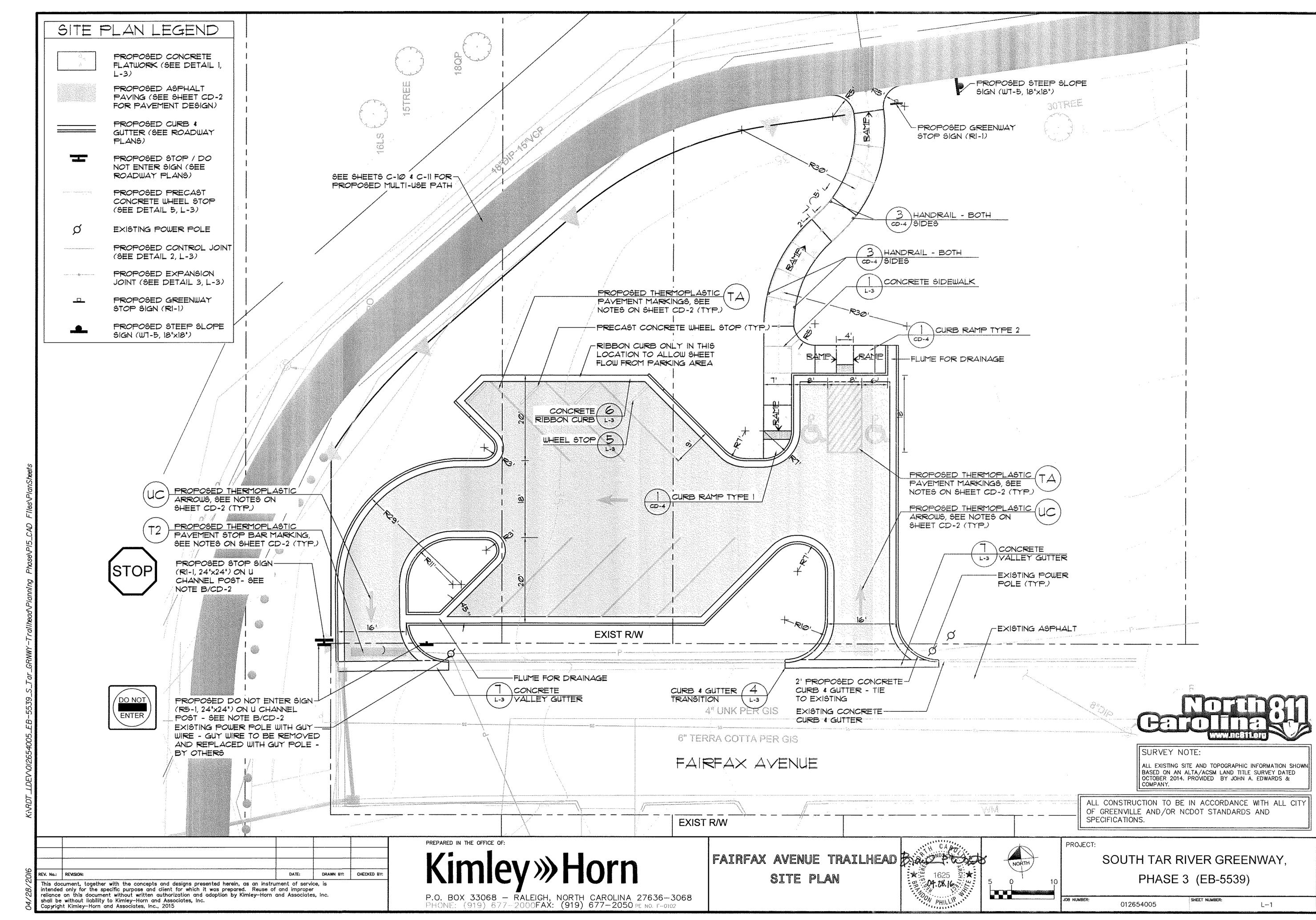


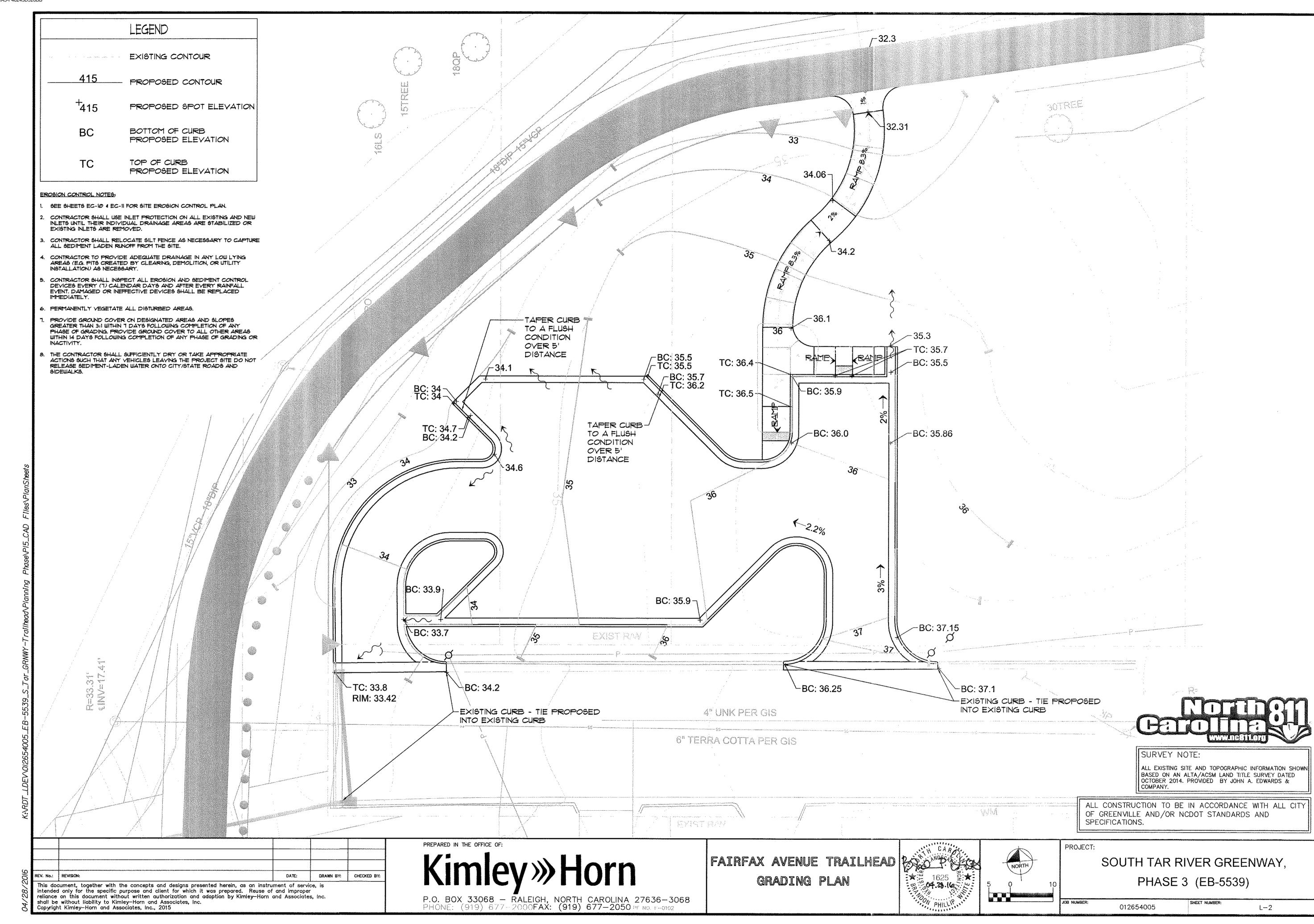
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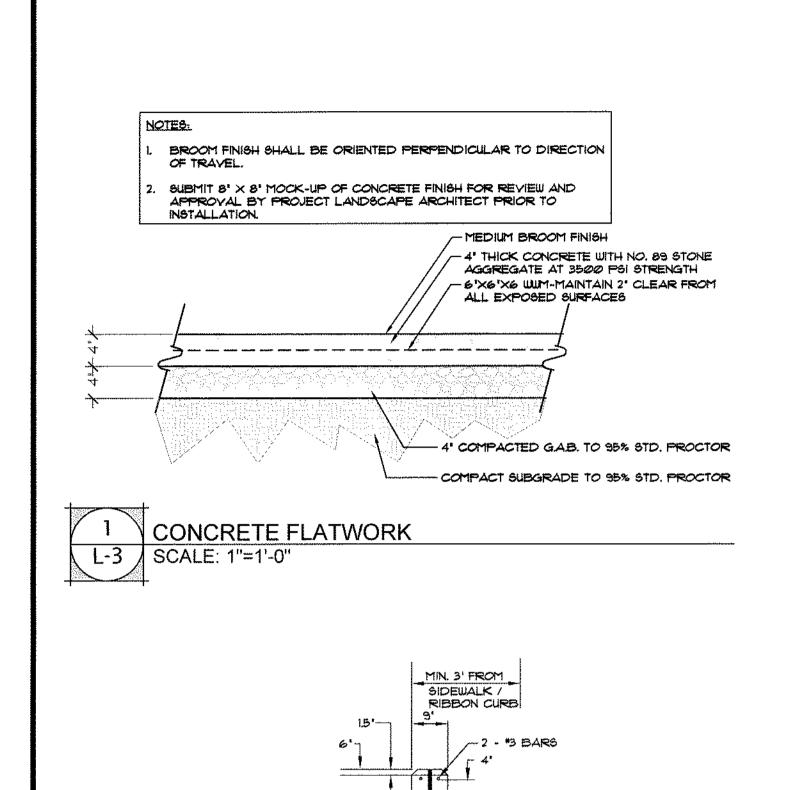
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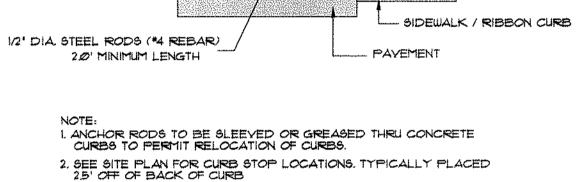
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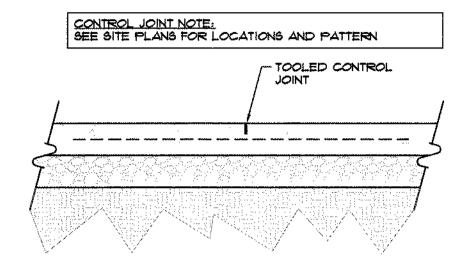




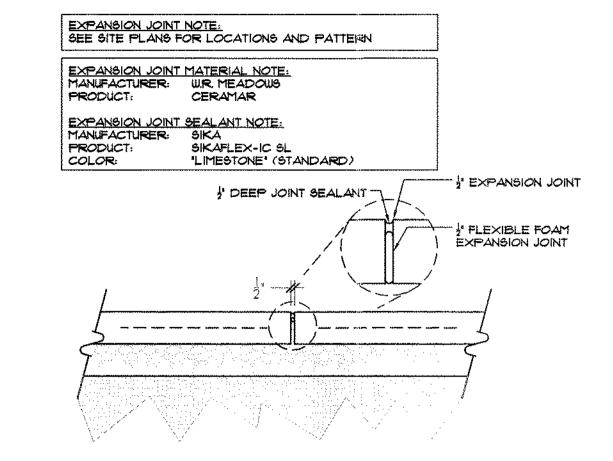




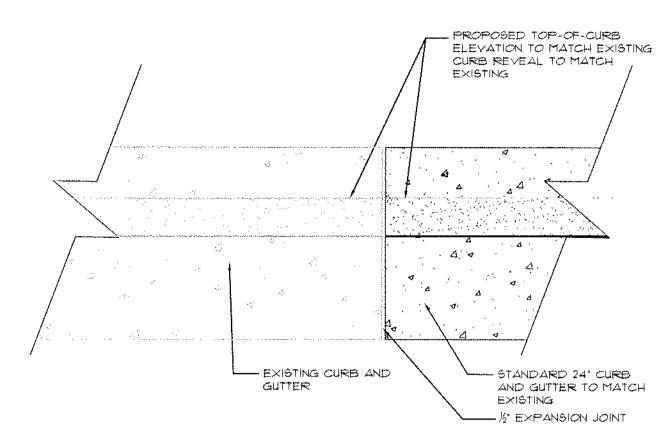




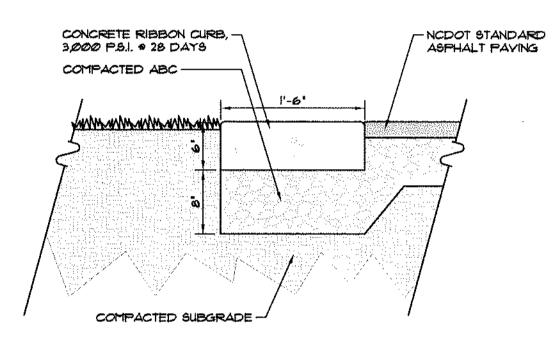


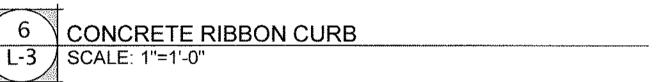


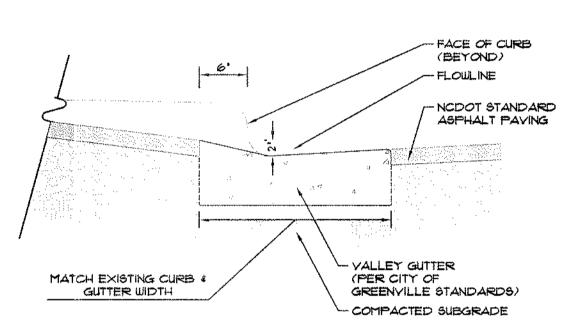


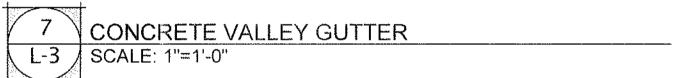














SURVEY NOTE:

ALL EXISTING SITE AND TOPOGRAPHIC INFORMATION SHOWN BASED ON AN ALTA/ACSM LAND TITLE SURVEY DATED OCTOBER 2014. PRÓVIDED BY JOHN A. EDWARDS & COMPANY.

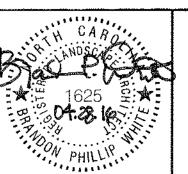
ALL CONSTRUCTION TO BE IN ACCORDANCE WITH ALL CITY OF GREENVILLE AND/OR NCDOT STANDARDS AND SPECIFICATIONS.

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SITE DETAILS

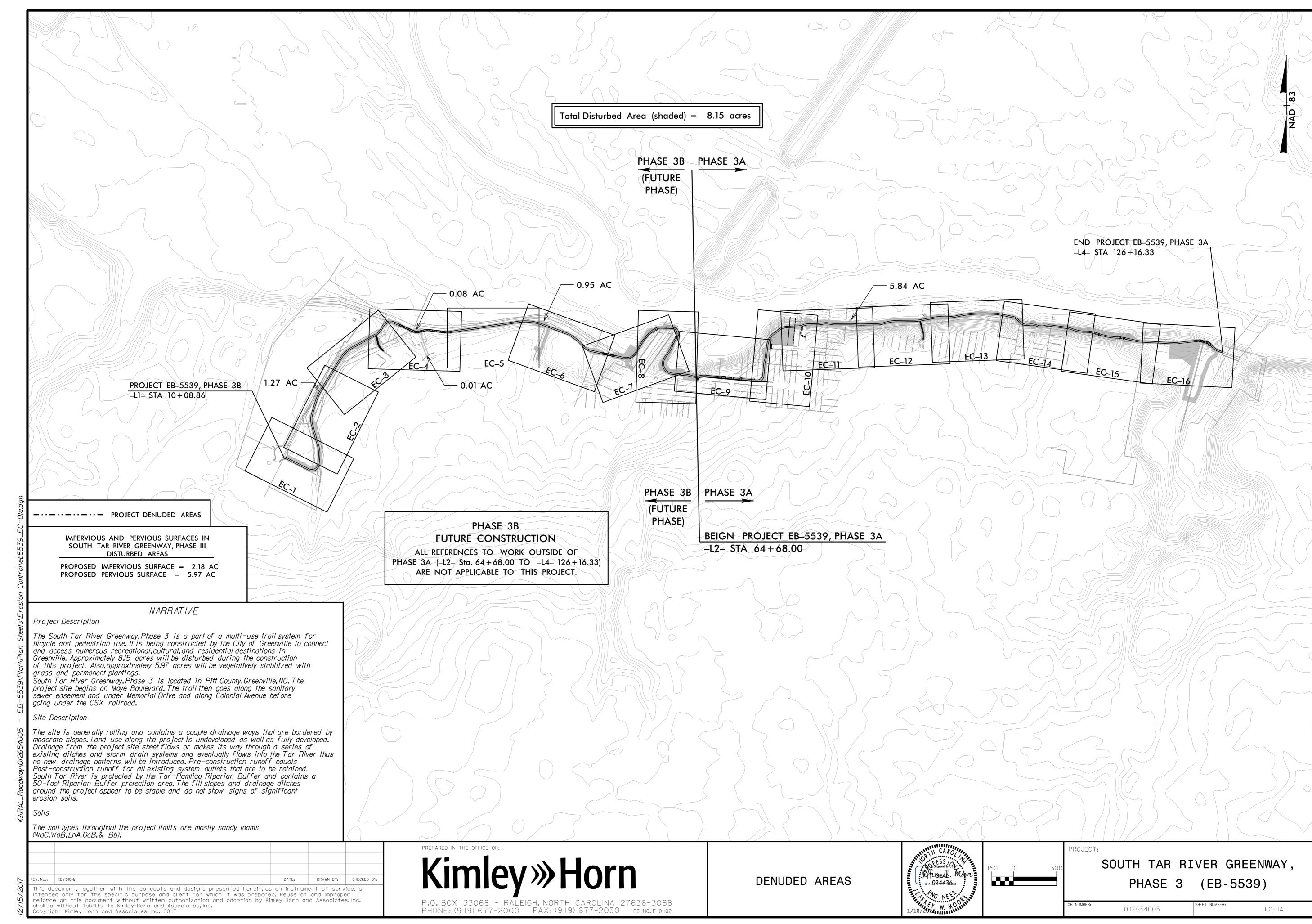


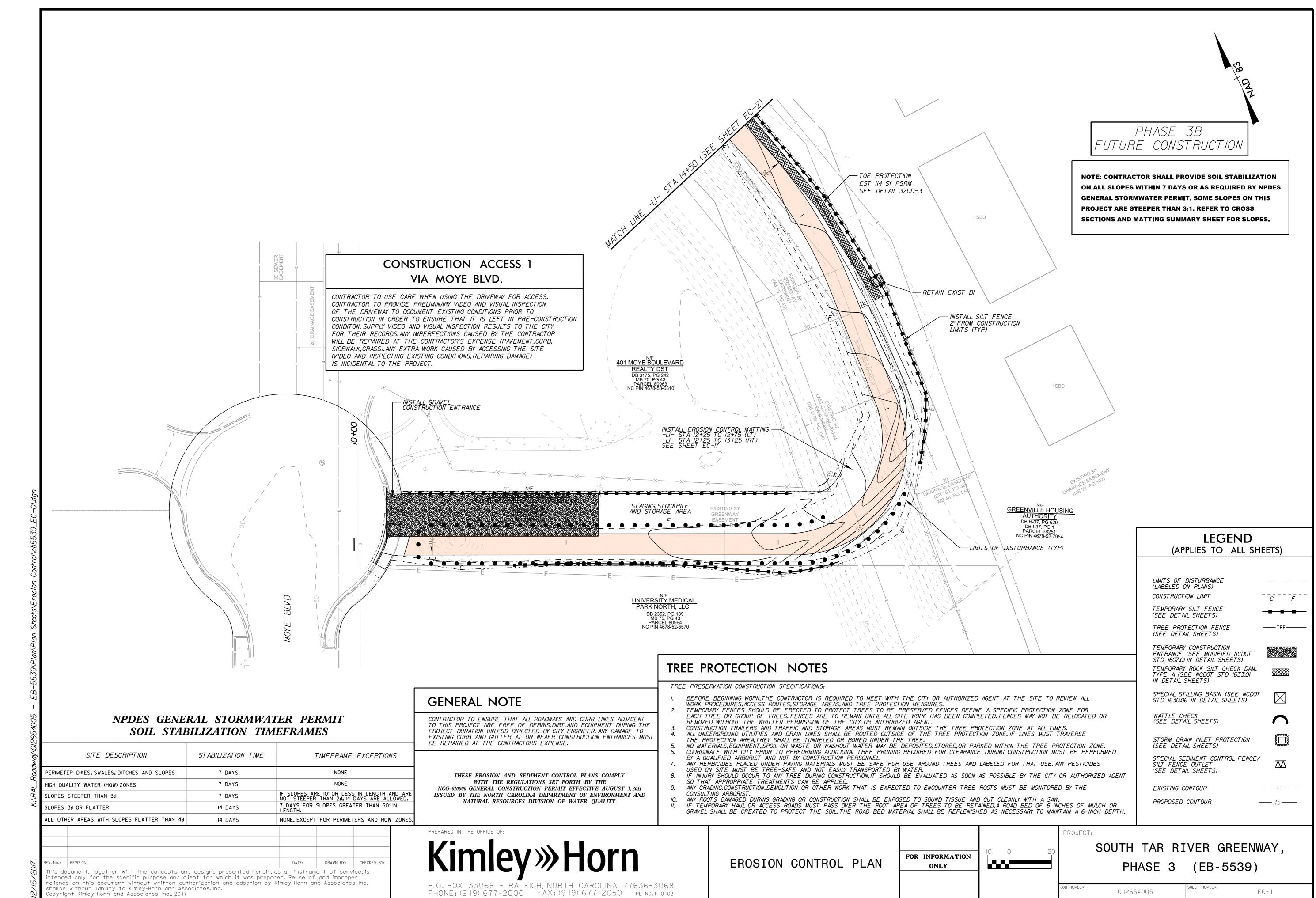
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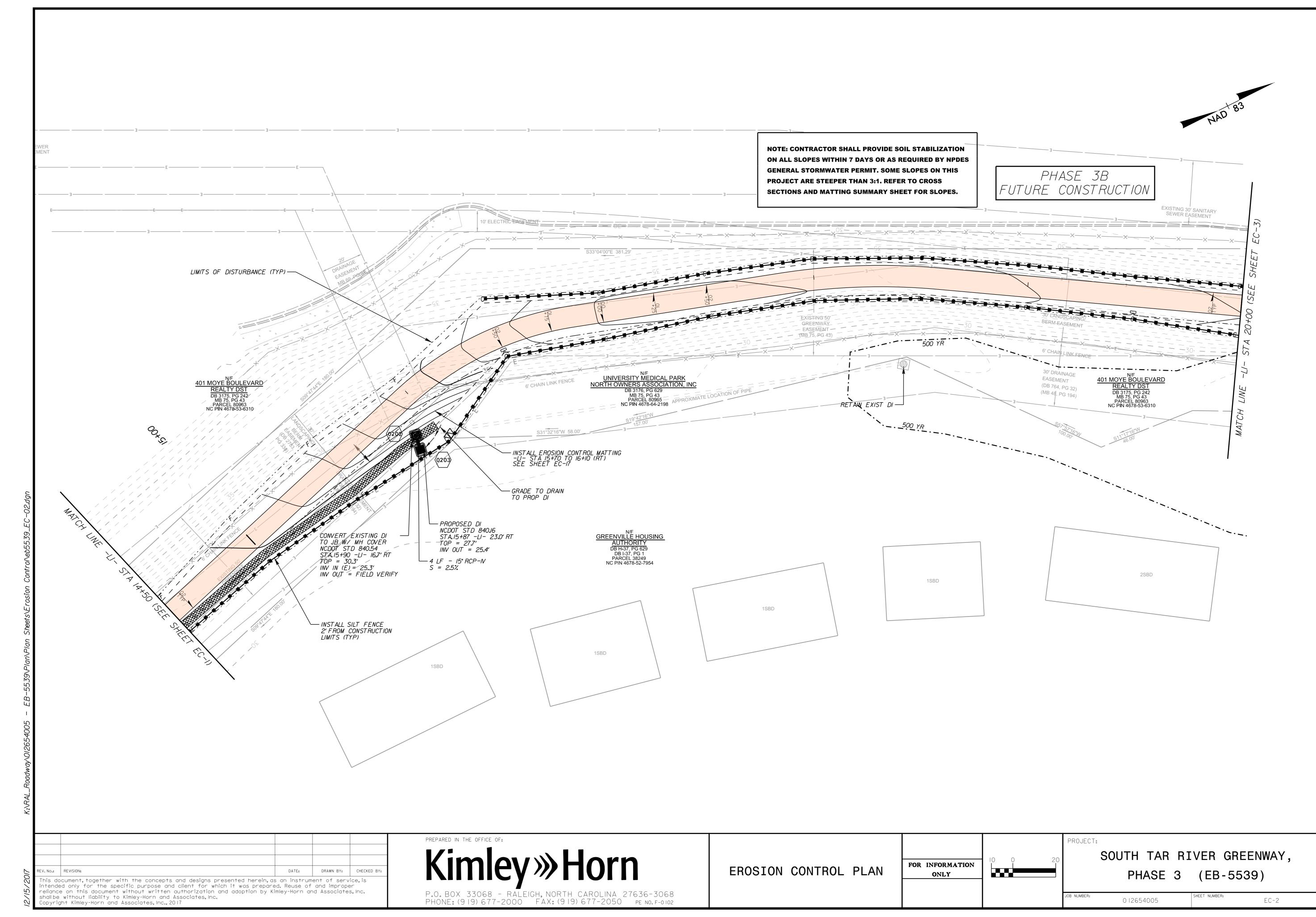
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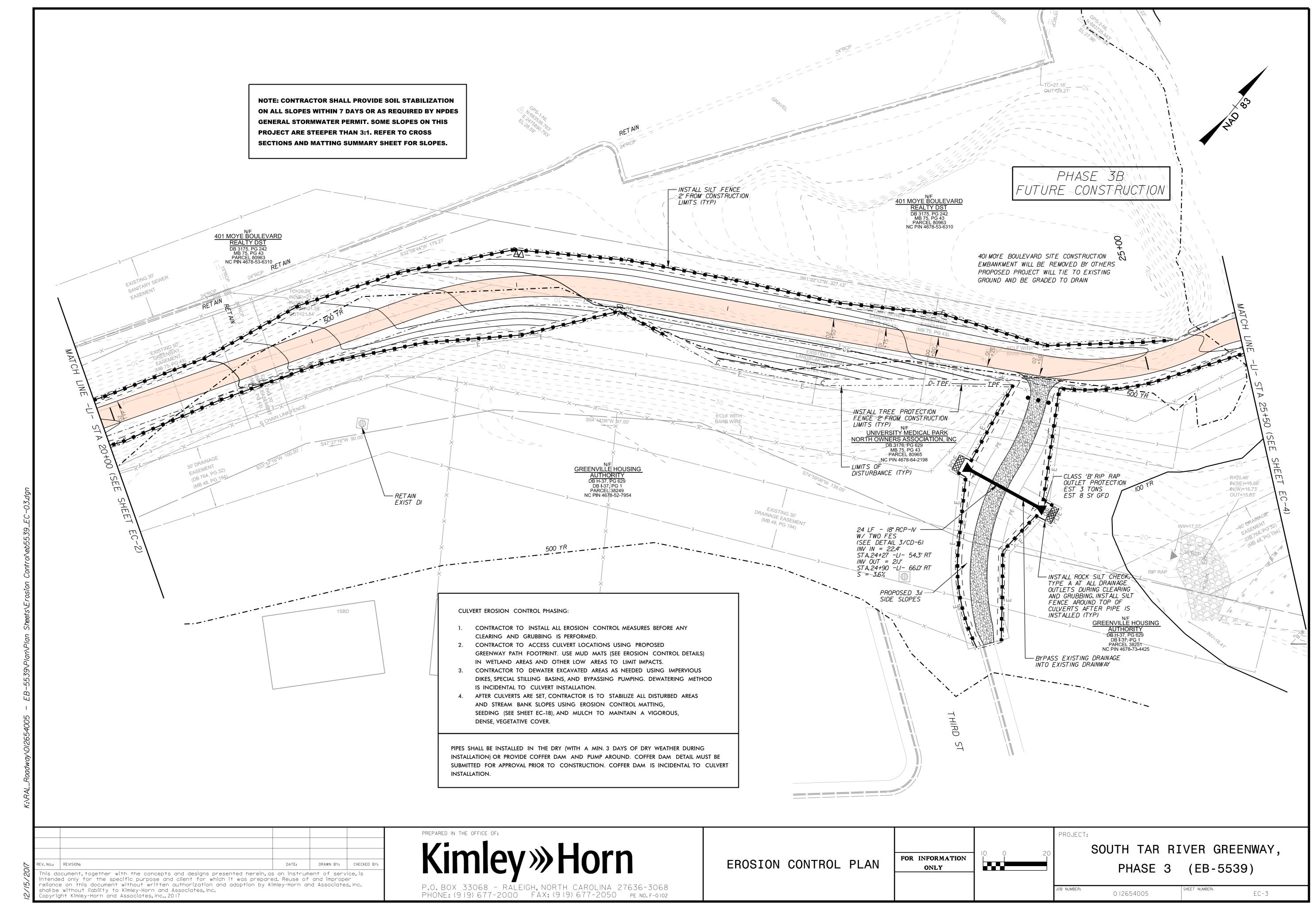
SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

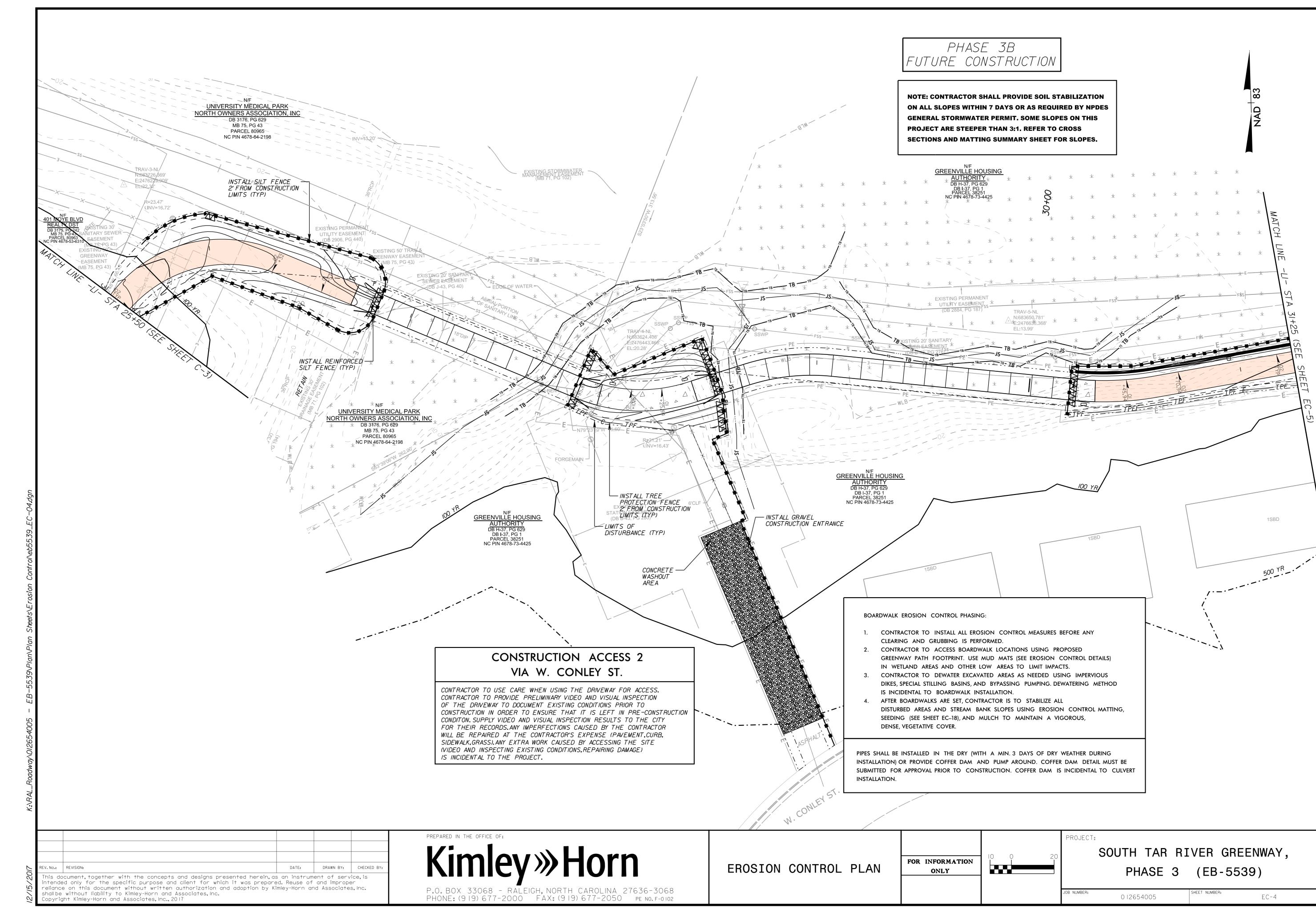
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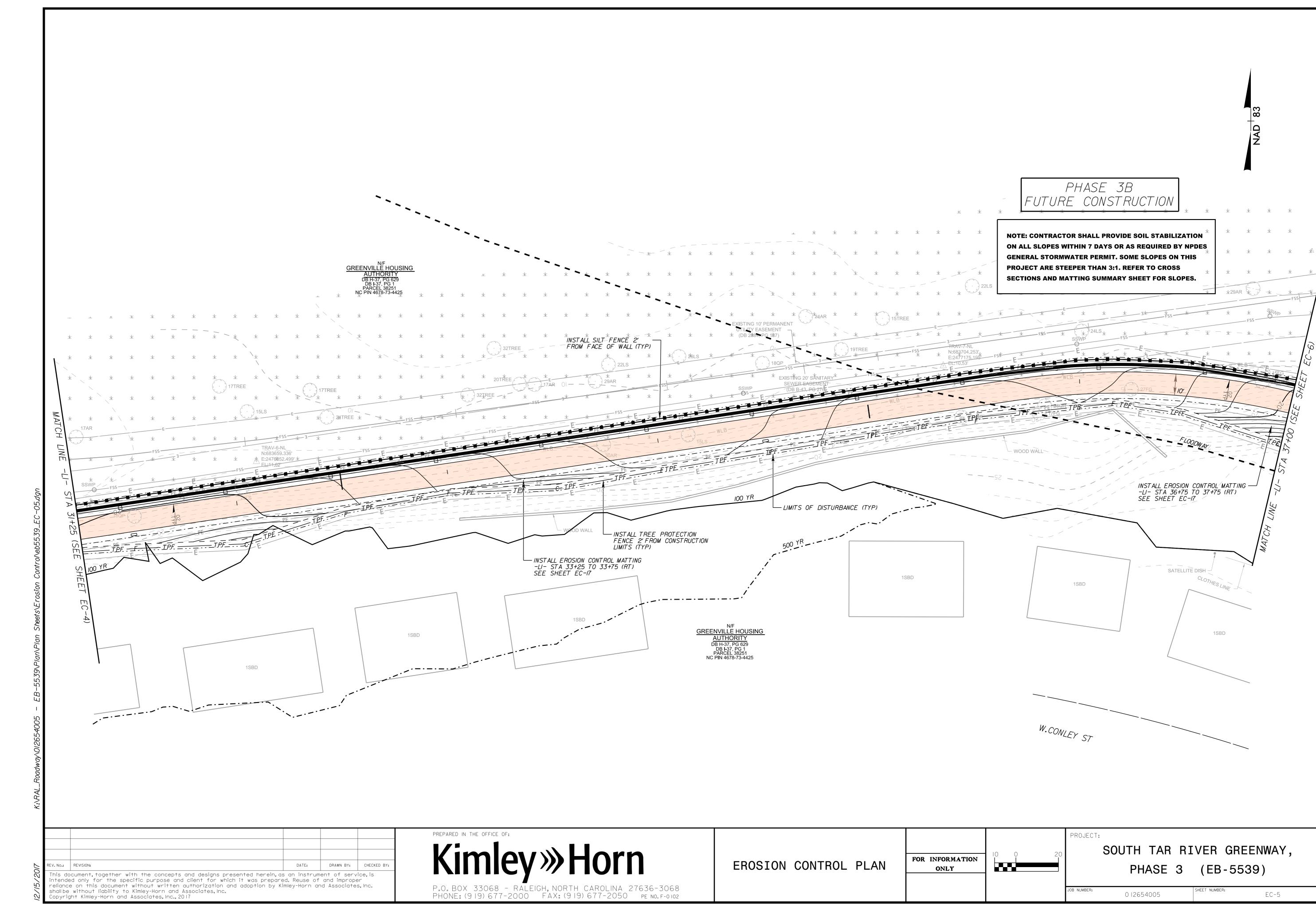


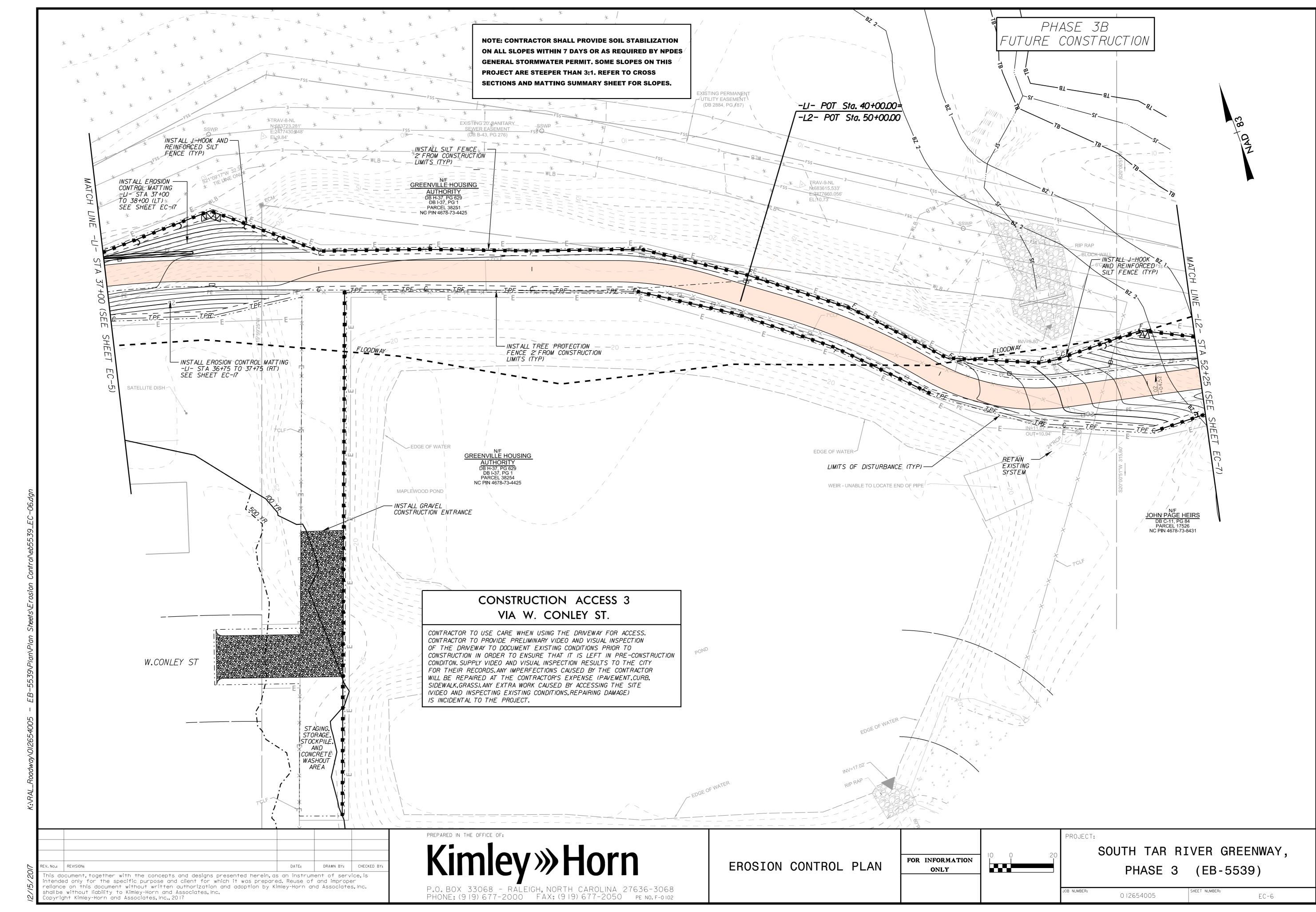


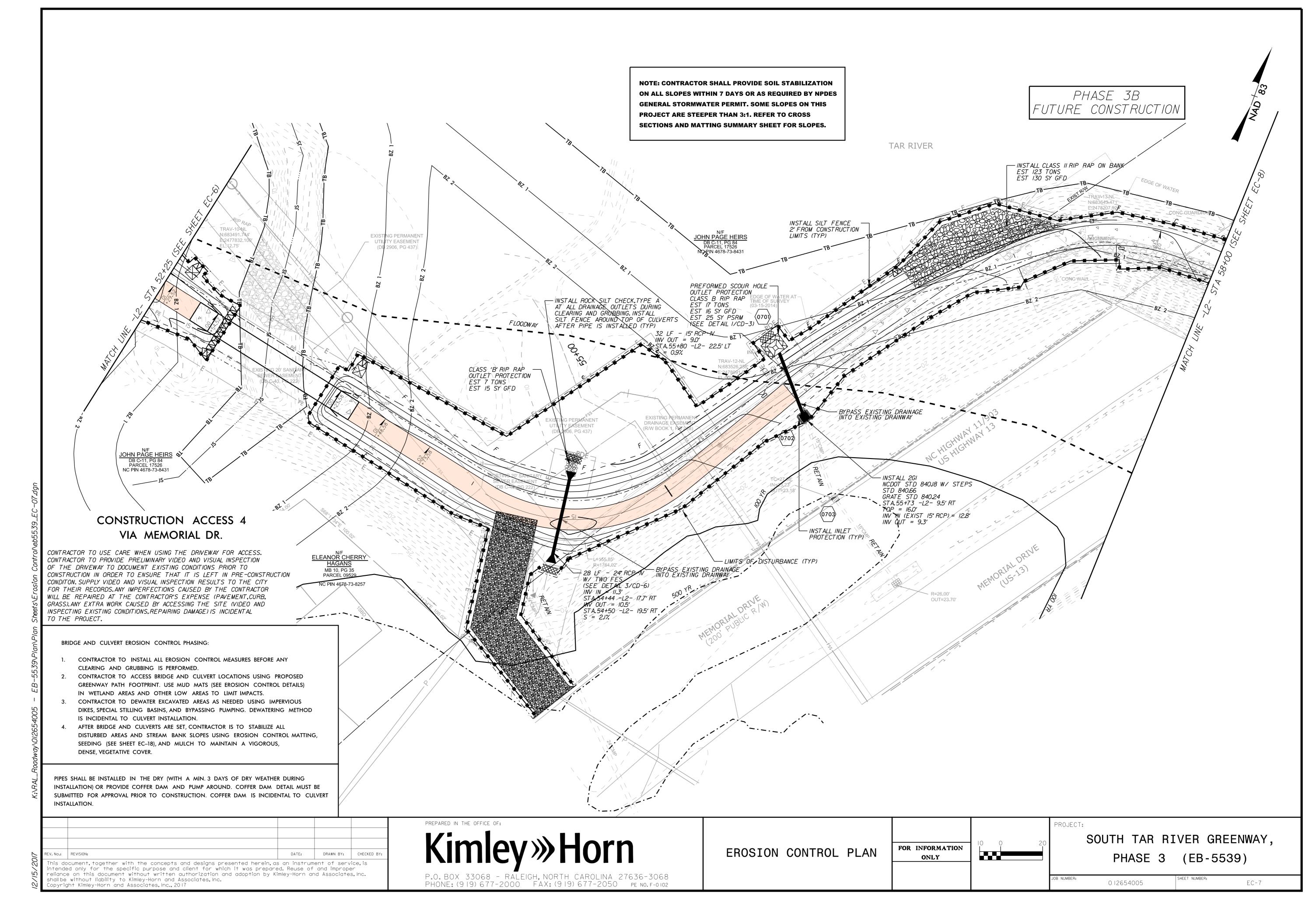


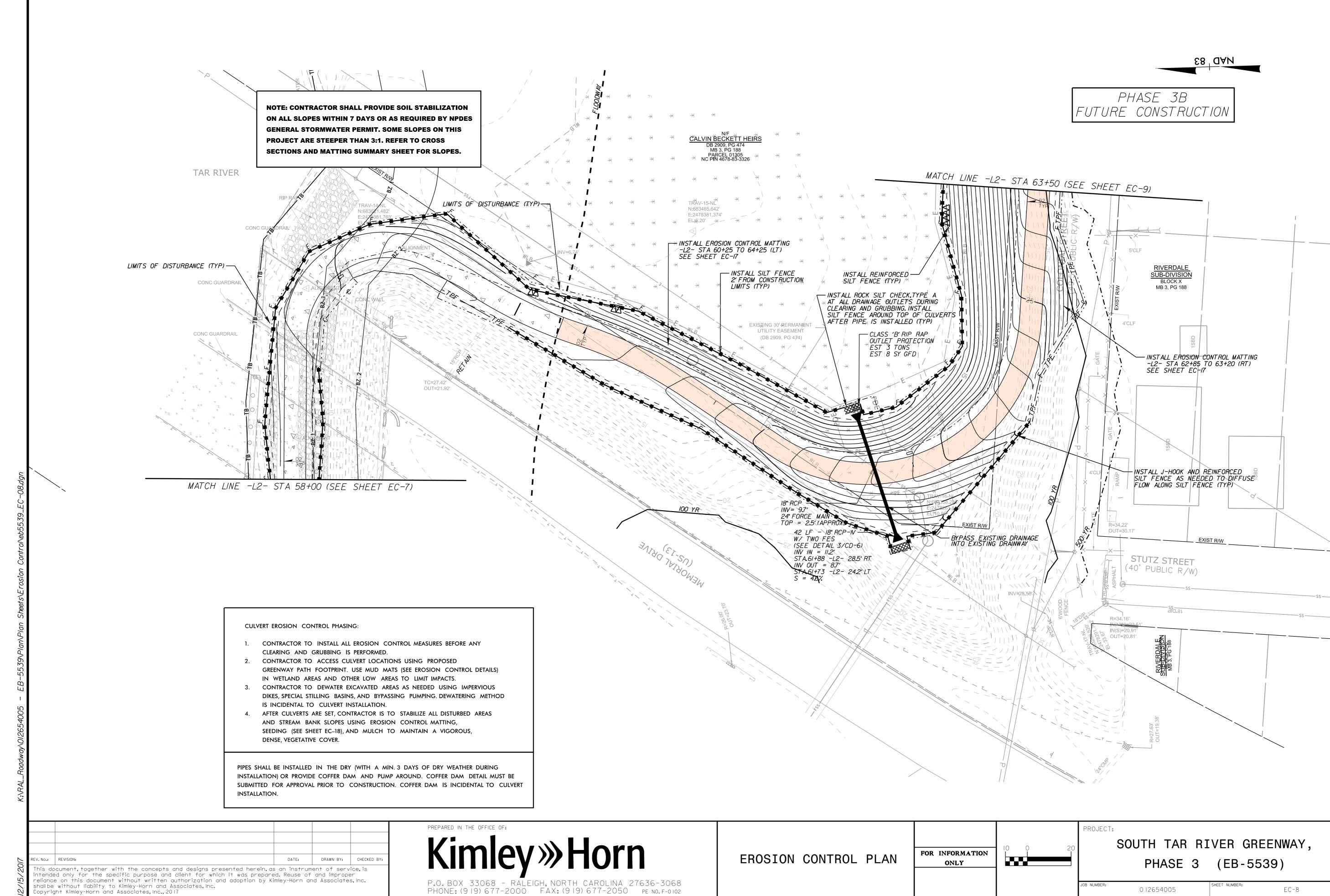








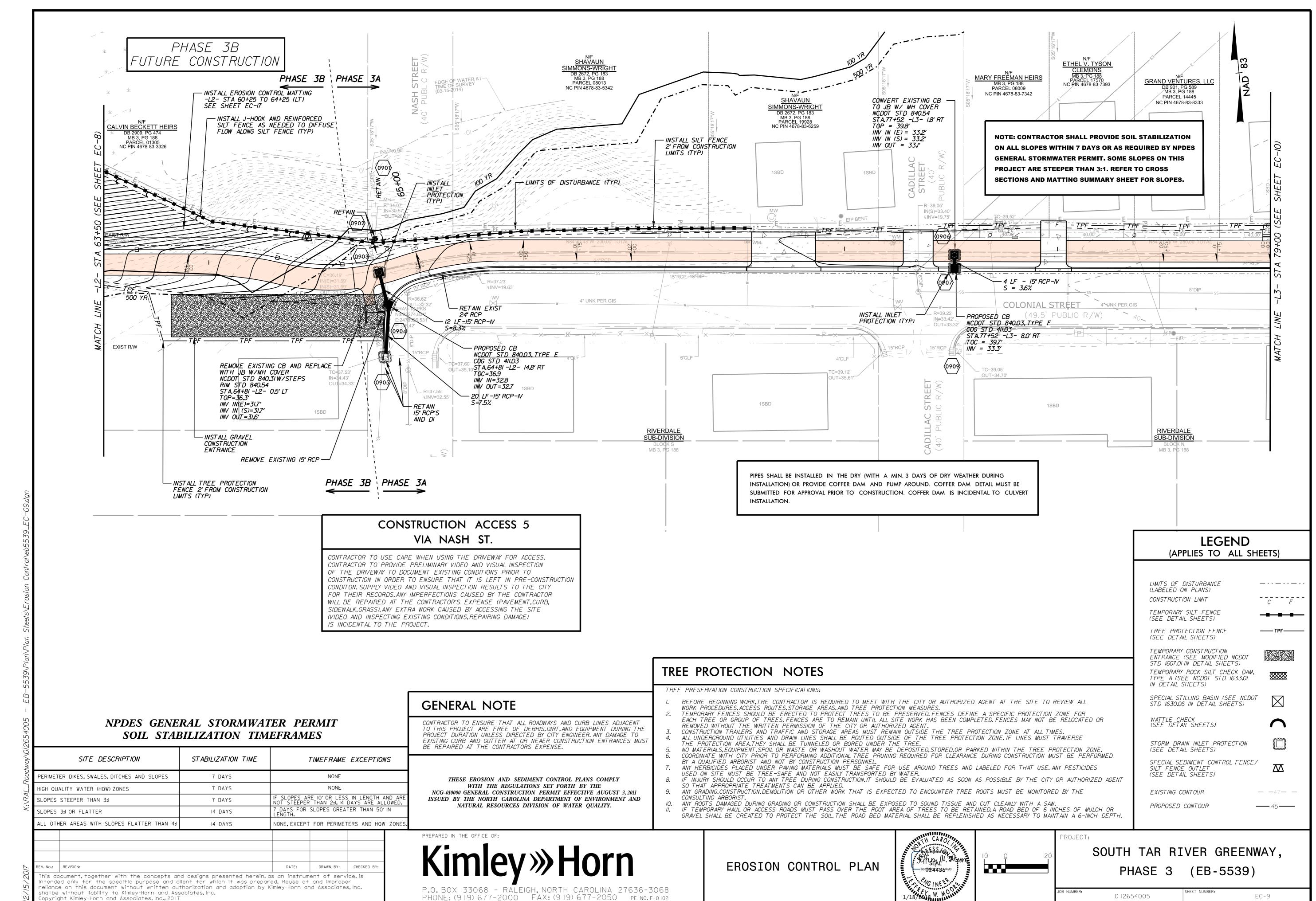




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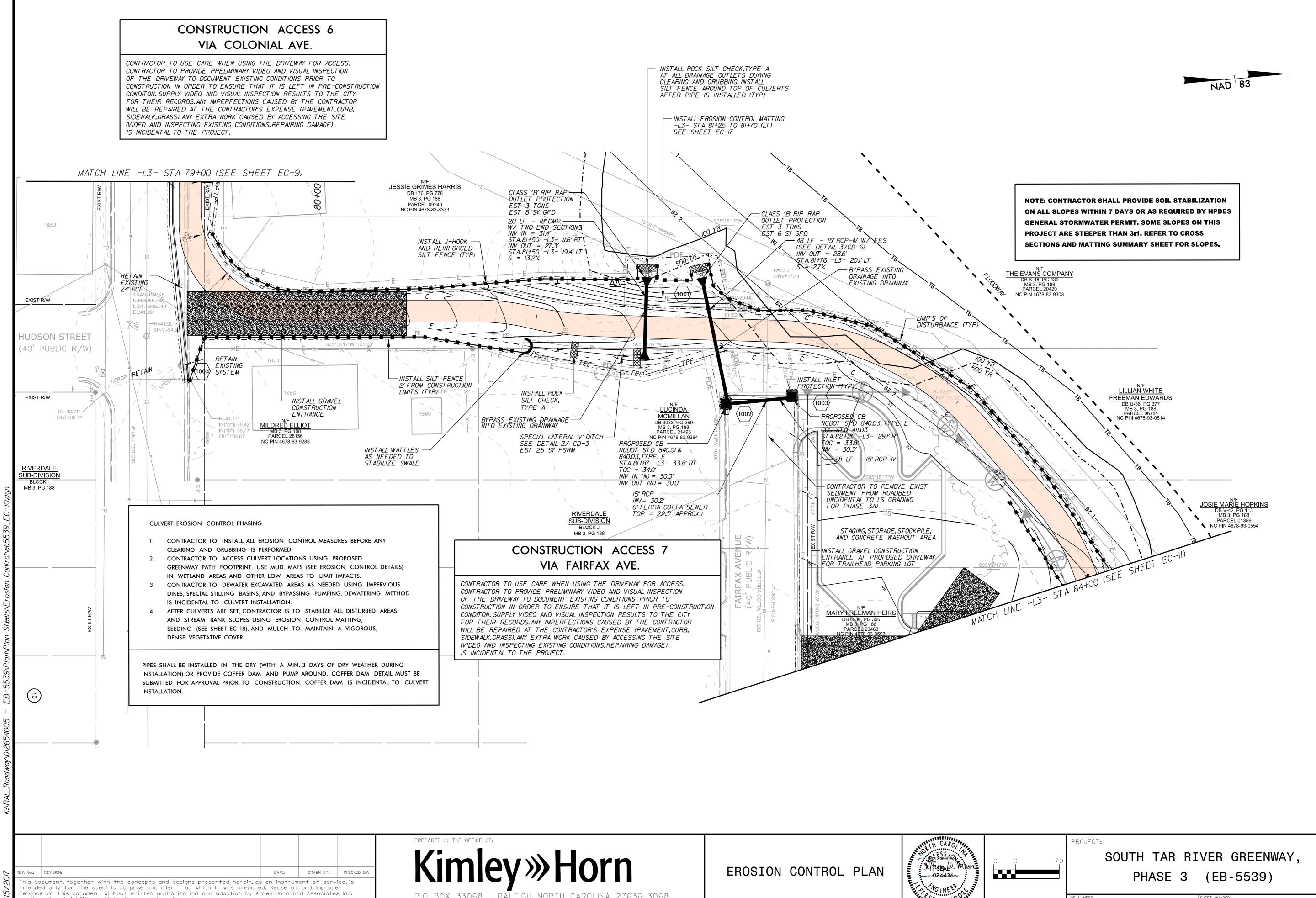
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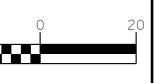
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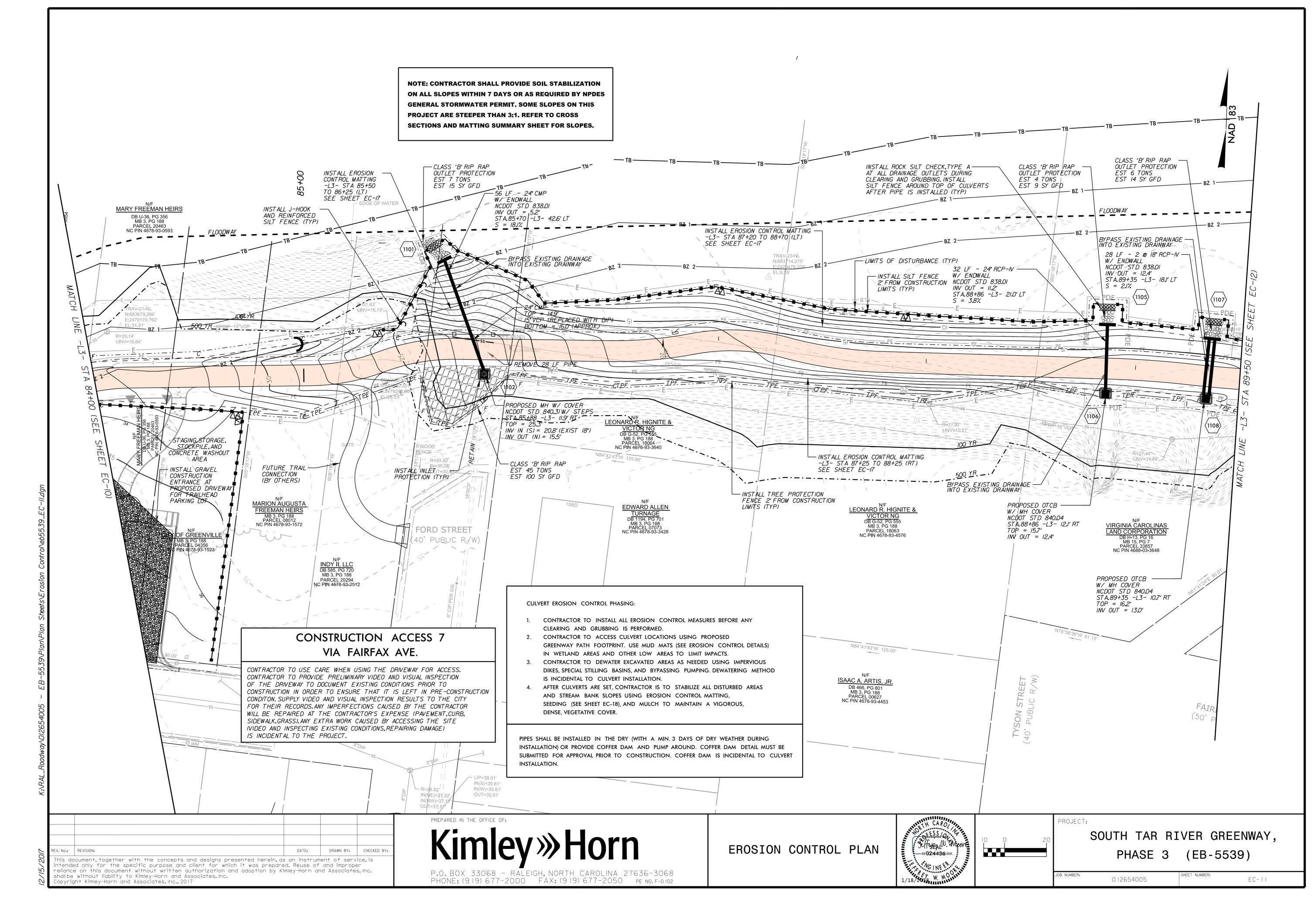
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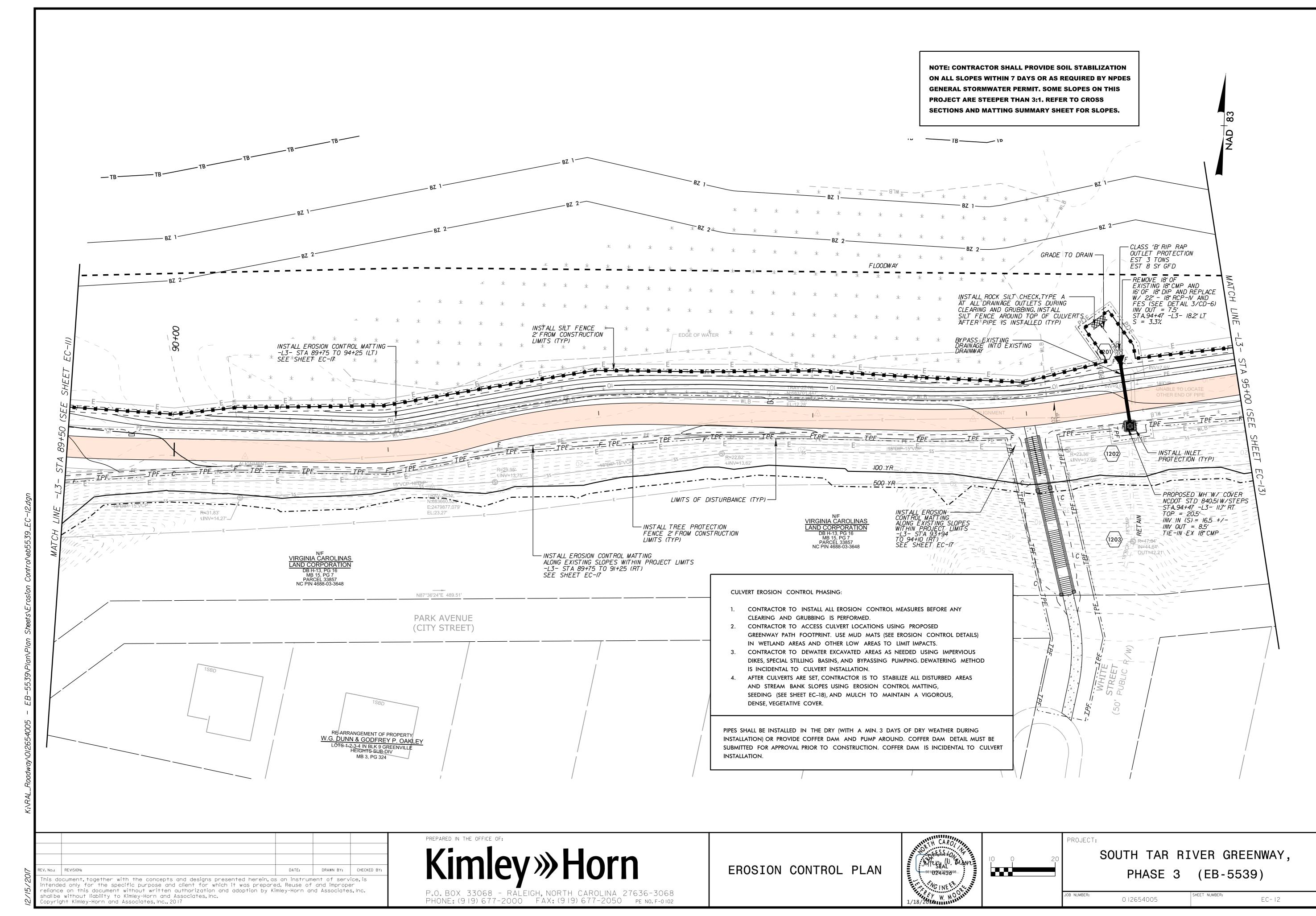


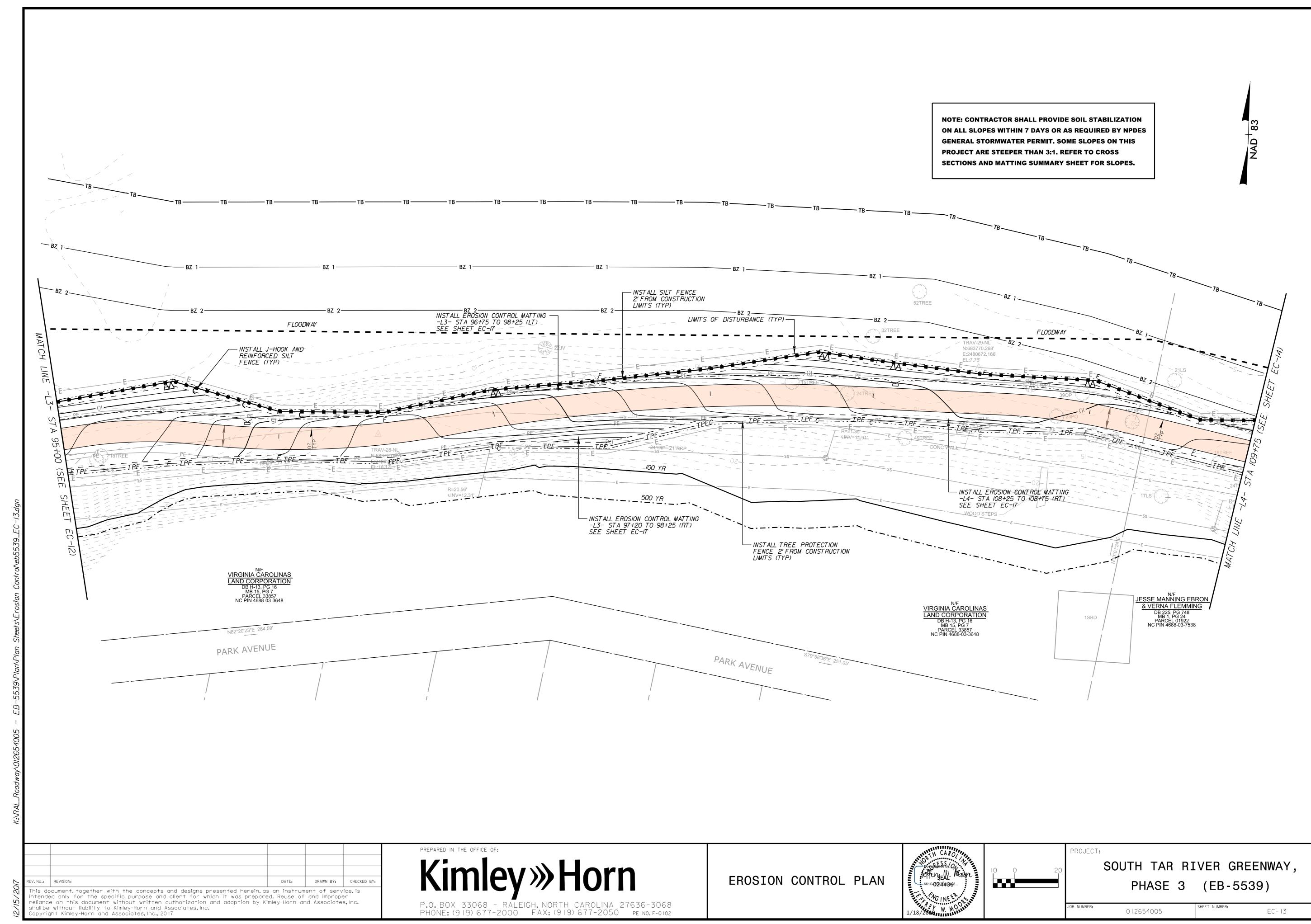


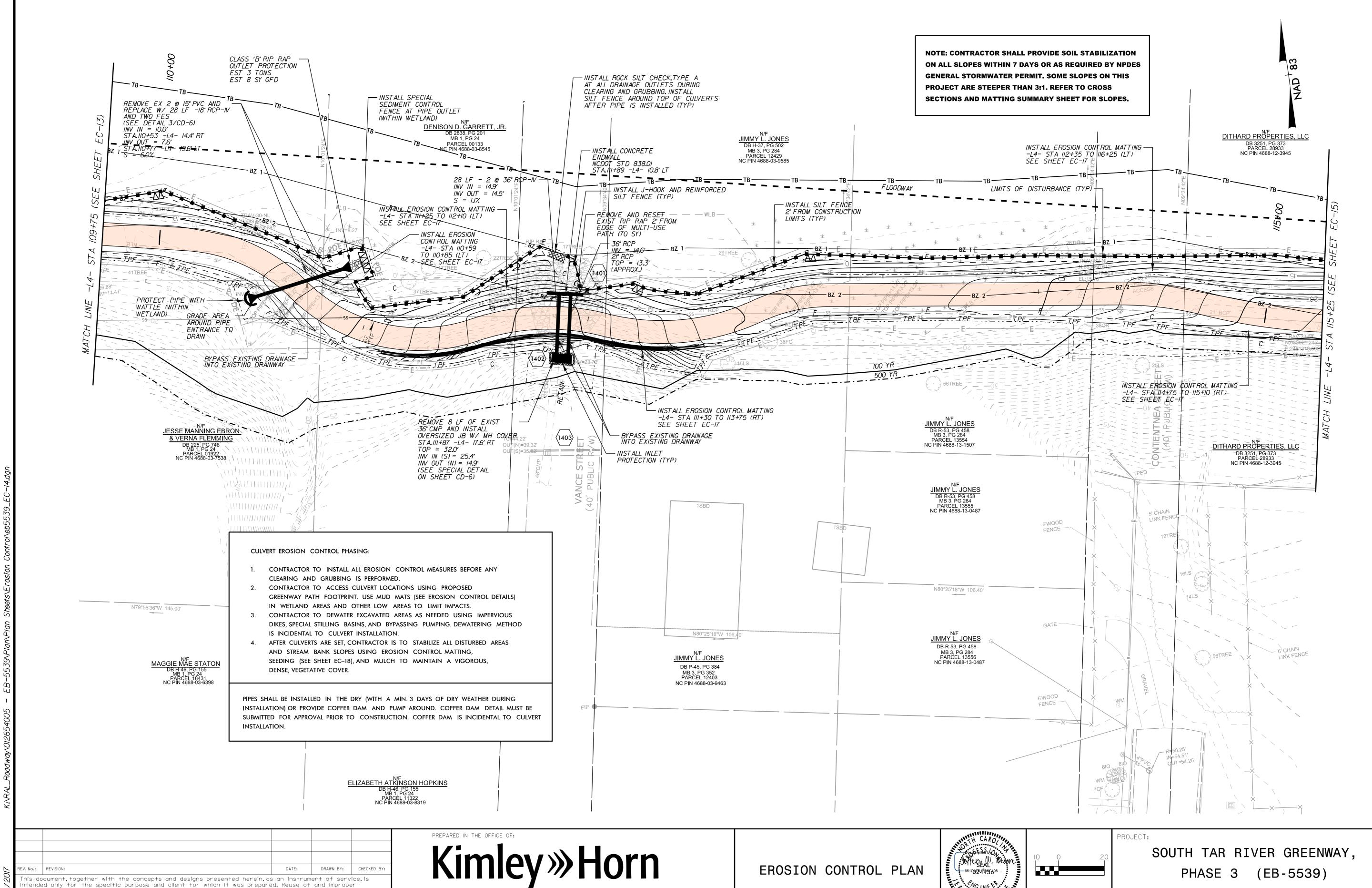
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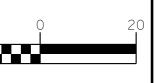
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EROSION CONTROL PLAN

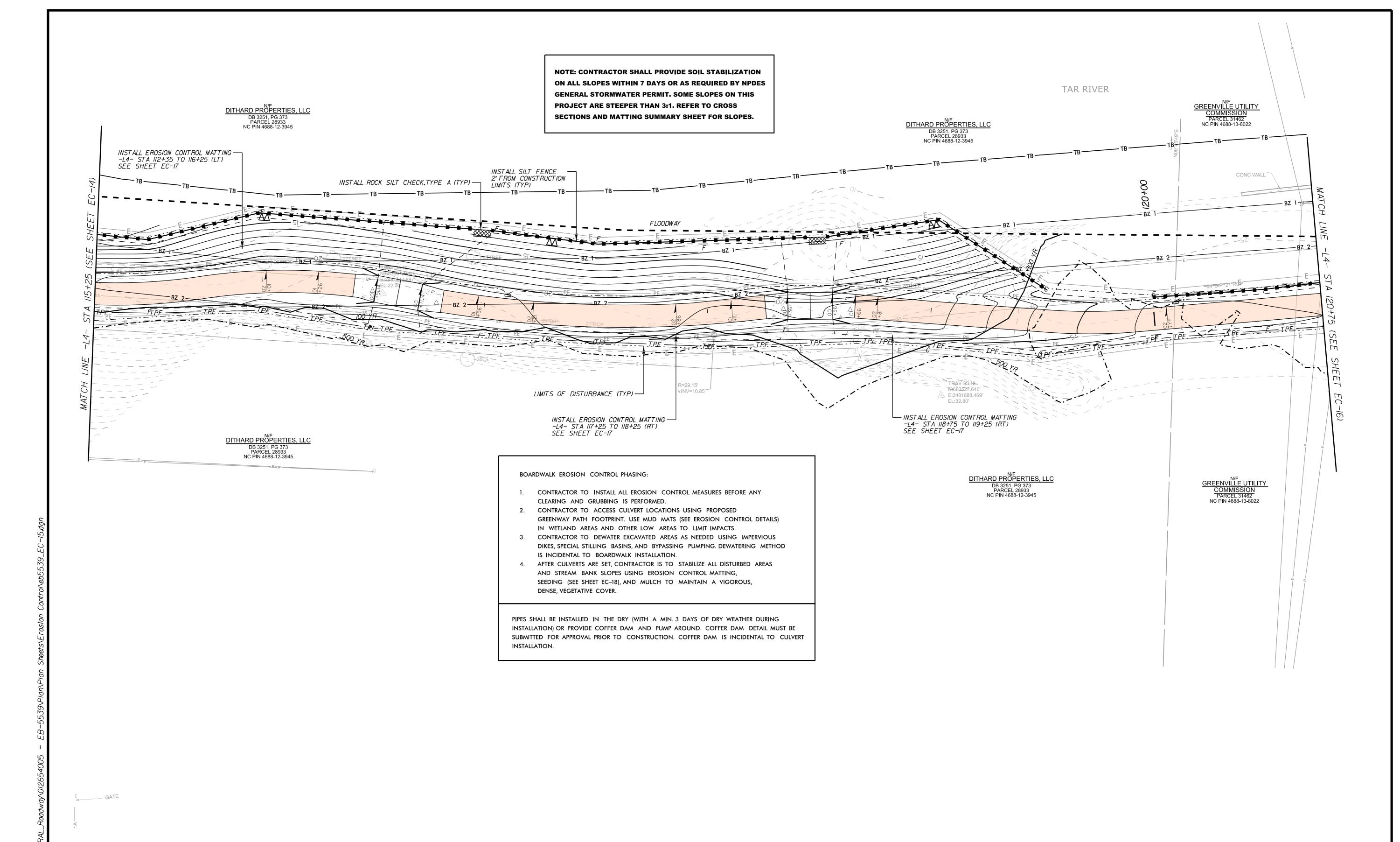




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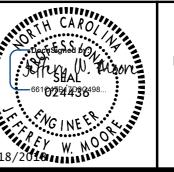
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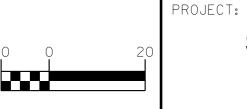
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EROSION CONTROL PLAN

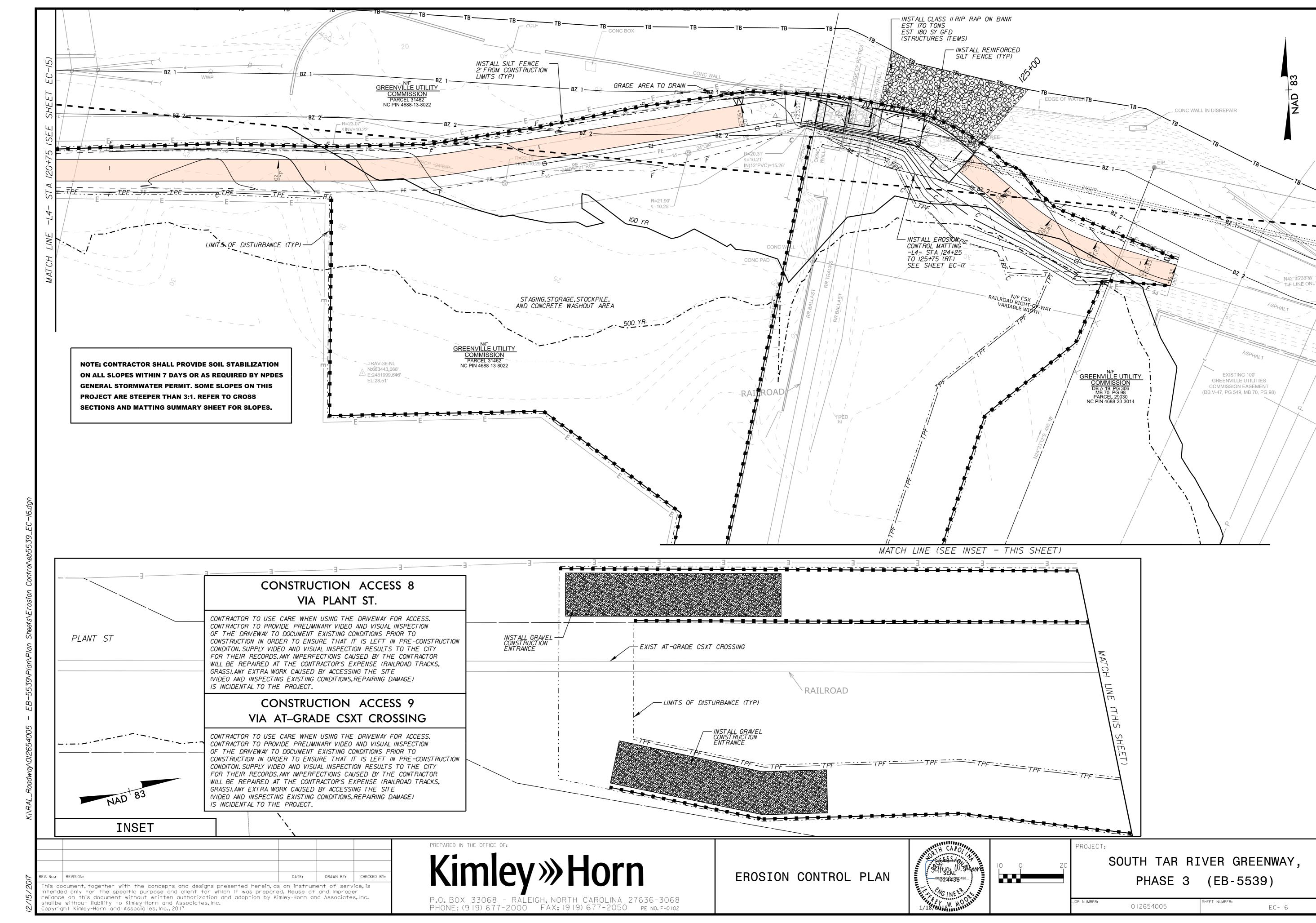




SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

EC-15

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SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
EC-1	- -	12+25	12+75	レイ	65
EC-1	- -	12+25	13+25	R1	115
EC-2	- -	15+70	16+10	R1	60
EC-5/EC-6	- -	33+25	33+75	R1	30
EC-6	- -	36+75	37+75	R1	135
EC-6	- -	37+00	38+00	LT	170
EC-8/EC-9	-L2-	60+25	64+25	LT	835
EC-8	-L2-	62+85	63+20	R1	45
EC-10	-レ3-	81+25	81+70	LT	60
EC-11	-レ3-	85+50	86+25	LT	70
EC-11	-レ3-	87+20	88+70	LT	240
EC-11	-レ3-	87+25	88+25	R1	95
EC-12	-レ3-	89+75	94+25	LT	600
EC-12	-レ3-	89+75	91+25	R1	300
EC-13	-レ3-	93+94	94+10	RT	80
EC-13	-レ3-	96+75	98+25	LT	50
EC-13	-レ3-	97+20	98+25	R1	85
EC-13	- L4 -	108+25	108+75	R1	20
EC-14	- L4 -	110+59	110+85	LT	25
EC-14	- L4 -	111+25	112+10	LT	130
EC-14	- L4 -	111+30	113+75	R1	150
EC-14/EC-15	- L4 -	112+35	116+25	LT	485
EC-14	- L4 -	114+75	115+10	R1	10
EC-15	- L4 -	117+25	118+25	R1	45
EC-15	- L4 -	118+75	119+25	R1	45
EC-16	- L4 -	124+25	125+75	R1	160
			SUF	3T0TAL	4105
MISCELLANE	OUS MATTING TO BE INSTA	LLED AS DIRE			675
				TOTAL	4780
				SAY	4800
		<u> </u>			

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EROSION CONTROL DETAILS



NOT TO SCALE

SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

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CONSTRUCTION SPECIFICATIONS

- I. Hold preconstruction conference at least one week prior to starting construction and invite NCDEMLR LQS contact number is (919) 707-9200.
- 2. Flag the work limits for protection.
- 3. Prior to any land disturbing (including demolition) activities, install silt/tree protection fence, construction entrance, temporary stream crossings, and all other erosion control devices.
- 4. In accordance with the erosion control plans, grade trail, install crosspipes, and place reinforced silt fence as shown. Place additional erosion control matting and stabilize 2:I slopes as indicated. Modify silt fence placement around trail and outlets as necessary, place silt fence around all temporary crossings and culverts as needed.
- 5. Complete final grading for trail.
- 6. Finish grading of slopes, topsoil critical areas and permanently vegetate, seed
- 7. All graded areas will be seeded, fertilized and mulched according to NCDOT specifications to maintain a vigorous, dense, vegetative cover within 7 to 14 days or sooner of completion of any phase of grading. Refer to the latest NPDES Rules dated August 2011 for more information on ground stabilization requirements. In summary, perimeter dikes, swales, ditches, slopes steeper than 3:1, and high quality (HQW) zones should be stabilized within 7 days. All other areas and slopes shall be stabilized within 14 days. If work on the project ceases for more than the above mentioned length of time, all disturbed areas shall have temporary vegetative ground cover established and erosion control devices maintained.
- 8. All erosion and sediment control practices will be inspected weekly and after rainfall events. Needed repairs will be made immediately.
- 9. Estimated time before final stabilization is 12 months.
- 10. Site includes approximately 5.97 acres of permanent vegetation area.
- II. AFTER SITE IS STABILIZED, construction entrances, and construction staging and material area stockpile areas, and all other erosion control devices shall be removed, restored as existing, and permanently vegetated as described in the maintenance and vegetative plan. Contractor must receive permission by project inspector before any measures are

MAINTENANCE

Follow the construction sequence throughout project development. Adequate erosion and sediment control measures must be installed, maintained and adjusted as needed during the demolition or clearing and grubbing phases as well as throughout the life of and until permanent vegetation on the project is established. When changes in construction activities are needed, amend the sequence schedule in advance to maintain management control.

Notification of Land Resources Sediment and Erosion Control Self-Inspection Program

The Sedimentation Pollution Control Act was amended in 2006 to require that persons responsible for land-disturbing activities inspect a project after each persons responsible for land-disturbing activities inspect a project after each phase of the project to make sure that the approved erosion and sedimentation control plan is being followed. Rules detailing the documentation of these inspections took effect October 1,2010. To simplify documentation of Self-Inspection Reports and NPDES Self-Monitoring Reports, a combined form is now available. The new form was developed to satisfy the requirements of the Sedimentation Pollution Control Act and the NPDES Stormwater Permit for Construction Activities, NCG 010000. Beginning August 1,2013, the Division of Energy, Mineral, and Land Resources is responsible for administering both the SPCA and the NPDES Stormwater Permit for Construction Activities, NCG 010000. The combined form should make it easier to comply with self-inspection requirements.

The Combined Self-Monitoring form is available as a PDF and Word document from the Land Quality web site, http://portal.ncdenr.org/web/Ir/erosion

If you have questions, please contact the Land Quality Section at a DENR Regional Office at 919-707-9200.

If the same person conducts the land—disturbing activity & any related borrow or waste activity, the related borrow or waste activity shall constitute part of the land—disturbing activity unless the borrow or waste activity is regulated under the Mining Act of 1971, or is a landfill regulated by the Division of Waste Management of the land—disturbing landfill regulated by the Division of Waste Management.It activity and any related borrow or waste activity are not conducted by the same person, they shall be considered separate land—disturbing activities and must be permitted either through the Sedimentation Pollution Control Act as a one—use borrow site or through the Mining Act.

MAINTENANCE PLAN

- I. The Contractor shall check all erosion and sediment control practices for stability and operation following every runoff producing rainfall but in no case less than once every week. Any needed repairs will be made immediately by the Contractor to maintain all practices as designed. Also per National Pollutant Discharge Elimination System (NPDES) general stormwater permit, a rain gauge must be installed on site. The rain gauge must be kept onsite and inspections by the Contractor must be made and logged after every half inch of rainfall and once a week.
- 2. The Contractor shall remove sediment from erosion control devices when storage capacity has been approximately 50% filled. Gravel will be cleaned or replaced when the sediment pools no longer drains properly.
- 3. The Contractor shall remove sediment from behind silt fence when it becomes 0.5 feet deep at the fence. Silt fence will be repaired as necessary to maintain a barrier.
- 4. The Contractor shall fertilize, reseed as necessary, and mulch all seeded areas according to specifications in the vegetative plan to maintain a vigorous, dense vegetative
- 5.The Contractor shall provide ground cover on exposed slopes or other areas within 7 to 14 days or sooner of completion of any phase of grading. Refer to the latest NPDES Rules dated August Portagonal ground stabilization requirements. Permanent ground cover is to be provided for all disturbed areas within 7 to 14 days or sooner following completion of construction or development.
- 6. The City of Greenville contact is Lynn Raynor, (252) 329-4620

RIP RAP (6.15)

CONSTRUCTION SPECIFICATIONS

Subgrade Preparation — Prepare the subgrade for riprap and filter to the required lines and grades shown on the plans. Compact any fill required in the subgrade to a density approximating that of the surrounding undisturbed material or overfill depressions with riprap. Remove brush, trees, stumps and other objectional material. Cut the subgrade sufficiently deep that the finished grade of the riprap will be at the elevation of the surrounding area. Channels should be excavated sufficiently to allow placement of the riprap in a manner such that the finished inside dimensions and grade of the riprap meet design specifications.

Sand and gravel filter blanket — Place the filter blanket immediately after the ground foundation is prepared. For gravel, spread filter stone in a uniform layer to the specified depth. Where more than one layer of filter material is used, spread the layers with minimal mixing.

Synthetic filter fabric - Place the cloth filter directly on the prepared foundation. Overlap the edges by at least 12 inches, and space anchor pins every 3 ft along the overlap. Bury the upstream end of the cloth a minimum of 12 inches below ground and where necessary, bury the lower end of the cloth or overlap with the next section as required. Take care not to damage the cloth when placing riprap. If damage occurs remove the riprap and repair the sheet by adding another layer of filter material with a minimum overlap of 12 inches around the damaged area. If extensive damage is suspected, remove and replace the entire sheet.

Where large stones are used or machine placement is difficult, a 4-inch layer of fine gravel or sand may be needed to protect the filter cloth.

Stone Placement — Placement of riprap should follow immediately after placement of the filter. Place riprap so that if forms a dense well—graded mass of stone with a minimum of voids. The desired disbribution of stones throughout the mass may be obtained by selective loading at the quarry and controlled dumping during final placement. Place riprap to its full thickness in one operation. Do not place riprap by dumping through chutes or other methods that cause segregation of stone sizes. Take care not to dislodge the underlying base or filter when placing

The finished slope should be free of pockets of small stone or clusters of large stones. Hand placing may be necessary to achieve the proper distribution of stone sizes to produce a relatively smooth, uniform surface. The finished grade of the riprap should blend with the surrounding area. No overfall or protrusion of riprap should be apparent.

MAINTENANCE

Inspect channels at regular intervals as well as after major rains, and make repairs promptly. Give special attention to the outlet and inlet sections and other points where concentrated flow enters. Carefully check stability at road crossings and look for indications of piping, scour holes, or bank failures. Make repairs immediately. Maintain all vegetation adjacent to the channel in a healthy, vigorous condition to protect the area from erosion and scour during out—of—bank flow. Control of weed and brush growth may be needed in some locations.

LAND GRADING (6.02)

CONSTRUCTION SPECIFICATIONS

I. Construct and maintain all erosion and sedimentation control practices and measures in accordance with the approved sedimentation control plan and construction schedule.

2. Remove good topsoil from areas to be graded and filled, and preserve it for use in finishing the grading of all critical greas.

3. Scarify areas to be topsoiled to a minimum depth of 2 inches before placing topsoil.

4. Clear and grub areas to be filled to remove trees, vegetation, roots, or other objectionable material that would affect the planned stability of the fill.

5. Ensure that fill material is free of brush, rubbish, rocks, logs, stumps, building debris, and other materials inappropriate for constructing stable fills.

6. Place all fill in layers not to exceed 9 inches in thickness, and compact the layers as required to reduce erosion, slippage, settlement, or other related problems.

7. Do not incorporate frozen material or soft or highly compressible materials into fill slopes.

8. Do not place fill on a frozen foundation, due to possible subsidence and

9. Keep diversions and other water conveyance measures free of sediment during all phases of development.

10. Handle seeps or springs encountered during construction in accordance with approved methods.

II. Permanently stabilize all graded areas immediately after final grading is completed on each area in the grading plan. Apply temporary stabilization measures on all graded areas when work is to be interrupted or delayed for 15 working days or longer.

12. Show topsoil stockpiles, borrow areas, and spoil areas on the plans, and make sure they are adequately protected from erosion. Include final stabilization of these areas in the plan.

MAINTENANCE

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Periodically check all graded areas and the supporting erosion and sedimentation control practices, especially after heavy rainfalls. Promptly remove all sediment from diversion and other water-disposal practices. If washouts or breaks occur, repair them immediately. Prompt maintenance of small eroded areas before they become significant gullies is an essential part of an effective erosion and sedimentation control plan.

GRASS-LINED CHANNELS (6.30)

CONSTRUCTION SPECIFICATIONS

I. Remove all trees, brush, stumps, and other objectionable material from the foundation area and dispose of properly.

2. Excavate the channel and shape it to neat lines and dimensions shown on the plans plus a 0.2-ft overcut around the channel perimeter to allow for bulking during seedbed preparations and sod buildup.

3. Remove and properly dispose of all excess soil so that surface water may enter the channel freely.

4. The procedure used to establish grass in the channel will depend upon the severity of the conditions and selection of species. Protect the channel with mulch or a temporary liner sufficient to withstand anticipated velocities during the establishment period.

MAINTENANCE

During the establishment period, check grass—lined channels after every rainfall. After grass is established, periodically check the channel; check it after every heavy rainfall event. Immediately make repairs. It is particularly important to check the channel outlet and all road crossings for bank stability and evidence of piping or scour holes. Remove all significant sediment accumulations to maistain the designed carrying eappairs. Keep the grass in a to maintain the designed carrying capacity. Keep the grass in a healthy, vigorous condition at all times, since it is the primary erosion protection for the channel.

VEGETATIVE PLAN (NCDENR 6.11)

SEEDING SCHEDULE Shoulders Side Ditches Slones (3.1)

	Shoulders, Side Ditches, Slopes (3:1)	
<u>Date</u>	Туре	Planting Rate
Aug.15-Nov.1 Nov.1-Mar.1	Tall Fescue Tall Fescue & Abruzzi Rye	250 Ibs./acre 250 Ibs./acre 25 Ibs./acre
Mar.I-Apr.I5 Apr.I5-Jun.I5 Jun.I5-Aug.I5	Tall Fescue Hulled Common Bermudagrass Tall Fescue & Browntop Millet *** or Sorghum-Sudan Hybrids ***	250 lbs./acre 12 lbs./acre 250 lbs./acre 35 lbs./acre 30 lbs./acre
	Slopes (3;1 to 2:1)	
Mar.I-Jun.I	Sericea Lespedeza (scarified) and	50 lbs./acre
(Mar.I-Apr.I5) (Mar.I-Jun.30) (Mar.I-Jun.30) Jun.I-Sep.I	Add Tall Fescue or Add Weeping Lovegrass or Add Hulled Common Bermudagrass Tall Fescue *** & Browntop Millet ***	250 Ibs./acre 35 Ibs./acre
Sep.I-Mar.I	or Sorghum-Sudan Hybrids *** Sericea Lespedeza (unhulled-unscarified) & Tall Fescue	30 Ibs./acre 70 Ibs./acre 250 Ibs./acre
(Nov.I-Mar.I)	Add Abruzzi Rye	25 lbs./acre

Consult Conservation Engineer or Soil Conservation Service for additional information concerning other alternatives for vegetation of denuded areas. The above vegetation rates are those which do well under local conditions: other seeding rate combinations are possible.

*** Temporary. - Reseed according to optimum season for desired permanent vegetation. Do not allow temporary cover to grow over 12 inches in height before mowing, otherwise, fescue may be shaded out.

SEEDING SPECIFICATIONS

- After rough grading is completed, till soil in areas to be seeded and planted to a depth of six inches.
- 2) Apply agricultural lime, fertilizer, and superphosphate to
 - disturbed areas to be vegetated. 90 lbs.limestone/1000 sq.ft.(2 tons/acre) *35 lbs.10-10-10 fertilizer/1000 sq.ft.(1500 lbs/acre)*
- 40 lbs.50% superphosphate/1000 sq.ft.(1750 lbs/acre) Disk nutrients into soil to a depth of six inches until surface is uniform and free of large dirt clods.
- Seeding permanent grass. 3.0 lbs.KY-31 tall fescue/1000 sq.ft.(130 lbs./acre) during February 15 through May 15 or August 15 through
- -0R-3.0 lbs.KY-31 tall fescue and 2.0 lbs.annual ryegrass/1000 sq.ft.during November 15 through February 15. Mulch seeded area with small grain straw at 90 lbs/1000 sq.ft.(2 tons/acre). Spread uniformly. Approximately 1/2 of ground surface should be visible to avoid blocking sunlight to seedlings. On areas where the ground surface equals or exceeds a 3:1 slope, and in the inverts of all drainage swales, tack mulch with asphalt emulsion
- at a rate of 450 gallons emulsion per acre of straw. Mulch around shubbery and trees with pine straw to depth of 3 inches.
- 7) Temporary cover

November 15.

- I.O Ibs. brown top millet/IOOO sq.ft. May through August 25.
- I.O Ibs.annual ryegrass/IOOO sq.ft. August 25 through April.

TOPSOILING (6.04)

CONSTRUCTION SPECIFICATIONS

MATERIALS

Determine whether the quality and quantity of available topsoil justifies selective handling. Quality topsoil has the following characteristics:

Texture - loam,sandy loam,and silt loam are best;sandy clay loam,silty clay loam, clay loam, and loamy sand are fair. Do not use heavy clay and organic soils such as peat or muck as topsoil.

Organic matter_content - (sometimes referred to as "humic matter") should be greater than 1.5% by weight.

Acidity — pH should be greater than 3.6 before liming, and liming is required if it is less than 6.0.

Soluble salts - should be less than 500 ppm.

Sodium - sodium adsorption ratio should be less than 12.

The depth of material meeting the above qualifications should be at least 2 inches. Soil factors such as rock fragments, slope, depth to water table, and layer thickness affect the ease of excavation and spreading of topsoil.

Generally, the upper part of the soil, which is richest in organic matter, is most desirable; however, material excavated from deeper layers may be worth storing if it meets the other criteria listed above.

Organic soils such as mucks and peats do not make good topsoil. They can be identified by their extremely light weight when dry.

STRIPPING

Strip topsoil only from those areas that will be disturbed by excavation, filling, roadbuilding, or compaction by equipment. A 4 to 6-inch stripping depth is common, but depth varies depending on the site. Determine depth of stripping by taking soil cores at several locations within each area to be stripped. Topsoil depth generally varies along a gradient from hilltop to toe of the slope. Put sediment basins, diversions, and other controls into place before stripping.

STOCKPILING

Select stockpile location to avoid slopes and natural drainageways, avoiding traffic routes. On large sites, respreading is easier and more economical when topsoil is stockpiled in small piles located near areas where they will be used. All stockpile areas used shall be stabilized with silt fence and seeded.

Sediment barriers - Use sediment fences or other barriers where necessary to retain sediment.

Temporary seeding - Protect topsoil stockpiles by temporarily seeding as soon as possible, no more than 30 working days or 120 calendar days after the formation of the stockpile.

Permanent vegetation - If stockpiles will not be used within 12 months they must be stabilized with permanent vegetation to control erosion and weed

SITE PREPARATION

Before spreading topsoil, establish erosion and sedimentation control practices such as diversions, berms, dikes, waterways, and sediment basins.

Grading — Maintain grades on the areas to be topsoiled according to the approved plan and do not alter them by adding topsoil.

Liming of subsoil - Where the pH of the existing subsoil is 6.0 or less, or the soil is composed of heavy clays, incorporate agricultural limestone in amounts recommended by soil tests or specified for the seeding mixture to be used. Incorporate lime to a depth of at least 2 inches by disking.

Roughening — Immediately prior to spreading the topsoil.loosen the subgrade by disking or scarifying to a depth of at least 4 inches, to ensure bonding of the topsoil and subsoil. If no amendments have been incorporated, loosen the soil to a depth of at least 6 inches before spreading topsoil.

SPREADING TOPSOIL

Do not spread topsoil while it is frozen or muddy or when subgrade is wet or frozen. Correct any irregularities in the surface that result from topsoiling or other operations to prevent the formation of depressions or water pockets.

Compact the topsoil enough to ensure good contact with the underlying soil, but avoid excessive compaction, as it increases runoff and inhibits seed germination. Light packing with a roller is recommended where high-maintenance turf is to be established.

On slopes and areas that will not be mowed, the surface may be left rough after spreading topsoil. A disk may be used to promote bonding at the interface between topsoil and subsoil.

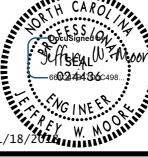
After topsoil application, follow procedure for seedbed preparation, taking care to avoid excessive mixing of topsoil into the subsoil.

PROJECT:

PREPARED IN THE OFFICE OF:



EROSION CONTROL DETAILS



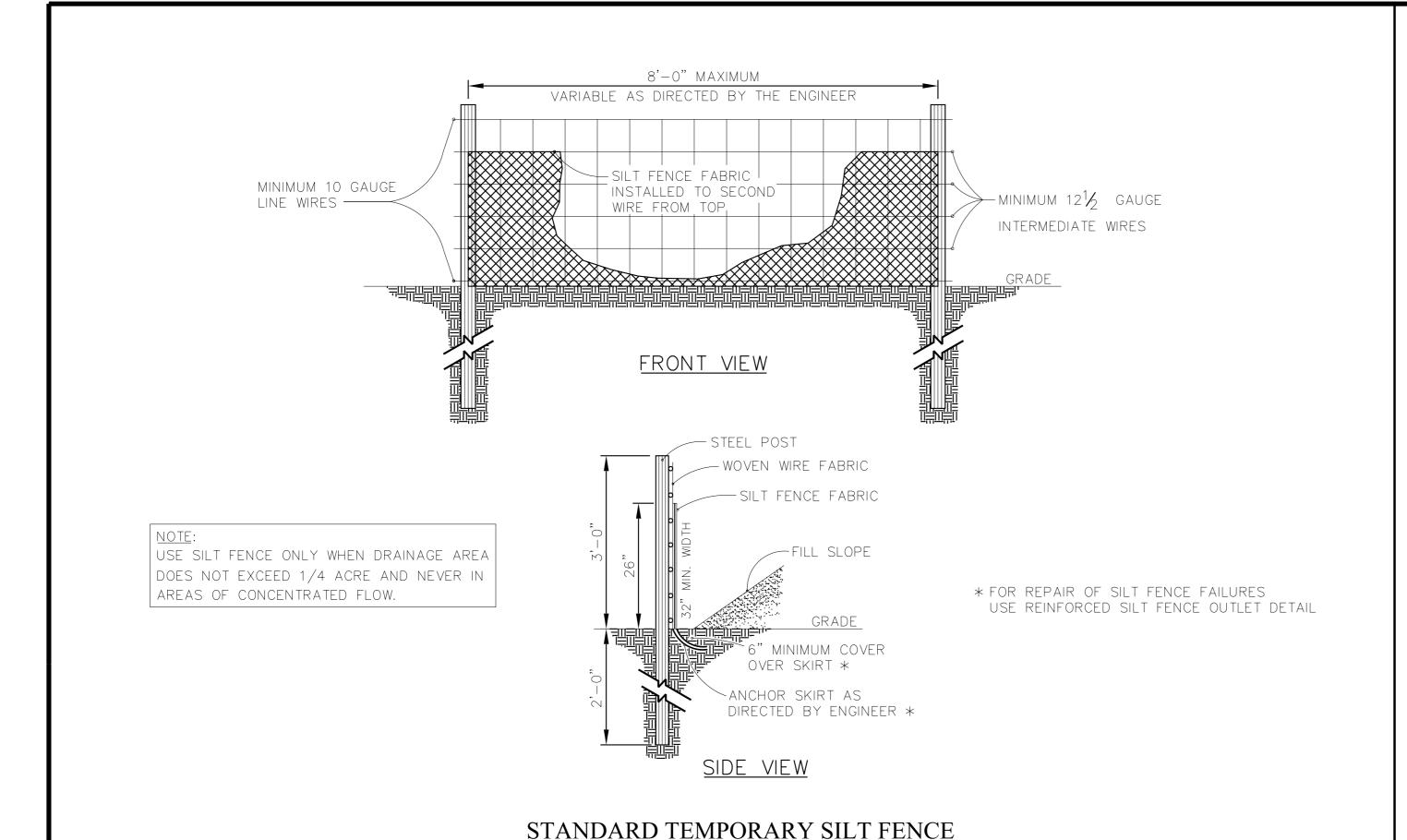
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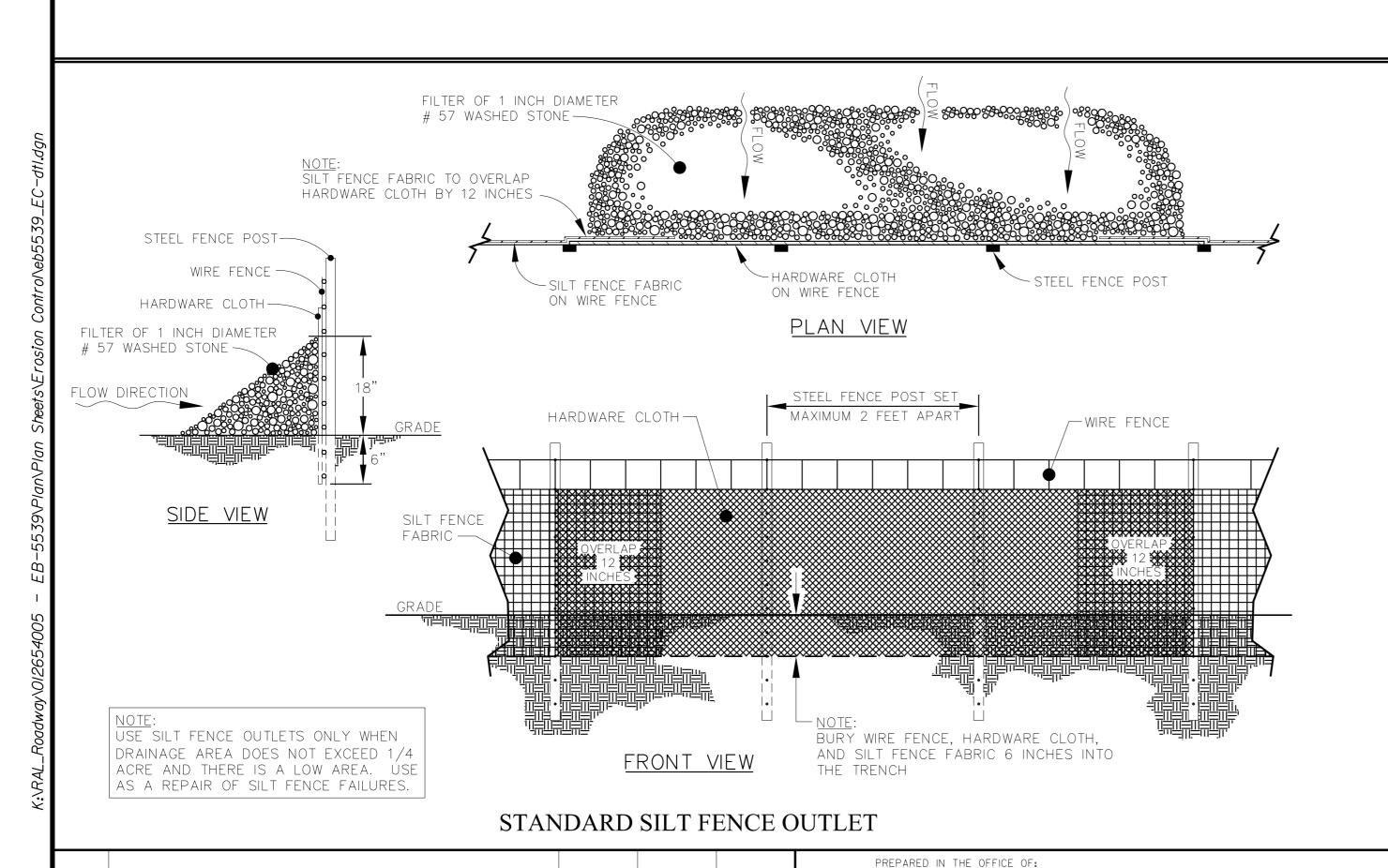
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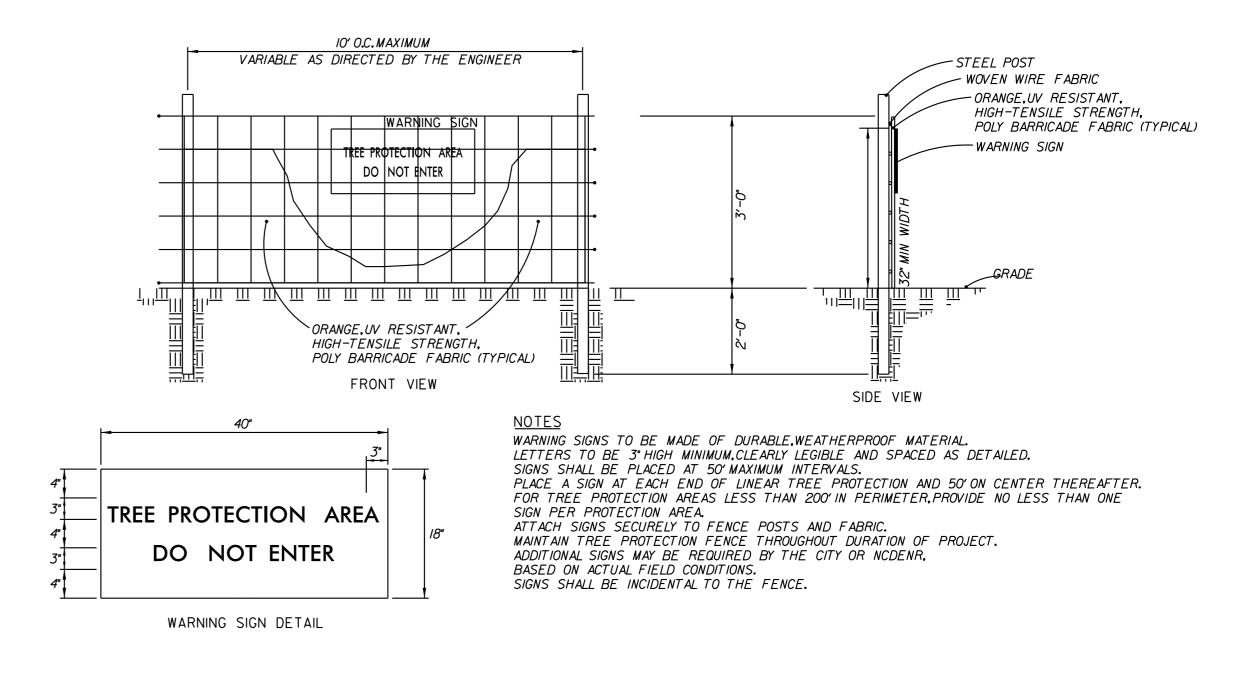
HARDWARE CLOTH AND GRAVEL INLET PROTECTION DETAIL (6.51) CONSTRUCTION SPECIFICATIONS I. As synthetic fabric, use a previous sheet of nylon, polyester, or ethylene yarn — extra strength (50 lb/l inch minimum) — that contains ultraviolet ray inhibitors and stabilizers. Fabric should be sufficiently porous to provide adequate drainage of the temporary sediment pool. Burlap may be used for short-term applications. It must be replaced every 60 days. 2. Cut fabric from a continuous roll to eliminate joints. 3. For stakes, use 5-foot steel posts. 4. Space stakes evenly around the perimeter of the inlet a maximum of 4 ft apart, and securely drive them into the ground, approximately 24 inches deep. 5. Place a 2 foot flap of wire mesh under the gravel for anchoring. 6. Fasten fabric securely to the stakes and frame. Joints must be overlapped to the next stake. 7. The top of the frame and fabric must be well below the ground elevation downslope from the drop inlet to keep runoff from bypassing the inlet. It may be necessary to build a temporary dike on the down slope side of the structure to prevent bypass flow. Material from within the STANDARDIZED METAL POSTS 2'-O"IN GROUND sediment pool may be used for diking. 8.*57 washed stone shall be paid for at the contract unit price per ton "Sediment Control Stone." MAINTENANCE Inspect the fabric barrier after each rain and make repairs as needed. GALVANIZED HARDWARE WIRE EXTENDS TO TOP OF BOX Remove sediment from the pool area as necessary to provide adequate storage volume for the next rain. Take care not to damage or undercut the fabric during sediment removal. - METAL POST - ¼" HARDWARE



DRAWN BY: CHECKED BY:

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TREE PROTECTION DETAIL

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EROSION CONTROL DETAILS

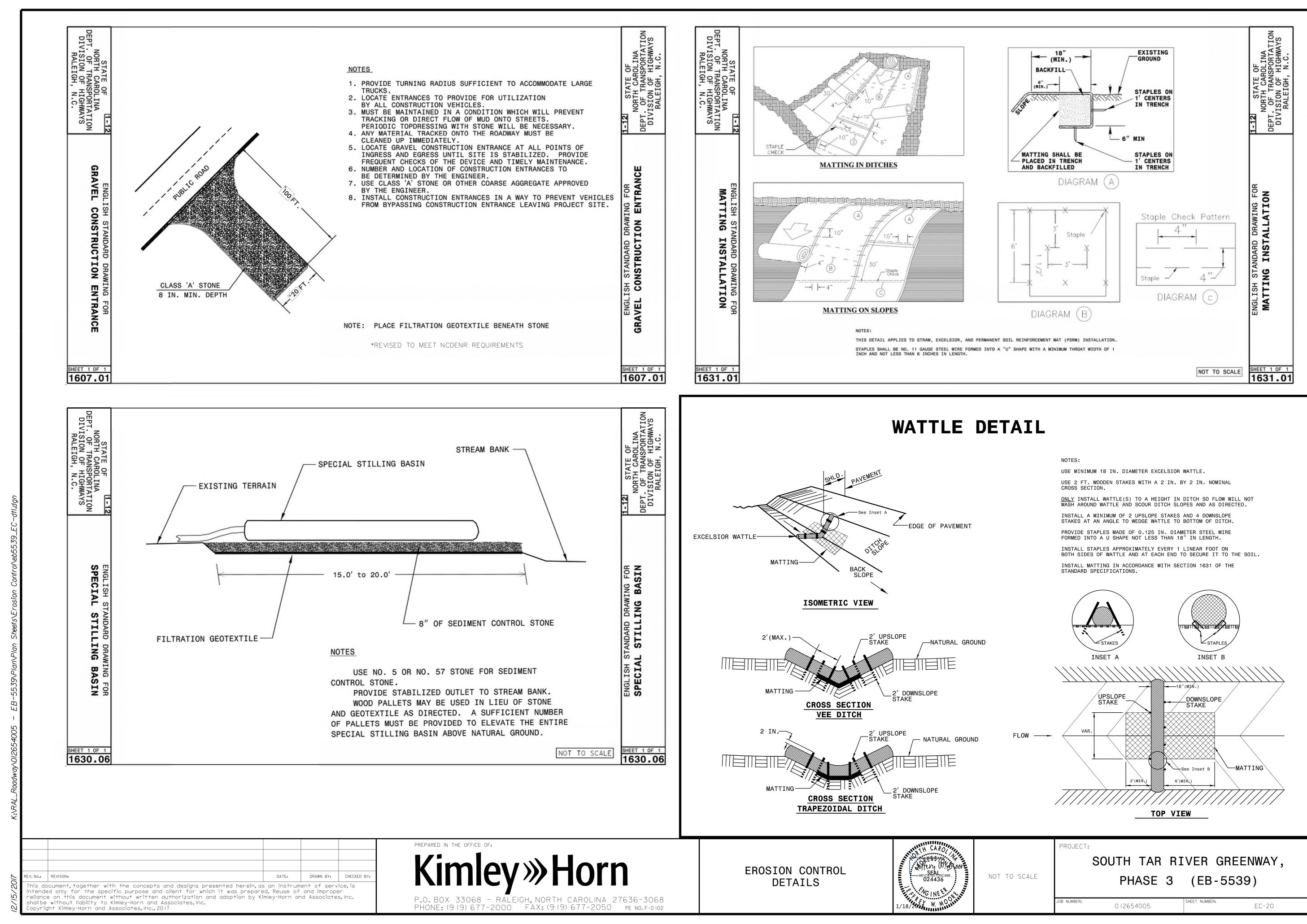


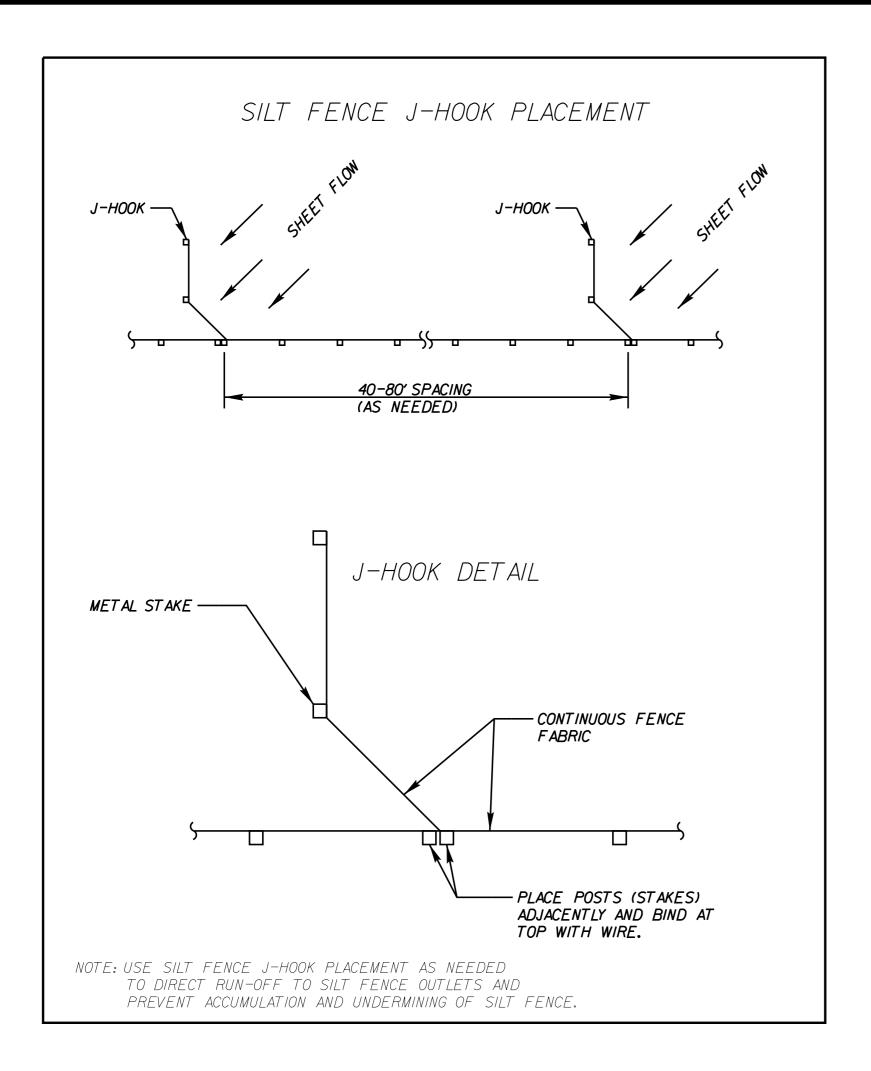
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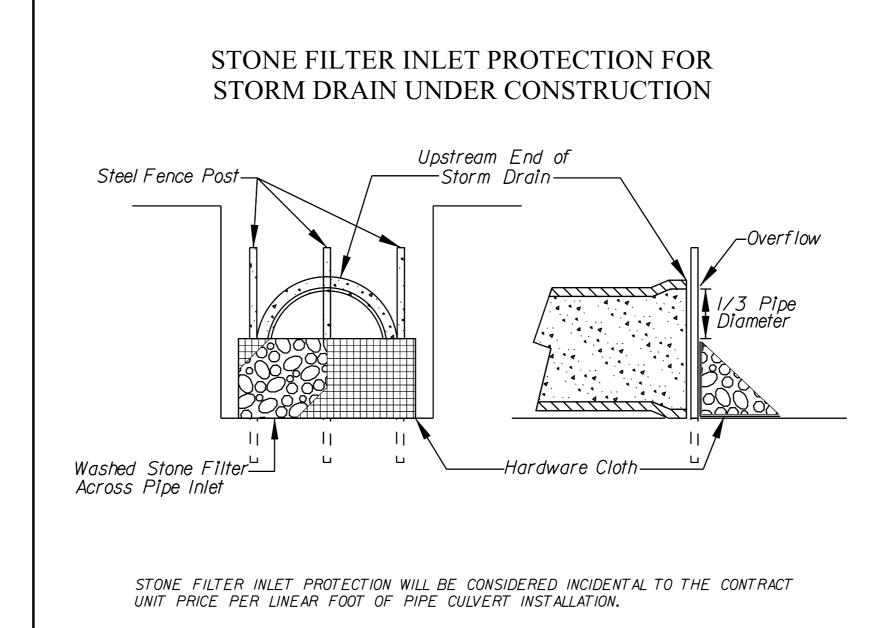
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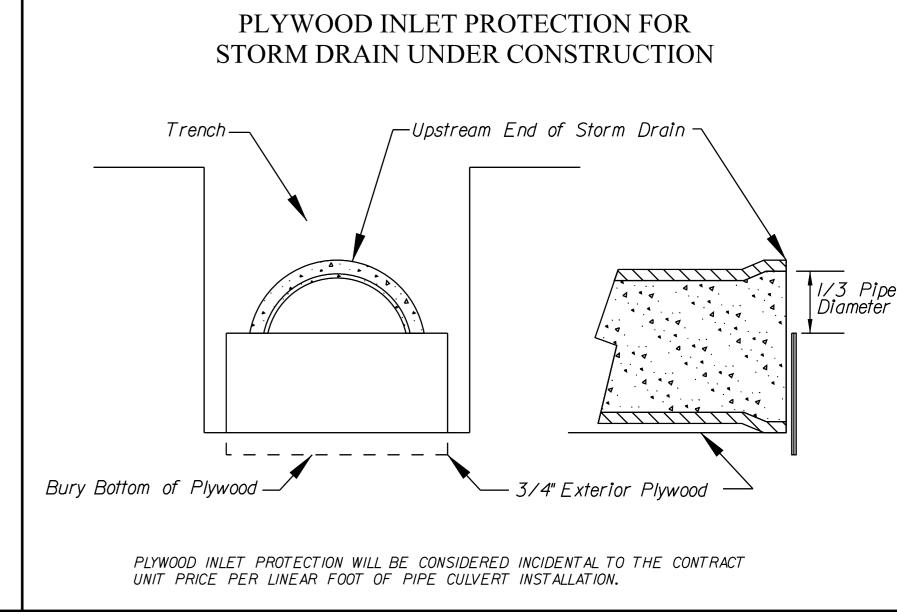
SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

0 12654005 EC-19









LOOPS SIZED FOR

1" REBAR, LIFT

FOR HANDLES.

OVERFLOW HOLES

GEOTEXTILE BAG-

1/4" BRIGHTLY COLORED NYLON ROPE EXPANSION \

(OPTIONAL)

RESTRAINT

FILTER BAG FROM

INLET USING REBAR

NPDES GENERAL STO SOIL STABILIZATI

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

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EROSION CONTROL DETAILS

NOTE: USE INLET TUBES AS NEEDED TO PREVENT SILT AND

L-----

Materials

---FILTER SACK INLET

PROTECTION

Inlet tubes may be temporarily moved during construction as needed.

Inspector or Manufacturer's Representative at the contractor's expense.

INLET TUBES "SILT SOCK" NOT TO SCALE

Replace inlet tubes damaged during installation as directed by the

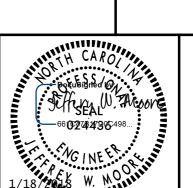
(SEE DETAIL)

DEBRIS FROM ENTERING INLETS DURING CONSTRUCTION.

OR SILT FENCE

GUTTER

SEDIMENT



✓INLET GRATE

LOOPS SIZED FOR

REBAR FOR A HANDLE

1" REBAR, USE

TO EMPTY FILTER

LOCATION.

2"X2"X3/4"-

BLOCK (TYP)

1/4"BRIGHTLY -COLORED NYLON

PROPERTIES

MULLEN BURST

TRAPEZOID TEAR

GRAB TENSILE STRENGTH ASTM D-4632
GRAB TENSILE ELONGATION ASTM D-4632

EXPANSION

RESTRAINT

EMPTY FILTER SACK WHEN BRIGHTLY COLORED LOW TO MODERATE FLOW GEOTEXTILE FABRIC SPECIFICATION TABLE

SACK AT A

SEDIMENT COLLECTION

ISOMETRIC VIEW

PROJECT: SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

NOT TO SCALE

ORM	WA I	ER	PER A	MIT	
ION	TIM	EFR.	4MES	5	

	STABILIZATION TIME	TIMEFRAME EXCEPTIONS	Use inlet tubes that exhibit the following properties: Produced by a Manufacturer experienced in sediment tube manufacturing. Composed of compacted geotextiles, curled excelsior wood, natural coconut
LOPES	7 DAYS	NONE	fibers or hardwood mulch or a mix of these materials enclosed by a flexible netting material. Do not use straw, straw fiber, straw bales, pine needles or leaf mulch under this specification. Utilize an outer netting that consists
	7 DAYS	NONE	of seamless, high—density polyethylene photodegradable materials treated with ultraviolet stabilizers or a seamless, high—density polyethylene
	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.	non—degradable materials. Installation:
	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.	Install inlet tubes lying flat on the ground, with no gaps between the underlying surface and the inlet tube.
TIIANI 1-I	LA DAVC	NONE EVERDI FOR DEDIMETERS AND HOW ZONES	Inspection and Maintenance:

EXPANSION RESTRAINT CAN NO LONGER BE GEOTEXTILE WILL BE A WOVEN

POLYPROPYLENE FABRIC THAT MEETS OR EXCEEDS REQUIREMENTS IN THE SPECIFICATIONS TABLE

AN OIL ADSORBENT PAD OR PILLOW CAN BE PURCHASED WHEN OIL SPILLS ARE A CONCERN.

INSPECT PER REGULATORY REQUIREMENTS. THE WIDTH, "W", OF THE FILTER SACK WILL MATCH THE INSIDE WIDTH OF THE GRATED

INLET BOX. THE DEPTH, "D", OF THE FILTER SACK WILL

BE BETWEEN 18 INCHES AND 36 INCHES. THE LENGTH, "L", OF THE FILTER SACK WILL MATCH THE INSIDE LENGTH OF THE GRATED INLET BOX.

ASTM D-4355 UV RESISTANCE 40 US SIEVE APPARENT OPENING SIZE ASTM D-4751 40 GAL/MIN/SQ FT FLOW RATE ASTM D-4491 PERMITTIVITY ASTM D-4491 MODERATE TO HIGH FLOW GEOTEXTILE FABRIC SPECIFICATION TABLE PROPERTIES TEST METHOD UNITS GRAB TENSILE STRENGTH ASTM D-4632 GRAB TENSILE ELONGATION | ASTM D-4632 ASTM D-4833 135 LBS MULLEN BURST ASTM D-3786 420 PSI ASTM D-4533 TRAPEZOID TEAR 45 LBS UV RESISTANCE ASTM D-4355 20 US SIEVE APPARENT OPENING SIZE ASTM D-4751 200 GAL/MIN/SQ FT 1.5 SEC -1 FLOW RATE ASTM D-4491

SECURE LIFTING LOOPS TO

OR UNDER SURROUNDING

SURFACE.

ELEVATION VIEW

PROFILE VIEW OF

INSTALLED FILTER

SACK

ASTM D-4833

ASTM D-4533

ASTM D-3786

ASTM D-4491

TEST METHOD UNITS

120 LBS

120 LBS

800 PSI

FILTER SACK INLET PROTECTION

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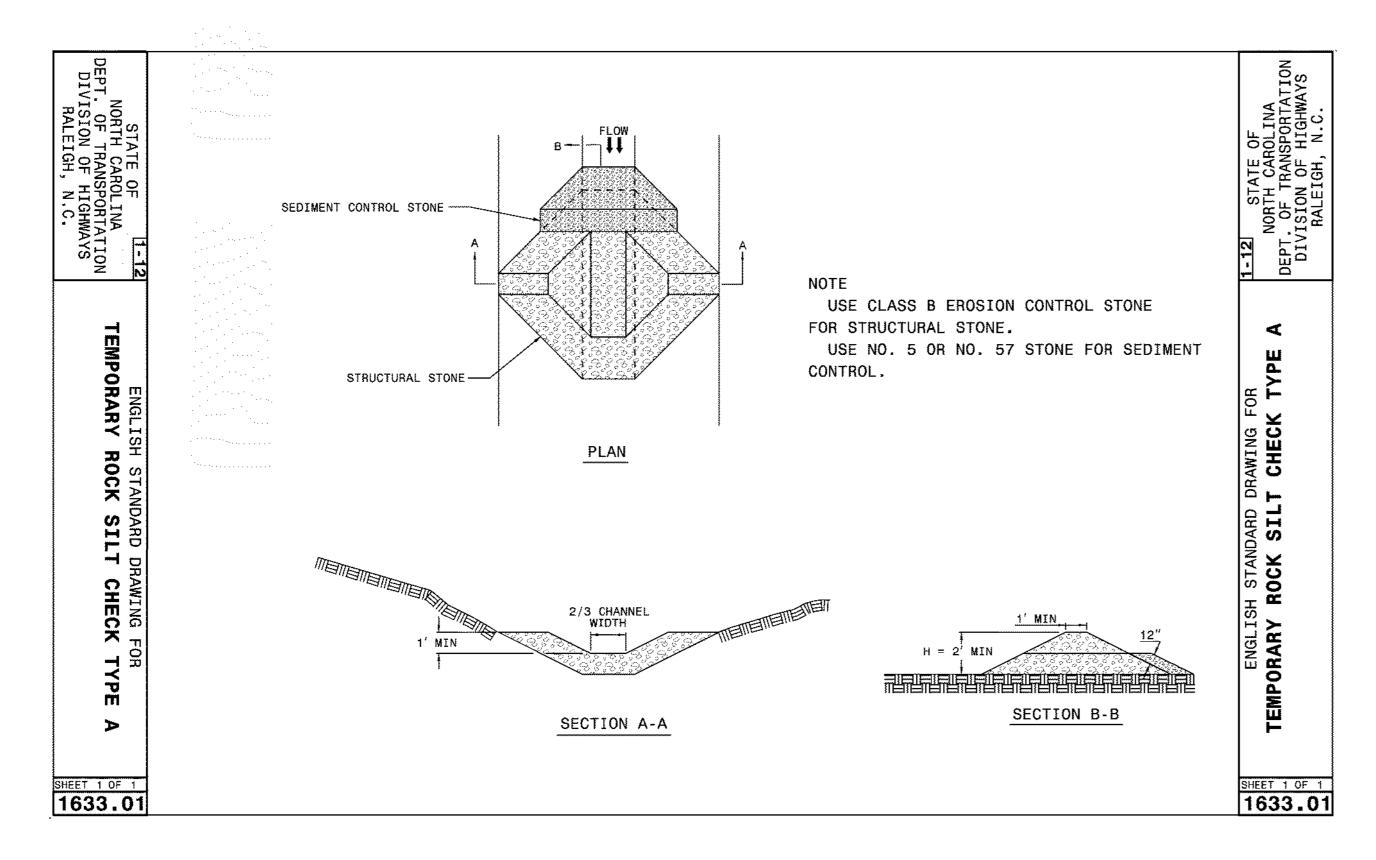
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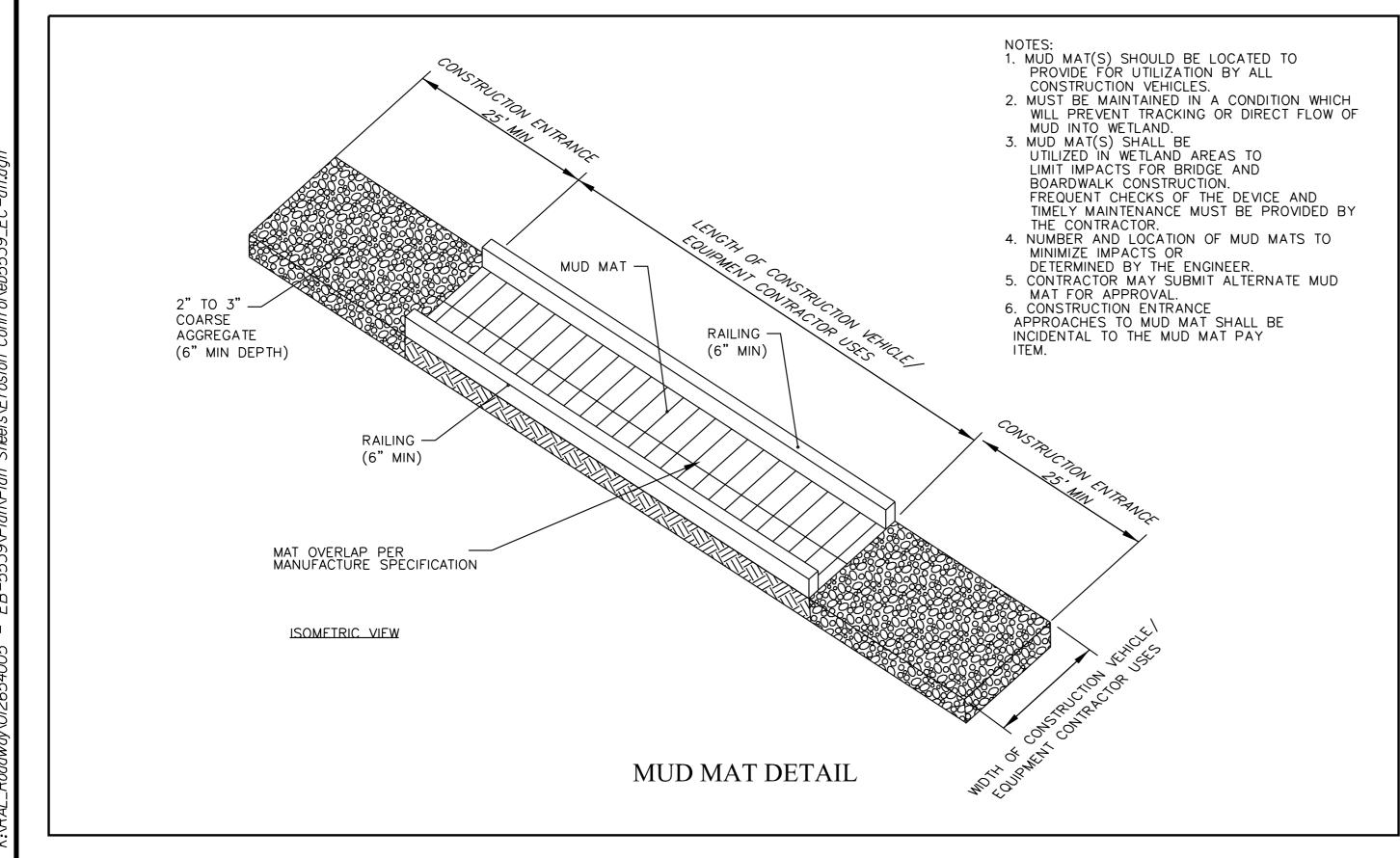
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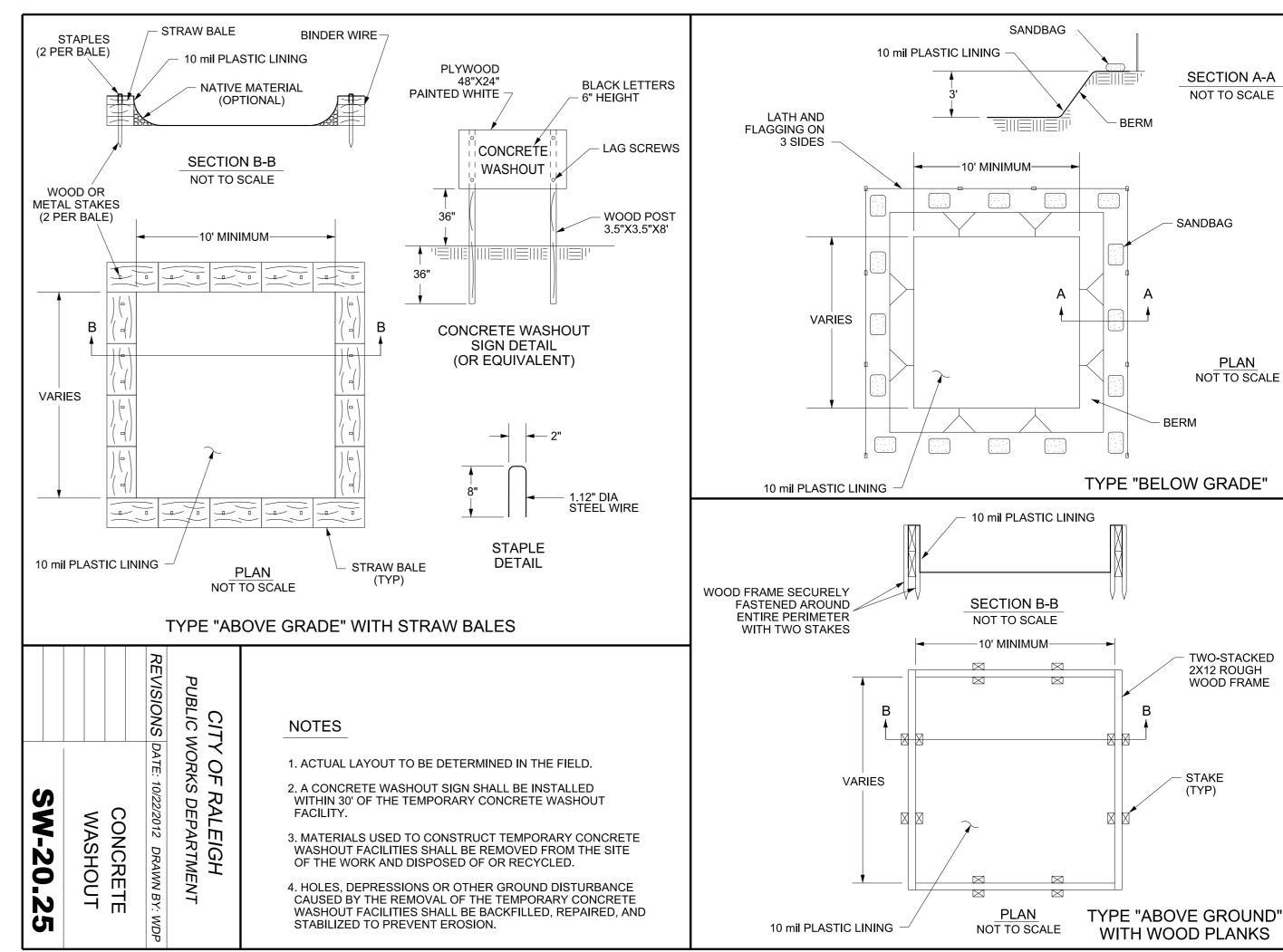
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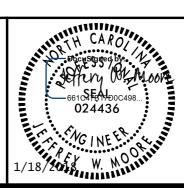


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EROSION CONTROL DETAILS



PROJECT: SOUTH TAR RIVER GREENWAY, PHASE 3 (EB-5539)

0 12654005 EC-22

GENERAL NOTES

SPECIFICATION

- 1. CURRENT EDITION OF THE AASHTO LRFD GUIDE SPECIFICATIONS FOR PEDESTRIAN BRIDGES, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION BICYCLE FACILITIES PLANNING AND DESIGN GUIDELINES, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STRUCTURES DESIGN MANUAL, NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES, AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, AND THE INCORPORATED PROJECT SPECIAL PROVISIONS.
- 2. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- 3. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- 4. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- 5. FOR PREFABRICATED PEDESTRIAN BRIDGE, SEE SPECIAL PROVISIONS.
- 6. FOR TIMBER BOARDWALK. SEE SPECIAL PROVISIONS.
- 7. FOR RETAINING WALLS, SEE SPECIAL PROVISIONS.
- 8. FOR TIMBER CANOPIES, SEE SPECIAL PROVISIONS.
- 9. FOR PRECAST CONCRETE BOARDWALK, SEE SPECIAL PROVISIONS.
- 10. FOR PILE SUPPORTED SLAB, SEE SPECIAL PROVISIONS.
- 11. FOR PRECAST CONCRETE BOARDWALK, SEE SPECIAL PROVISIONS.

MATERIAL AND WORKMANSHIP

PROVIDE ALL MATERIAL AND WORKMANSHIP IN ACCORDANCE WITH THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES, 2012 EDITION, UNLESS OTHERWISE SPECIFIED ON THE PLANS OR IN THE SPECIAL PROVISIONS.

BOARDWALK AND PEDESTRIAN BRIDGE DESIGN DATA

- 1. UNIFORM PEDESTRIAN LIVE LOAD_____90 PSF
- 2. VEHICULAR LIVE LOAD______AASHTO H-5-44 (BRIDGE, BW #1, & BW #2)

 AASHTO H-10-44 (BW #3 & BW #4)
- 3. WIND LOAD_____PER AASHTO
- 4. WATER LOAD_____PER AASHTO
- 5. SEISMIC LOAD_____PER AASHTO
- 6. TEMPERATURE LOAD_____PER AASHTO

FOUNDATIONS

- 1. FOR FOUNDATION RECOMMENDATIONS AND DESIGN INFORMATION, REFER TO THE FALCON ENGINEERING GEOTECHNICAL REPORT DATED MAY 12, 2015 AND REVISIONS DATED NOVEMBER 18, 2015 AND JUNE 7, 2016.
- 2. TIMBER BOARDWALKS
 - A. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. TIMBER PILES AT BOARDWALK #1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 8 TONS PER PILE.
 - DRIVE TIMBER PILES AT BOARDWALK #1 TO A REQUIRED DRIVING RESISTANCE OF 16 TONS PER PILE.
 - TIMBER PILES AT BOARDWALK #2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 8 TONS PER PILE.
 - DRIVE TIMBER PILES AT BOARDWALK #2 TO A REQUIRED DRIVING RESISTANCE OF 16 TONS PER PILE.
 - TIMBER PILES AT BOARDWALK #3 ARE DESIGNED FOR A FACTORED RESISTANCE OF 8 TONS PER PILE.
 - DRIVE TIMBER PILES AT BOARDWALK #3 TO A REQUIRED DRIVING RESISTANCE OF 16 TONS PER PILE. TIMBER PILES AT BOARDWALK #4 ARE DESIGNED FOR A FACTORED
 - RESISTANCE OF 8 TONS PER PILE. DRIVE TIMBER PILES AT BOARDWALK #4 TO A REQUIRED DRIVING
 - RESISTANCE OF 16 TONS PER PILE. INSTALL TIMBER PILES AT ALL BENTS TO A TIP ELEVATION NO
 - HIGHER THAN 2 FEET. K. THE DESIGN SCOUR ELEVATION AT ALL BOARDWALK BENTS IS APPROXIMATELY 12 FEET.
- 3. CONCRETE BOARDWALKS

DRAWN BY: <u>J. I. KIMB</u>LE

CHECKED BY: J. J. PICCIRILLI

DESIGN ENGINEER OF RECORD: <u>J.C. WILSON</u>

A. FOUNDATION RECOMMENDATION FOR TIMBER BOARDWALK (SECTION 2) ARE FOR TIMBER BOARDWALK ONLY. RECOMMENDATIONS HAVE NOT BEEN DEVELOPED FOR PRECAST CONCRETE BOARDWALKS. CONTRACTOR SHALL BE RESPONSIBLE FOR ADDITIONAL GEOTECHNICAL INFORMATION AND RECOMMENDATIONS FOR TIMBER PILES IF THE PRECAST CONCRETE BOARDWALKS BID ALTERNATE IS CHOSEN.

DATE: 12/17

DATE: 12/17

DATE: 12/17

- 4. PREFABRICATED PEDESTRIAN BRIDGE
- A. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- B. HP12×53 STEEL PILES AT END BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 30 TONS PER PILE.
- DRIVE HP12x53 STEEL PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 50 TONS PER PILE.
- D. HP12×53 STEEL PILES AT END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 30 TONS PER PILE.
- E. DRIVE HP12x53 STEEL PILES AT END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 50 TONS PER PILE.
- 5. PILE SUPPORTED SLAB
 - A. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
 - B. STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES 1 THROUGH 14. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
 - C. IF NECESSARY, PREDRILL PILE LOCATIONS 1 THROUGH 14 TO ELEVATION 5 FT. WITH EQUIPMENT THAT WILL RESULT IN A MAXIMUM PREDRILLING DIAMETER OF 12". FOR PREDRILLING FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
 - D. HP12×53 PILES 1 THROUGH 8 ARE DESIGNED FOR A FACTORED RESISTANCE OF 37 TONS PER PILE.
 - E. DRIVE HP12×53 STEEL PILES 1 THROUGH 8 TO A REQUIRED DRIVING RESISTANCE OF 55 TONS PER PILE.
 - F. HP12×53 STEEL PILES 9 THROUGH 14 ARE DESIGNED FOR A FACTORED RESISTANCE OF 22 TONS PER PILE.
 - G. DRIVE HP12×53 STEEL PILES 9 THROUGH 14 TO A REQUIRED DRIVING RESISTANCE OF 35 TONS PER PILE.

TIMBER BOARDWALKS

1. TIMBER BOARDWALKS SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER BASED UPON THE SPECIFIED DESIGN CRITERIA. THE TIMBER BOARDWALK PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. CONSTRUCTION OF THE TIMBER BOARDWALK SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.

BOARDWALK STRUCTURAL TIMBER AND LUMBER

- 1. ALL TIMBER PILING, STRUCTURAL FRAMING, DECKING, NAILERS, AND PEDESTRIAN RAIL COMPONENTS SHALL BE PRESSURE TREATED SOUTHERN PINE SURFACE DRY (S4S) WITH A MOISTURE CONTENT OF 19% OR LESS, MEETING THE REQUIREMENTS OF SECTION 1082 OF THE STANDARD SPECIFICATIONS. MINIMUM GRADE SHALL BE NO. 1 DENSE FOR LUMBER 2" AND 4" THICK.LUMBER 5" AND THICKER SHALL BE MINIMUM GRADE DENSE STRUCTURAL 65.
- 2. TIMBER AND LUMBER SHALL BE TREATED WITH WATERBORNE PRESERVATIVES (CCA OR ACQ) IN ACCORDANCE WITH AWPA STANDARD U1, COMMODITY SPECIFICATION A, TO THE REQUIREMENTS OF THE FOLLOWING USE CATEGORIES:
 - A. PILES: UC4C B. BACKWALLS, WINGWALLS, CAP BEAMS AND STRINGERS: UC4B
- C. DECKING, PEDESTRIAN RAILING COMPONENTS, ALL OTHER LUMBER: UC3B
- 3. EACH DECKING MEMBER SHALL BE INSTALLED BARK SIDE UP TO PREVENT CUPPING.
- 4. MAXIMUM SPACING OF DECKING MEMBERS SHALL BE $\frac{1}{8}$ %.
- 5. ALL VERTICAL MEMBERS SHALL BE PLUMB.
- 6. ALL SAW CUTS, BOLT HOLES, AND OTHER HOLES SHALL BE TREATED WITH APPROPRIATE PRESERVATION SOLUTION PRIOR TO INSTALLING BOLTS.
- 7. UNLESS NOTED OTHERWISE, MECHANICAL WOOD CONNECTIONS SHALL BE INSTALLED PER MANUFACTURER RECOMMENDATIONS, WITH ALL FASTENER HOLES FULLY POPULATED.
- 8. ALL CONNECTORS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
- 9. ALL FASTENERS, CONNECTORS AND BOLTS SHALL BE HOT-DIP GALVANIZED AND CONFORM TO ASTM A325, WITH NUTS CONFORMING TO ASTM A563 AND WASHERS CONFORMING TO ASTM F436, UNLESS NOTED OTHERWISE.

PHASE 3B **FUTURE CONSTRUCTION**

ALL REFERENCES TO WORK OUTSIDE OF PHASE 3A (-L2- Sta. 64+68.00 TO -L4- 126+16.33) ARE NOT APPLICABLE TO THIS PROJECT.

- 1. PRECAST CONCRETE BOARDWALK SUPERSTRUCTURE, SUBSTRUCTURE, CONNECTIONS, ANCHOR BOLTS, AND BEARING PADS SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER BASED UPON THE SPECIFIED DESIGN CRITERIA. THE PRECAST CONCRETE BOARDWALK PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. THE PRECAST CONCRETE BOARDWALK PLANS CALCULATIONS SHALL INCLUDE A SUMMARY OF PILE REACTIONS. PRECAST CONCRETE BOARDWALK FABRICATION SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.
- 2. THE BOARDWALK CLEAR PATH WIDTH SHALL BE 10'-0" AND SHALL BE MEASURED BETWEEN THE INSIDE FACES OF RAILING ELEMENTS.
- 3. PEDESTRIAN RAILING SHALL BE A MINIMUM OF 4'-6"ABOVE THE TOP OF BOARDWALK
- 4. THE MINIMUM COMPRESSIVE STRENGTH OF THE CONCRETE SHALL BE 4,000 PSI MEASURED AT 28-DAYS.
- 5. CONNECTIONS FOR THE PRECAST CONCRETE BOARDWALK SHALL BE DESIGNED FOR THE APPROPRIATE LATERAL AND UPLIFT FORCES AS DETERMINED FOLLOWING THE SPECIFIED DESIGN CRITERIA.
- 6. ALL FASTENERS, CONNECTORS AND BOLTS SHALL BE HOT-DIP GALVANIZED AND CONFORM TO ASTM A325, WITH NUTS CONFORMING TO ASTM A563 AND WASHERS CONFORMING TO ASTM F436. UNLESS NOTED OTHERWISE.

CABLE STRANDS FOR PEDESTRIAN RAILS

- 1. CABLE STRANDS SHALL BE $\frac{1}{8}$ " DIAMETER 1×19 GALVANIZED STEEL.
- 2. END ANCHORAGES SHALL BE AS SHOWN ON THE PLANS.

PRECAST CONCRETE BOARDWALK

- 3. CABLES SHALL BE TENSIONED USING TURNBUCKLE ASSEMBLY. TENSION CABLES TO RESIST RAILING LOADS AS SPECIFIED IN SECTION 13 OF AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- 4. BARRIER CABLES THAT PASS THROUGH A HOLE IN THEIR ANCHORAGE SHALL HAVE THE HOLE SEALED TO PREVENT WATER FROM FOLLOWING THE PATH OF THE BARRIER CABLE TO THE ANCHORAGE.

PROJECT NO. EB-5539 PITT COUNTY N/A STATION:_

CITY OF GREENVILLE

SOUTH TAR RIVER GREENWAY, PHASE 3 GENERAL NOTES

Phone (919) 677-2000 NC LICENSE #

1/19/2018

Jeffrey C. Wilson

SHEET NO REVISIONS S-1 DATE: DATE: BY: NO. BY: TOTAL SHEETS

GENERAL NOTES

PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE

- PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE, ANCHOR BOLTS, AND BEARING PADS SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER BASED UPON THE SPECIFIED DESIGN CRITERIA. THE PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. THE PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE CALCULATIONS SHALL INCLUDE A SUMMARY OF REACTIONS FOR BENTS. THE PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE FABRICATION SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.
- 2. PREFABRICATED PEDESTRIAN BRIDGE SHALL BE A STEEL PRATT TRUSS.
- 3. THE BRIDGE CLEAR PATH WIDTH SHALL BE 10'-0", AND SHALL BE MEASURED BETWEEN THE INSIDE FACES OF SAFETY RAILING ELEMENTS.
- 4. SAFETY RAILING SYSTEM SHALL BE A MINIMUM OF 4'-6" ABOVE THE TOP OF BRIDGE DECK.
- 5. ALL STRUCTURAL STEEL FOR PREFABRICATED PEDESTRIAN BRIDGE SHALL BE WEATHERING STEEL AND SHALL CONFORM TO NCDOT STANDARD SPECIFICATIONS AND PREFABRICATED PEDESTRIAN BRIDGE SPECIAL PROVISION.
- 6. ALL STRUCTURAL STEEL WELDS SHALL CONFORM TO THE LATEST PROVISIONS OF THE STRUCTURAL WELDING CODE, ANSI/AWS D1.1. ALL WELDERS SHALL BE QUALIFIED IN ACCORDANCE WITH THE ABOVE AWS CODE.
- 7. SPLICES, IF REQUIRED FOR THE PREFABRICATED PEDESTRIAN BRIDGE SUPERSTRUCTURE, SHALL BE CLEARLY NOTED ON THE SHOP DRAWINGS, AND NECESSARY CALCULATIONS PROVIDED.
- B. WEEP HOLES SHALL BE PROVIDED FOR DRAINAGE OF BRIDGE TUBULAR MEMBERS, AND SHALL BE CLEARLY NOTED ON THE SHOP DRAWINGS.
- 9. THE BRIDGE DECK SHALL BE CONCRETE.

PREFABRICATED PEDESTRIAN BRIDGE SUBSTRUCTURE

- HE CAST-IN-PLACE CONCRETE PREFABRICATED PEDESTRIAN BRIDGE SUBSTRUCTURE SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER BASED UPON THE SPECIFIED DESIGN CRITERIA. THE PREFABRICATED PEDESTRIAN BRIDGE SUBSTRUCTURE PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. CONSTRUCTION OF THE PEDESTRIAN BRIDGE END BENTS SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.
- 2. PEDESTRIAN BRIDGE END BENT DETAILS SHALL BE COORDINATED WITH THE PREFABRICATED PEDESTRIAN BRIDGE PLANS, TO BE PROVIDED BY THE PREFABRICATED PEDESTRIAN BRIDGE MANUFACTURER. CONSTRUCTION OF THE END BENTS SHALL NOT BEGIN UNTIL SUPERSTRUCTURE SHOP DRAWINGS ARE APPROVED AND ALL CONFLICTS RESOLVED.
- 3. THE TOP SURFACE OF THE END BENT CAPS, EXCEPT AT BRIDGE SEAT BUILD-UPS AND BOARDWALK SEAT, SHALL BE SLOPED TRANSVERSELY AT A MINIMUM RATE OF 2%.
- 4. THE TOP SURFACE OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH NCDOT STANDARD SPECIFICATIONS, EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.
- 5. BACKWALL SHALL BE PLACED BEFORE APPLYING THE EPOXY PROTECTIVE COATING.
- 6. PAYMENT FOR THE EPOXY PROTECTIVE COATING SHALL BE INCIDENTAL TO THE COST OF THE PREFABRICATED PEDESTRIAN BRIDGE.

CAST-IN-PLACE CONCRETE

- 1. ALL CONCRETE SHALL BE CLASS A, UNLESS NOTED OTHERWISE.
- 2. CONCRETE WORK SHALL FOLLOW THE PROVISIONS OF SECTION 420 OF THE STANDARD SPECIFICATIONS.
- 3. CHAMFER ALL EXPOSED EDGES $\frac{3}{4}$ " UNLESS OTHERWISE NOTED.
- 4. THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

APPROACH RAILINGS

DRAWN BY: J. I. KIMBLE

CHECKED BY: J. J. PICCIRILLI

DESIGN ENGINEER OF RECORD: <u>J.C. WILSON</u>

1. TIMBER APPROACH RAILINGS SHALL BE PROVIDED AT EACH CORNER OF THE BOARDWALKS AND BRIDGE AS SHOWN ON THE PLANS. APPROACH RAILINGS SHALL BE FLARED AT 15 DEGREES, UNLESS SHOWN OTHERWISE.

RETAINING WALLS

- 1. RETAINING WALLS #1 & #2 SHALL BE DESIGNED BY THE CONTRACTOR'S ENGINEER BASED UPON THE SPECIFIED DESIGN CRITERIA. RETAINING WALL PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. PLANS AND CALCULATIONS SHALL BE SUBMITTED FOR REVIEW AND APPROVAL. CONSTRUCTION OF THE RETAINING WALLS SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.
- 2. FOR RETAINING WALL #1 DESIGN PARAMETERS AND NOTES, SEE "RETAINING WALL #1 DETAILS" SHEET 2 OF 2 (SHEET S-15).
- 3. FOR RETAINING WALL #2 DESIGN PARAMETERS AND NOTES, SEE "RETAINING WALL #2 DETAILS" SHEET (SHEET S-17).
- 4. FOR ADDITIONAL DESIGN INFORMATION, REFER TO THE FALCON ENGINEERING GEOTECHNICAL REPORT DATED MAY 12, 2015.

PILE SUPPORTED SLAB

- 1. PILE SUPPORTED SLAB DESIGN AND DETAILS ARE BASED ON DIRECTION FROM CSX THAT RIP RAP BENEATH RAILROAD BRIDGE AND IN FRONT OF ABUTMENT CAN BE REMOVED, EXPOSING FACE OF ABUTMENT, WITHOUT TEMPORARY SHORING OR IMPACTING RAILROAD BRIDGE STABILITY. FIELD MEETING OCCURRED ON MAY 12, 2014.
- 2. EXISTING GABION BASKET BETWEEN PILE LINES C & D SHALL BE REMOVED. CONTRACTOR SHOULD BE AWARE THAT LEAN CONCRETE IS PRESENT ON TOP OF RIP RAP NEAR THE PROPOSED SOUTHEAST WALL STEM.
- 3. FOR PROFILE GRADE AND SUPERELEVATION (AND SUPERELEVATION TRANSITIONS), SEE ROADWAY PLANS. FOR TOP OF SLAB ELEVATIONS AT EDGES OF SLAB ALONG PILE LINES, SEE "PILE SUPPORTED SLAB DETAILS" SHEET 1 OF 2 (SHEET S-20).
- 4. TEMPORARY SHORING ON SOUTHEAST SIDE OF RAILROAD SHALL BE A SOLDIER PILE WALL DESIGNED BY THE CONTRACTOR'S ENGINEER. PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER IN NORTH CAROLINA. SHORING SHALL BE SUBMITTED TO AND APPROVED BY CSXT PRIOR TO INSTALLATION.
- 5. THE FIRST PILE FOR THE TEMPORARY SHORING ON SOUTHEAST SIDE OF BRIDGE WALL SHALL BE DRIVEN AT A MINIMUM OF 12'-6"FROM THE CENTERLINE OF THE RAILROAD.
- 6. SHORING UNDER THE EXISTING BRIDGE IS REQUIRED FOR PROTECTION OF THE EXISTING ABUTMENT DURING CONSTRUCTION. SHORING SHALL BE DESIGNED BY CONTRACTOR'S ENGINEER. PLANS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A LICENSED PROFESSIONAL ENGINEER REGISTERED IN NORTH CAROLINA. SHORING SHALL BE SUBMITTED TO AND APPROVED BY CSXT PRIOR TO INSTALLATION.
- 7. TO AVOID DAMAGE TO EXISTING ABUTMENT DURING REMOVAL OF SHORING UNDER BRIDGE, IT IS RECOMMENDED THAT THIS SHORING REMAIN IN PLACE AFTER CONSTRUCTION IS COMPLETED.
- 8. THE TEMPORARY SHORING AT SOUTHEAST SIDE AND THE PERMANENT SHORING UNDER THE BRIDGE WILL BE PAID FOR IN THE LUMP SUM PAY ITEM FOR PILE SUPPORTED SLAB. NO SEPARATE PAYMENT WILL BE MADE.
- 9. FACE OF VERTICAL STEM SHALL BE TREATED WITH ANTI -GRAFFITI COATING AS SPECIFIED IN THE APPLICATION OF ANTI -GRAFFITI COATING SPECIAL PROVISION. PAYMENT WILL BE MADE UNDER THE LUMP SUM PAY ITEM FOR THE PILE SUPPORTED SLAB. NO SEPARATE PAYMENT SHALL BE MADE.
- 10. DETAILING REBAR SHALL BE IN ACCORDANCE WITH THE LATEST REVISION OF THE NCDOT STRUCTURE DESIGN MANUAL.ALL SHOP DRAWINGS PERTAINING TO REBAR DETAILS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW.

CONSTRUCTION

- 1. CONSTRUCTION OF THE BOARDWALK AND PEDESTRIAN BRIDGE SHALL BE PERFORMED SO AS TO NOT ALLOW DEBRIS TO FALL INTO THE WATER.
- 2. DURING CONSTRUCTION OF THE PILE SUPPORTED SLAB, THE INTEGRITY OF THE EXISTING RAILROAD BRIDGE SHALL BE VISUALLY MONITORED BY THE CONTRACTOR AND CSX. CONTRACTOR SHALL NOTIFY THE ENGINEER IMMEDIATELY IF DAMAGE IS OBSERVED.
- 3. A CSX REPRESENTATIVE WILL BE REQUIRED AS AN ON-SITE MONITOR DURING ALL CONSTRUCTION ACTIVITIES LOCATED ADJACENT TO THE EXISTING BRIDGE. CONTRACTOR SHALL COORDINATE DIRECTLY WITH CSX.
- 4. NO CONSTRUCTION EQUIPMENT OR STAGING OF MATERIALS SHALL BE PERMITTED ON THE EXISTING SLAB.

PHASE 3B FUTURE CONSTRUCTION

ALL REFERENCES TO WORK OUTSIDE OF PHASE 3A (-L2- Sta. 64+68.00 TO -L4- 126+16.33) ARE NOT APPLICABLE TO THIS PROJECT.

- 5. PROPOSED CONSTRUCTION SEQUENCE FOR PILE SUPPORTED SLAB:
 - A. INSTALL TEMPORARY SHORING ON SOUTHEAST SIDE OF THE RAILROAD TRACK.

 1. A TEMPORARY SOLDIER PILE RETAINING WALL SHALL BE INSTALLED ON THE SOUTHEAST OF THE RAILROAD. THE FIRST TEMPORARY SHORING PILE SHALL BE INSTALLED 12'-6" FROM THE CENTERLINE OF THE EXISTING RAILROAD BRIDGE AND APPROXIMATELY 2'-6" FROM THE EDGE OF THE EXISTING BRIDGE WALKWAY. THE TEMPORARY WALL WILL NOT EXTEND UNDER THE BRIDGE.
 - THE EXISTING GABION BASKETS SOUTHWEST OF THE RAILROAD WILL SERVE AS THE TEMPORARY SHORING.
 - B. ONCE THE TEMPORARY SHORING WALL AND PERMANENT SHORING BENEATH BRIDGE HAS BEEN COMPLETED AND THE EXISTING SOILS REMOVED TO BOTTOM OF SLAB ELEVATION, DRIVE THE STEEL PILES AS INDICATED IN PLANS AND SPECIFICATIONS.
 - C. CONSTRUCT THE SLAB AND TURN DOWN WALL ACCORDING TO THE PLANS AND SPECIFICATIONS.
 - D. CONSTRUCT THE STEM ACCORDING TO THE PLANS AND SPECIFICATIONS.
 - E. FILL THE VOID BEHIND THE STEM AND TEMPORARY SHORING/EXISTING ABUTMENT WITH #57 STONE.
 - F. INSTALL TIMBER CANOPIES AS INDICATED ON THE PLANS.
- 6. CONTRACTOR MUST PLAN AND PERFORM THE WORK IN A MANNER SUCH THAT THE CSXT TRACKS AT THE PROJECT LOCATION REMAIN FULLY CAPABLE OF CARRYING RAIL TRAFFIC THROUGHOUT THE WORK PERIOD AND RAIL TRAFFIC IS NOT DELAYED OR OTHERWISE IMPACTED DUE TO THE WORK BEING PERFORMED. THERE WILL BE NO TEMPORARY AT-GRADE CROSSINGS ALLOWED.

PILE DRIVING PLAN

- 1. CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLETING A PILE DRIVING PLAN FOR THE PILE SUPPORTED SLAB AT THE EXISTING RAILROAD TRACK.
- 2. PILE DRIVING PLAN MUST BE SUBMITTED TO AND APPROVED BY CSX TRANSPORTATION PRIOR TO THE INSTALLATION OF ANY PILES. AN APPROVED COPY OF THE PILE DRIVING PLAN SHALL BE SUBMITTED TO THE ENGINEER.
- 3. PILE DRIVING PLAN SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA.
- 4. AT A MINIMUM THE PILE DRIVING PLANS SHALL INCLUDE THE FOLLOWING FOR EACH PILE INSTALLED:
 - A. CRANE SIZE AND PILE DRIVING HAMMER USED DURING PILE INSTALLATION.

 B. LOCATION OF PROPOSED PILES IN RELATION TO THE CENTERLINE OF EXISTING
 - RAILROAD TRACK AND RIGHT OF WAY.

 C. LOCATION OF PILE INSTALLATION EQUIPMENT (CRANE, ETC.) IN RELATION TO
 - THE CENTERLINE OF EXISTING RAILROAD TRACK.

 D. CRANE PICK RADIUS IN RELATION TO THE CENTERLINE OF EXISTING RAILROAD
 - TRACK.
 E. TOTAL WEIGHT OF ALL PICKS NECESSARY TO INSTALL THE PILES.
- 5. RAILROAD FLAGGERS WILL BE REQUIRED FOR CONSTRUCTION ACTIVITIES AROUND RAILROAD. CONTRACTOR SHALL COORDINATE WITH RAILROAD AND FLAGGERS SHALL BE PROVIDED BY CSX AT NO ADDITIONAL COST TO THE CITY.
- 6. PILE DRIVING PLAN SHALL BE INCLUDED IN THE LUMP SUM PAY ITEM FOR THE PILE SUPPORTED SLAB AND NO SEPARATE PAYMENT WILL BE MADE.
- 7. FOR PILE DRIVING PLAN, SEE PILE SUPPORTED SLAB SPECIAL PROVISION.

PROJECT NO. <u>EB-5539</u>

PITT COUNTY

STATION: N/A

SEAL
040384

DocuSigned by:

Jeffrey C. Wilson

CDA045FAFCC9416...

CITY OF GREENVILLE

SOUTH TAR RIVER GREENWAY, PHASE 3

GENERAL NOTES

421 Fayetteville Street, Suite 600
Raleigh, NC 27601-1772
Phone (919) 677-2000

NC LICENSE #
F-0102

The species is integred only for the specific purpose and client for

REVISIONS

BY: DATE: NO. BY: DATE: S-2

3 TOTAL SHEETS
24

DATE: 12/17 DATE: 12/17 DATE: 12/17

TIMBER CANOPY NOTES

GENERAL NOTES

- 1. FOR TIMBER CANOPY. SEE SPECIAL PROVISIONS.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. ALL APPLICABLE SAFETY REGULATIONS TO BE FOLLOWED STRICTLY.
- THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETED STRUCTURE. APPLICATIONS OF CONSTRUCTION LOADS TO THE PARTIALLY COMPLETED STRUCTURE SHALL BE CONSIDERED BY THE CONTRACTOR AND SO INCLUDED IN THE DESIGN OF SHORING, BRACING, FORMWORK, AND ANY OTHER SUPPORTING ELEMENTS PROVIDED FOR CONSTRUCTION OF THE STRUCTURE. DURING ERECTION AND UNTIL ALL PERMANENT CONNECTIONS ARE MADE, THE CONTRACTOR MUST PROVIDE TEMPORARY BRACING FOR THE STRUCTURE IN ALL DIRECTIONS.
- THE GENERAL CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND GRADE CONDITIONS (BOTH NEW AND EXISTING), REPORTING ANY DISCREPANCIES PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH ANY PHASE OF THE WORK.
- 5. DO NOT SCALE DIMENSIONS FROM DRAWINGS. THE CONTRACTOR SHALL REQUEST FROM THE ENGINEER NECESSARY DIMENSIONS NOT SHOWN ON THE DRAWINGS.
- IF ANY BIDDER IS IN DOUBT AS TO THE INTENT OF THE PLANS OR SPECIFICATIONS, THEY SHALL REQUEST AN INTERPRETATION FROM THE ENGINEER IN WRITING AT LEAST TEN (10) DAYS PRIOR TO THE SCHEDULED BID DATE.
- 7. WHERE A CONFLICT BETWEEN DRAWINGS AND SPECIFICATIONS OCCUR THE MORE-STRINGENT REQUIREMENT SHALL APPLY.
- 8. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL APPLY FOR ALL LIKE OR SIMILAR CONDITIONS EVEN THOUGH NOT SPECIFICALLY REFERENCED ON THE DRAWINGS.

DESIGN CRITERIA

- 1. APPLICABLE CODES
 - A. 2012 NORTH CAROLINA STATE BUILDING CODE
 - (2009 INTERNATIONAL BUILDING CODE WITH REVISIONS)
 - B. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE 7-05)

C. NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION (NDS 2010)

- 2. LIVE LOAD:
- ROOF.....20 PSF
- 3. SNOW LOAD:
 - GROUND SNOW LOAD...10 PSF
- 4. WIND LOAD:
 - BASIC DESIGN WIND VELOCITY....110 MPH OCCUPANCY CLASSIFICATION.....CATEGORY II EXPOSURE CATEGORY.....C WIND IMPORTANCE FACTOR Iw....1.0 DESIGNED AS OPEN ROOF SYSTEM
 - COMPONENTS AND CLADDING: ALL BUILDING COMPONENTS AND CLADDING ENGINEERED BY THE COMPONENT MANUFACTURER ARE TO BE DESIGNED BY THE MANUFACTURER'S ENGINEER FOR WIND LOADS DETERMINED PER THE NORTH CAROLINA STATE BUILDING CODE FOR THE BASIC DESIGN WIND VELOCITY. IMPORTANCE FACTOR AND EXPOSURE LISTED ABOVE.
- 5. FUTURE LOADS: UNLESS SPECIFICALLY NOTED. THERE ARE NO PROVISIONS MADE FOR FUTURE FLOORS, ROOFS, OR OTHER LOADS.

LUMBER

- 1. ALL LUMBER AND ITS FASTENINGS SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, LATEST EDITION, BY THE AMERICAN FOREST AND PAPER ASSOCIATION.
- 2. ALL LUMBER SHALL BE PRESSURE TREATED SOUTHERN PINE GRADE NO.1 DENSE.
- 3. TIMBER AND LUMBER SHALL BE TREATED WITH WATERBORNE PRESERVATIVES (CCA OR ACQ) IN ACCORDANCE WITH AWPA STANDARD U1, COMMODITY SPECIFICATION A, TO THE REQUIREMENTS OF THE FOLLOWING USE CATEGORIES: A. TIMBER CANOPY LUMBER: UC3B
- 4. STRUCTURAL MEMBERS SHALL NOT BE CUT FOR PIPES, DUCTWORK, ETC. UNLESS SPECIFICALLY NOTED OR DETAILED.
- 5. HOLES FOR BOLTS SHALL BE BORED $\frac{1}{32}$ " TO $\frac{1}{16}$ " LARGER THAN THE NOMINAL BOLT DIAMETER.
- 6. ALL BOLTS SHALL BE RE-TIGHTENED PRIOR TO APPLICATION TO PLYWOOD, ETC.
- 7. ALL BOLTS BEARING ON WOOD SHALL HAVE WASHERS UNDER HEAD AND/OR NUT.
- 2× SOLID BLOCKING SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS, ENDS OF CANTILEVERS, AND HALFWAY BETWEEN SUPPORTS. CROSS BRIDGING OR SOLID BLOCKING SHALL BE PROVIDED AT 8'-0"O.C. MAXIMUM.
- 9. ALL SAW CUTS, BOLT HOLES, AND OTHER HOLES SHALL BE TREATED WITH APPROPRIATE PRESERVATION SOLUTION PRIOR TO INSTALLING BOLTS.
- 10. UNLESS NOTED OTHERWISE, MECHANICAL WOOD CONNECTIONS SHALL BE INSTALLED PER MANUFACTURER RECOMMENDATIONS, WITH ALL FASTENER HOLES FULLY POPULATED.
- 11. ALL CONNECTORS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
- 12. ALL NAILS SHALL BE GALVANIZED UNLESS NOTED OTHERWISE.
- 13. STANDARD WOOD CONNECTORS MUST BE PROVIDED BY THE GENERAL CONTRACTOR FOR WOOD FRAMED MEMBERS.INTERIOR FRAMING CONNECTORS MUST BE G90 GALVANIZED ZINC CONNECTORS-EXTERIOR FRAMING CONNECTORS MUST BE 0155 GALVANIZED ZINC COATING, MINIMUM.
- 14. ADHESIVES SHALL MEET THE REQUIREMENTS FOR WET CONDITIONS OF SERVICE.

PRE-FABRICATED WOOD TRUSSES

- 1. ALL LUMBER AND FASTENERS SHALL CONFORM TO THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION, LATEST EDITION, BY THE AMERICAN FOREST AND PAPER ASSOCIATION. CONFORM TO APPLICABLE PROVISIONS OF TPI DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES (LATEST EDITION).
- 2. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:

TOP CHORD: LL=20 PSF AT ROOF DL=10 PSF AT ROOF BOTTOM CHORD: DL=10 PSF

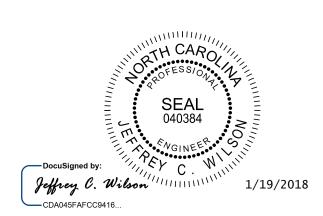
- TRUSS SUPPLIER SHALL CALCULATE UPLIFT LOADS BASED ON THE WIND LOAD CRITERIA LISTED IN THESE GENERAL NOTES.AT A MINIMUM THE NET WIND UPLFIT LOAD SHALL BE 25 PSF.
- 4. TRUSS CHORDS AND WEBS SHALL BE SOUTHERN PINE, PS 20, GRADED TO NFPA RULES: MAXIMUM MOISTURE CONTENT - 19% MINIMUM GRADE OF CHORD - NO. 2 MINIMUM GRADE OF WEB MEMBERS - NO. 2
- 5. ALL TRUSSES SHALL BE DESIGNED FOR THE ACTUAL DEAD LOAD PLUS LIVE LOAD (SPECIFIED ABOVE). MAXIMUM DEFLECTION DUE TO LIVE LOAD ONLY SHALL NOT EXCEED L/360. MAXIMUM DEFLECTION DUE TO TOTAL LOAD SHALL NOT EXCEED L/240. ROOF SLOPE SHALL BE 1/4" (PER FOOT OR GREATER AFTER LONG TERM DEFLECTION OCCURS.
- SUBMIT SHOP DRAWINGS FOR ALL TRUSSES. SHOP DRAWINGS SHALL INDICATE PLACING OF ALL FRAMING MEMBERS SHOWING TYPE, SIZE, NUMBER, LOCATION AND SPACING. THEY SHALL ALSO INDICATE SUPPLEMENTAL BRACING, SPLICES, BRIDGING, ACCESSORIES AND DETAILS REQUIRED FOR PROPER INSTALLATION. SHOP DRAWINGS' SUBMITTAL MUST BE PREPARED UNDER THE SUPERVISION OF AND SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA.
- 7. ALL TRUSSES AND CONNECTIONS SHALL BE DESIGNED BY THE SUPPLIER'S ENGINEER. SUBMIT CALCULATIONS FOR ALL TRUSSES AND THEIR CONNECTIONS THAT ARE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA.
- CONSTRUCTION OF THE TIMBER CANOPIES SHALL NOT BEGIN UNTIL ALL APPROVALS HAVE BEEN RECEIVED.
- TEMPORARY BRACING, WHERE REQUIRED, SHALL BE PROVIDED UNTIL THE ERECTION IS COMPLETE.

PLYWOOD

- 1. ALL PLYWOOD SHEATHING AND DIAPHRAGMS SHALL CONFORM TO U.S. PRODUCT STANDARD PS 1-95 WITH EXTERIOR GLUE.
- 2. PLYWOOD SHEETS SHALL BE LAID WITH LONG DIMENSION PERPENDICULAR TO THE SUPPORTING FRAMING.
- 3. PLYWOOD ROOF SHEATHING SHALL BE $\frac{3}{4}$ "CDX (INDEX 40/24) AND SHALL BE FASTENED WITH #6 SCREWS SPACED @ 12"O.C. AT ALL EDGES AND 10d NAILS @ 12"O.C. IN PANEL FIELD. PROVIDE 2×4 BLOCKING OR CLIPS AT MIDSPAN AND AT ALL PANEL EDGES.

EB-5539 PROJECT NO.___ PITI COUNTY N/A STATION:_

SHEET 3 OF 4



SOUTH TAR RIVER

CITY OF GREENVILLE

GREENWAY, PHASE 3 GENERAL NOTES

Phone (919) 677-2000 NC LICENSE # F-0102

SHEET N REVISIONS S-3 DATE: DATE: 10. BY: BY: TOTAL SHEETS

DRAWN BY: J.I.KIMBLE DATE: 12/17 CHECKED BY: J.J.PICCIRILLI DATE: 12/17 DESIGN ENGINEER OF RECORD: <u>J.C.WILSON</u> DATE: 12/17

- LUMP SUM PAY ITEMS FOR TIMBER BOARDWALK SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, EQUIPMENT, DELIVERY AND OTHER INCIDENTALS NECESSARY FOR THE TIMBER BOARDWALK, APPROACH RAILINGS, APPROACH SLABS, APPROACH FILL, DEWATERING, ETC.
- 2 LUMP SUM PAY ITEMS FOR PREFABRICATED PEDESTRIAN BRIDGE SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, EQUIPMENT, DELIVERY, AND OTHER INCIDENTALS NECESSARY FOR THE PREFABRICATED STEEL PRATT TRUSS PEDESTRIAN BRIDGE, CAST-IN-PLACE CONCRETE SUBSTRUCTURE, PILES, BEARING PADS, ANCHOR BOLTS, EPOXY PROTECTIVE COATING, APPROACH RAILINGS, APPROACH SLABS, APPROACH FILL, DEWATERING, ETC.
- 3 LUMP SUM PAY ITEM FOR RETAINING WALLS SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, EQUIPMENT, DELIVERY, AND OTHER INCIDENTALS NECESSARY FOR MSE AND SOLDIER PILE RETAINING WALLS.LUMP SUM PAYMENT FOR THIS ITEM SHALL INCLUDE APPLICATION OF BRIDGE (ANTI-GRAFFITI) COATING.
- 4 LUMP SUM PAY ITEM FOR PILE SUPPORTED SLAB SHALL INCLUDE SUBMITTALS, LABOR, MATERIALS, EQUIPMENT, DELIVERY AND OTHER INCIDENTALS NECESSARY FOR THE PILE SUPPORTED SLAB, TEMPORARY AND PERMANENT SHORING WALLS, RIP RAP ON RIVER BANK, GEOTEXTIAL FABRIC ON RIVER BAND, #57 STONE, ANTI-GRAFFITI COATING, DEWATERING, UTILITY LOCATION, STRUCTURAL MONITORING, WATERPROOF SEALANT, BOND BREAKER, JOINT MATERIAL, ETC.
- 5 LUMP SUM PAY ITEM FOR TIMBER CANOPIES SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, DELIVERY, AND OTHER INCIDENTALS NECESSARY FOR (4) TIMBER CANOPIES INCLUDING CHAIN LINK FENCE MOUNTED TO RETAINING WALL SYSTEM.
- 6 BID ALTERNATE SHALL BE FOR THE SUBSTITUTION OF TIMBER BOARDWALK WITH PRECAST CONCRETE BOARDWALK WITH METAL RAILING FOR THE REFERENCED STRUCTURES.LUMP SUM PAY ITEMS FOR PRECAST CONCRETE BOARDWALK WITH METAL RAILINGS SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, EQUIPMENT, DELIVERY AND OTHER INCIDENTALS NECESSARY FOR THE PRECAST CONCRETE BOARD, METAL AND CABLE RAILING SYSTEM, BEARING PADS, ANCHOR BOLTS, APPROACH RAILINGS, APPROACH SLABS, APPROACH FILL, ETC.
- 7 BID ALTERNATE SHALL BE FOR THE SUBSTITUTION OF TIMBER BOARDWALK WITH PRECAST CONCRETE BOARDWALK WITH TIMBER RAILING FOR THE REFERENCED STRUCTURES.LUMP SUM PAY ITEMS FOR PRECAST CONCRETE BOARDWALK WITH TIMBER RAILINGS SHALL INCLUDE ALL ENGINEERING, LABOR, MATERIALS, EQUIPMENT, DELIVERY AND OTHER INCIDENTALS NECESSARY FOR THE PRECAST CONCRETE BOARD, TIMBER AND CABLE RAILING SYSTEM, BEARING PADS, ANCHOR BOLTS, APPROACH RAILINGS, APPROACH SLABS, APPROACH FILL, ETC.

PHASE 3B **FUTURE CONSTRUCTION**

ALL REFERENCES TO WORK OUTSIDE OF PHASE 3A (-L2- Sta. 64+68.00 TO -L4- 126+16.33) ARE NOT APPLICABLE TO THIS PROJECT.

> PROJECT NO. EB-5539 PITT COUNTY N/A STATION:_



Phone (919) 677-2000 NC LICENSE # F-0102

SOUTH TAR RIVER GREENWAY, PHASE 3

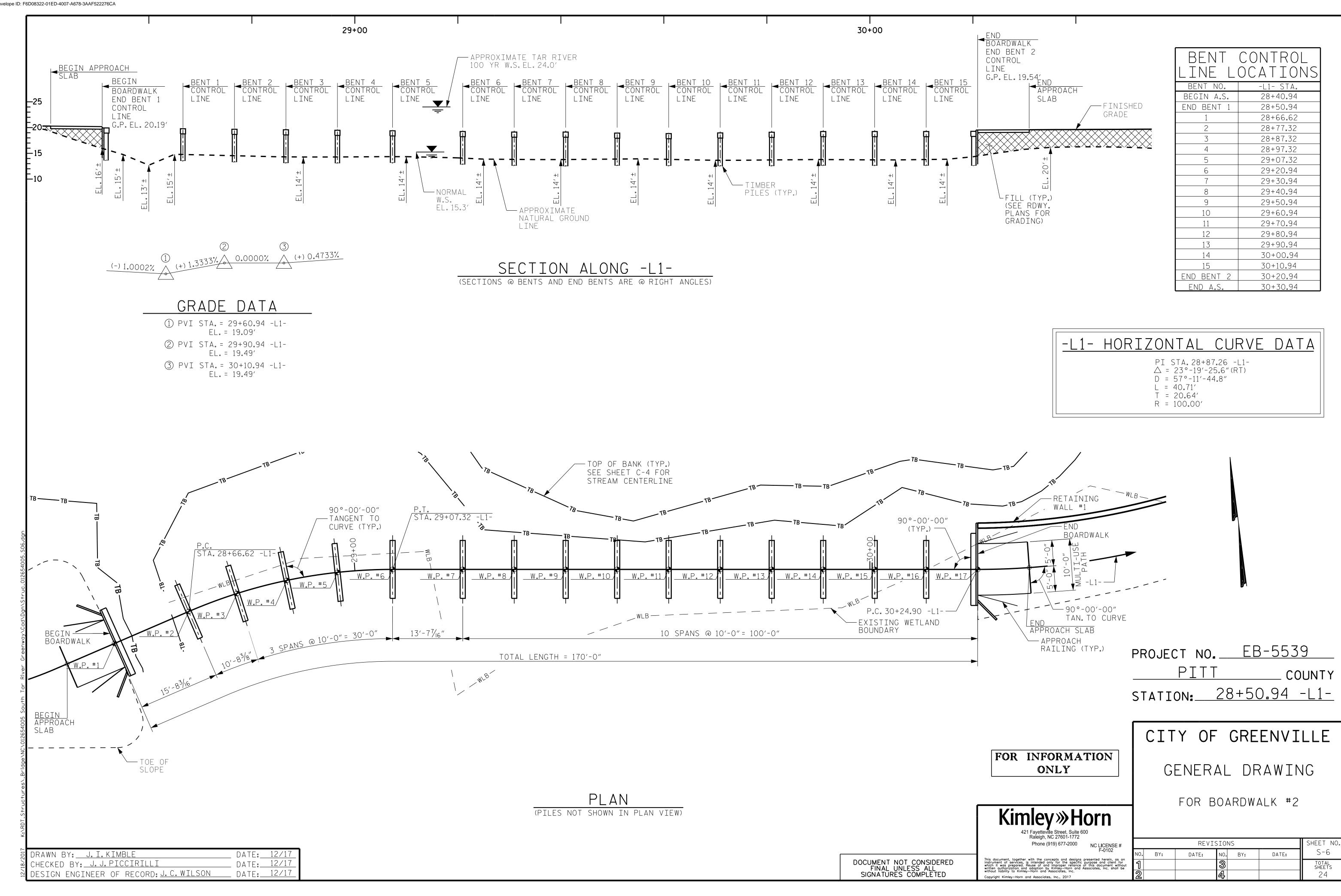
CITY OF GREENVILLE

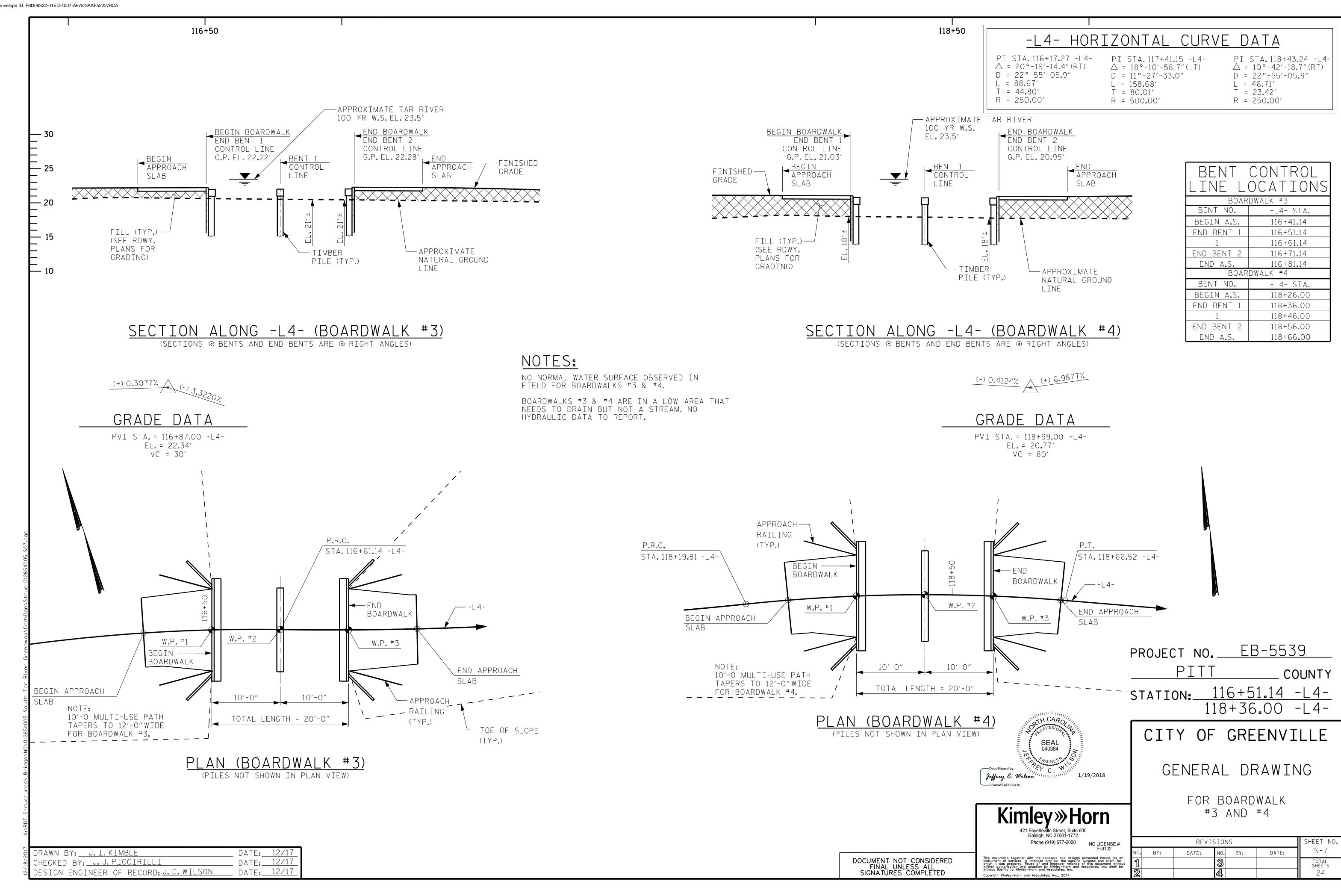
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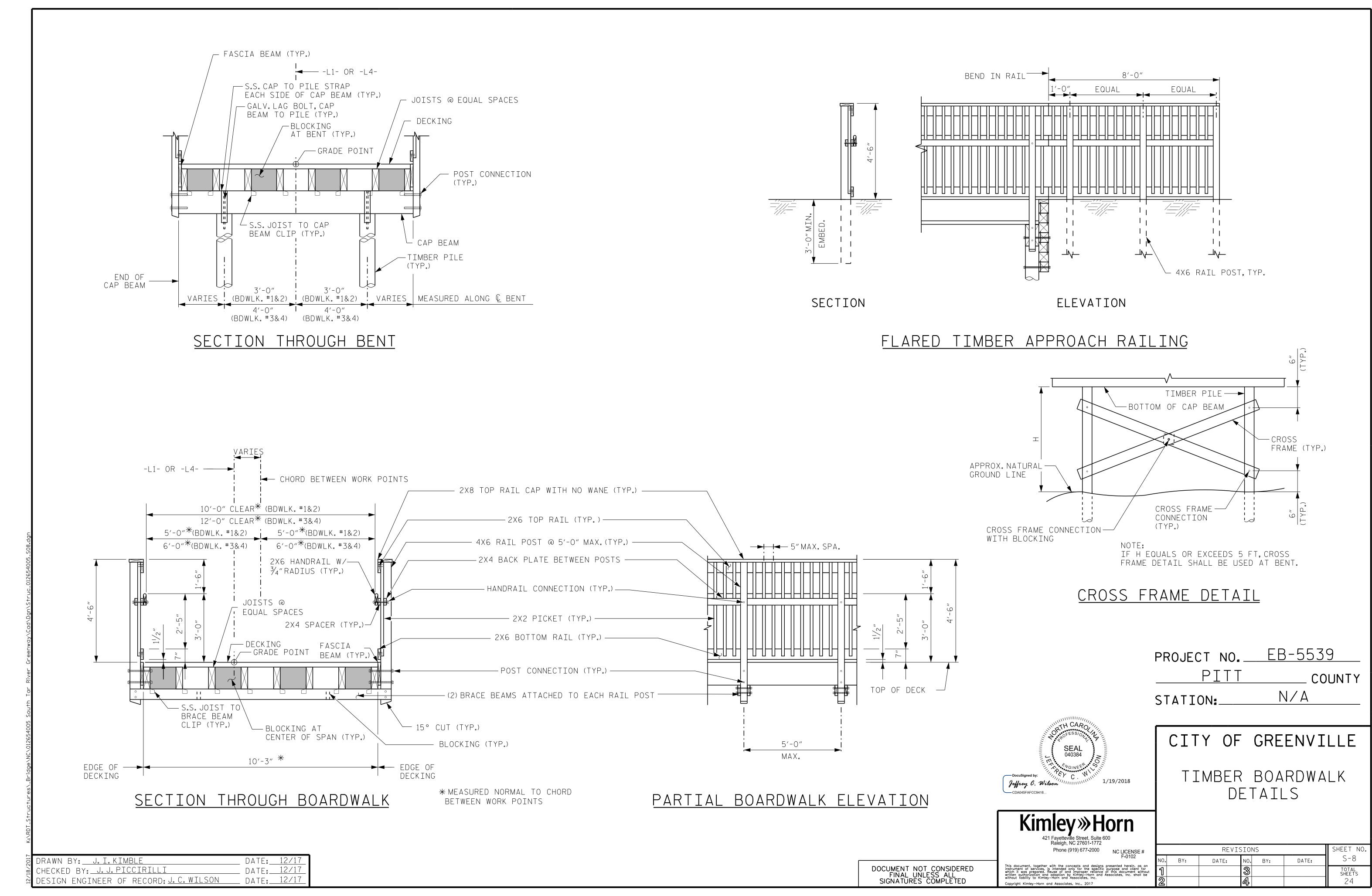
SHEET NO REVISIONS S-4 DATE: 10. BY: DATE: BY: TOTAL SHEETS

DRAWN BY: <u>J.I.KIMBLE</u> DATE:<u>12/17</u> DATE: 12/17 CHECKED BY: J. J. PICCIRILLI DESIGN ENGINEER OF RECORD: J. C. WILSON DATE: 12/17

ESIGN ENGINEER OF RECORD: J. C. WILSON

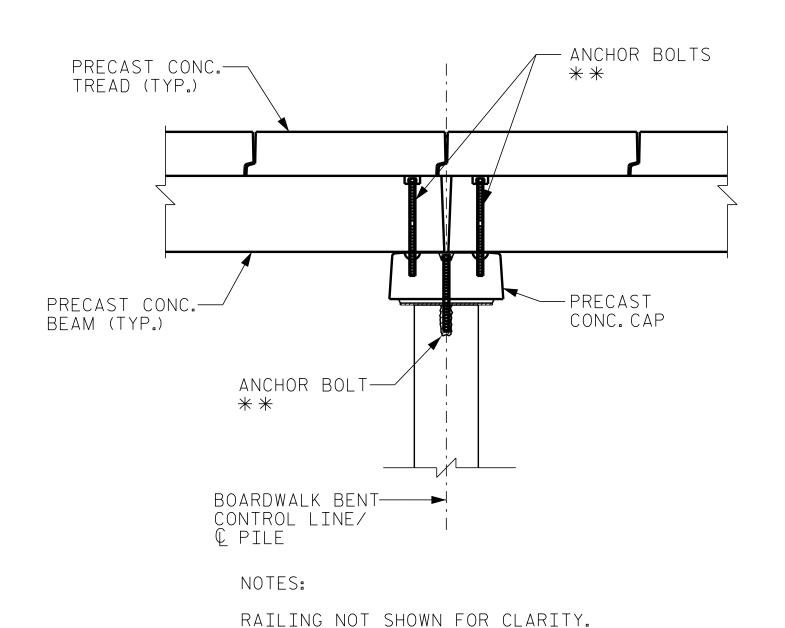






NOTES

* * REFER TO PRECAST CONCRETE BOARDWALK SYSTEM PLANS (BY OTHERS) FOR ANCHOR BOLT PLACEMENT AND DETAILS, AND FOR DIMENSIONS NOT SHOWN.



BEAM TO PILE CONNECTION

UPLIFT CONNECTION **

(BY OTHERS)

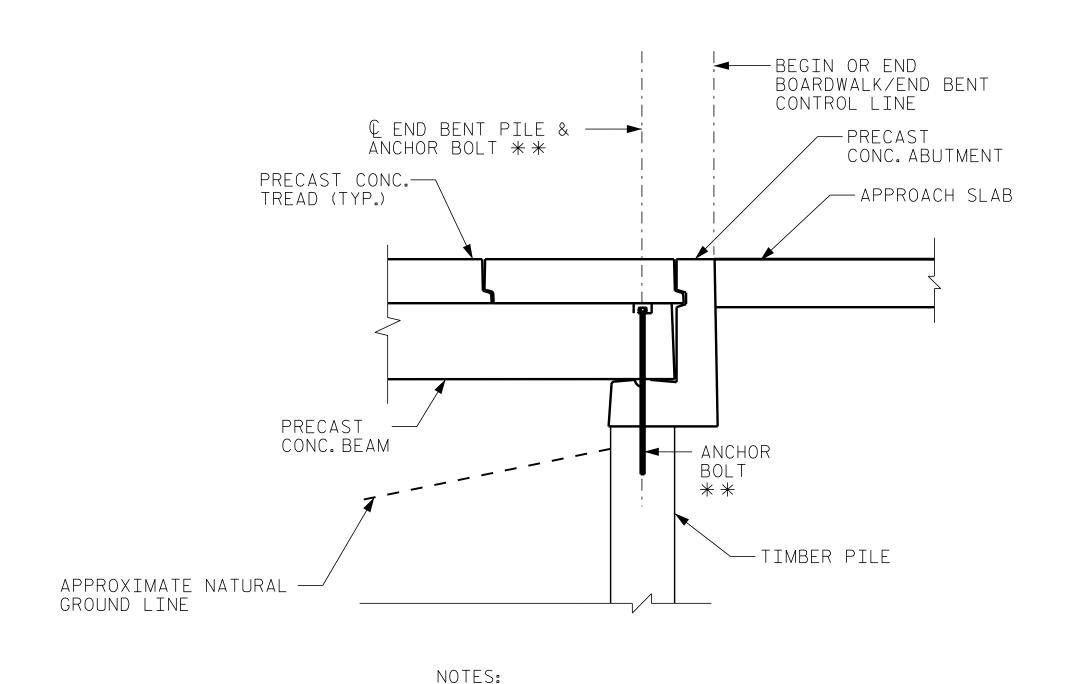
PRECAST
CONC. TREAD

PRECAST
CONC. BEAM

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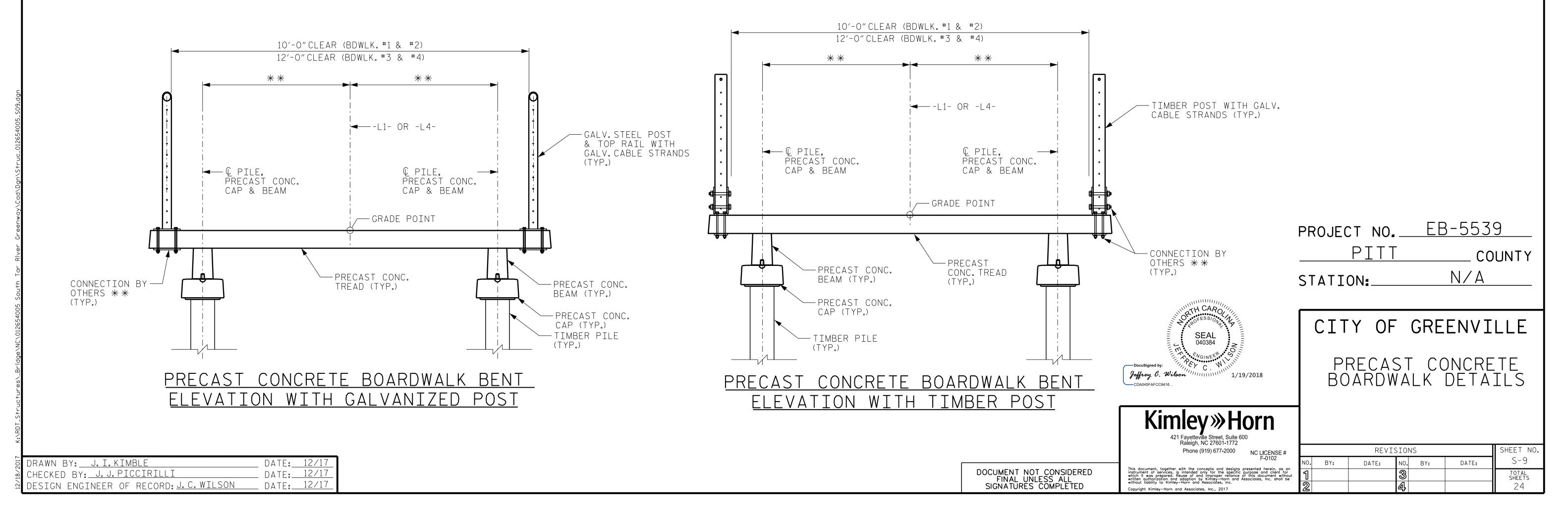
TYPICAL TREAD TO BEAM CONNECTION

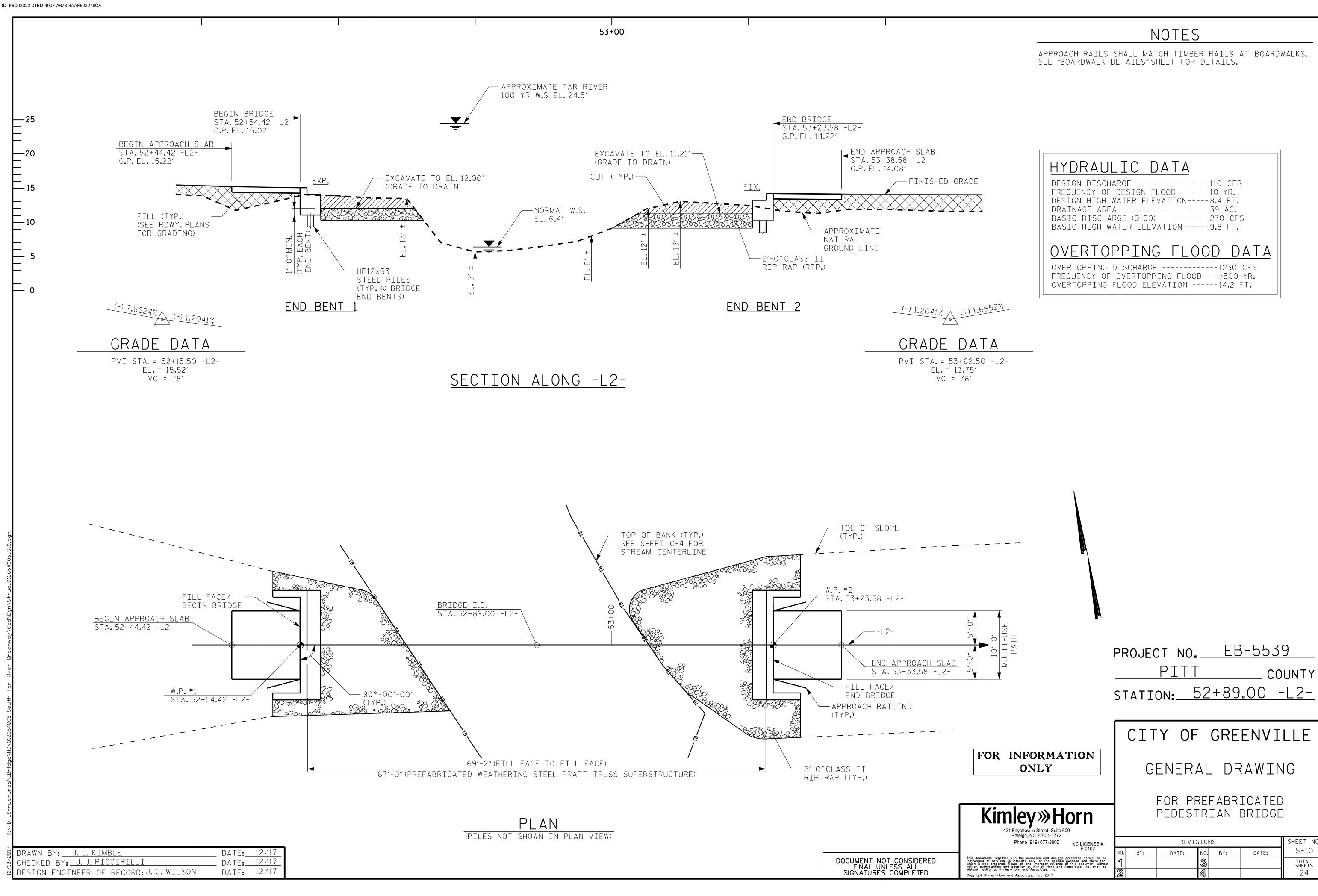
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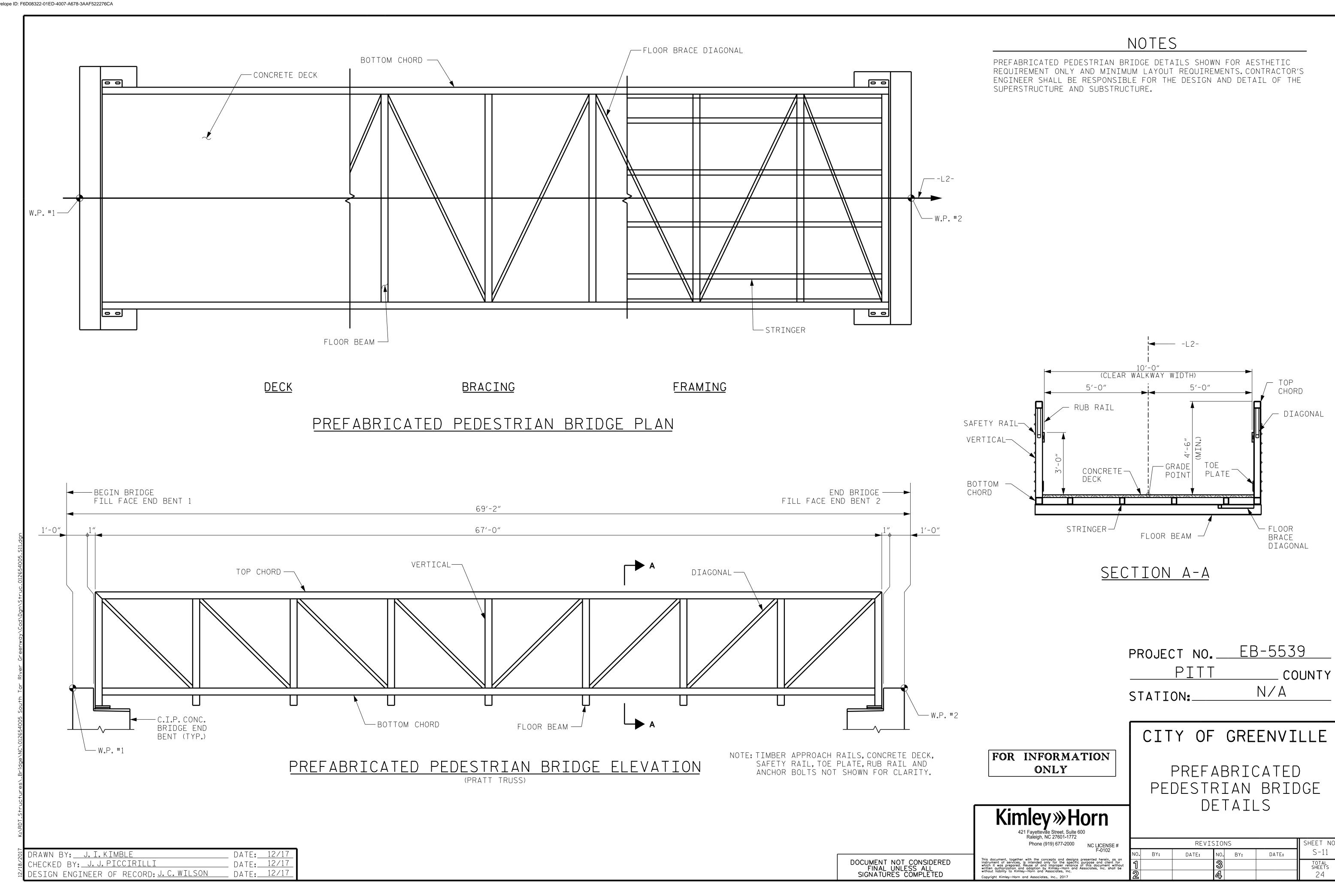


RAILING NOT SHOWN FOR CLARITY.

ABUTMENT TO PILE CONNECTION

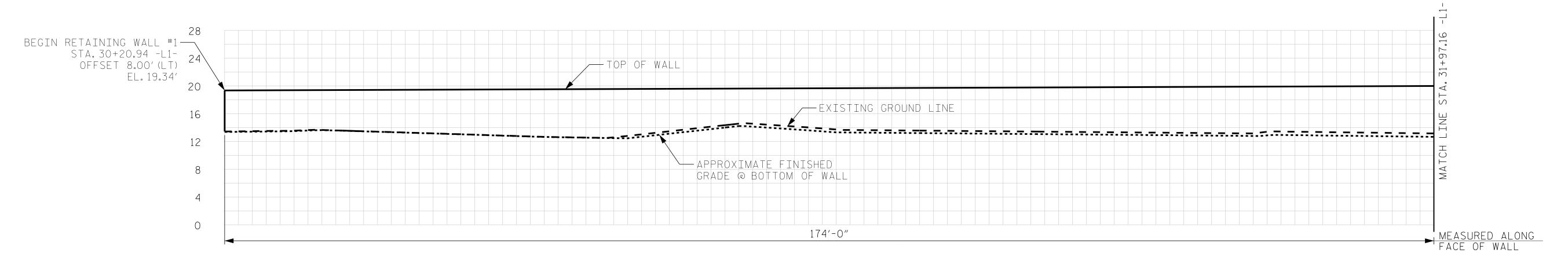




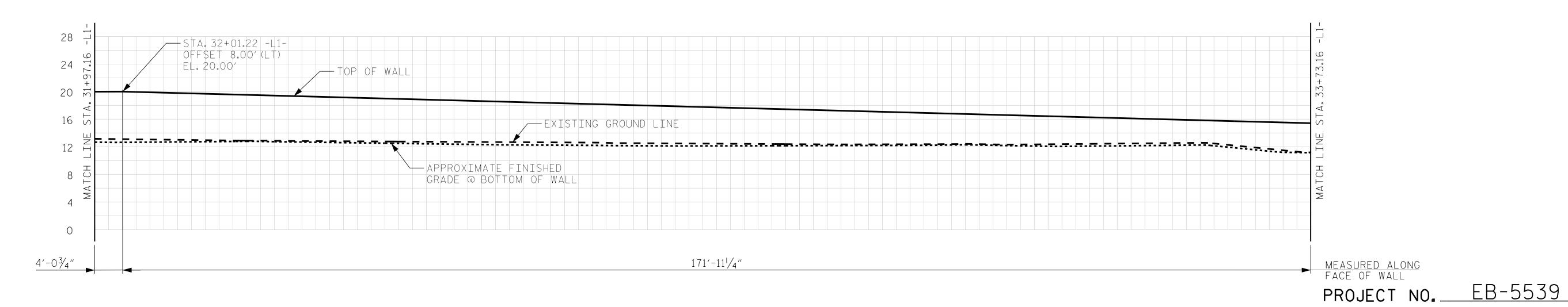




FOR PLAN VIEW OF RETAINING WALL, SEE ROADWAY PLANS. FOR RETAINING WALL TYPICAL SECTION AND NOTES, SEE "RETAINING WALL #1 DETAILS" SHEET.



RETAINING WALL #1 ENVELOPE



RETAINING WALL #1 ENVELOPE

FOR INFORMATION ONLY

CITY OF GREENVILLE

STATION: 30+20.94 -L1-

COUNTY

RETAINING WALL #1 ENVELOPE

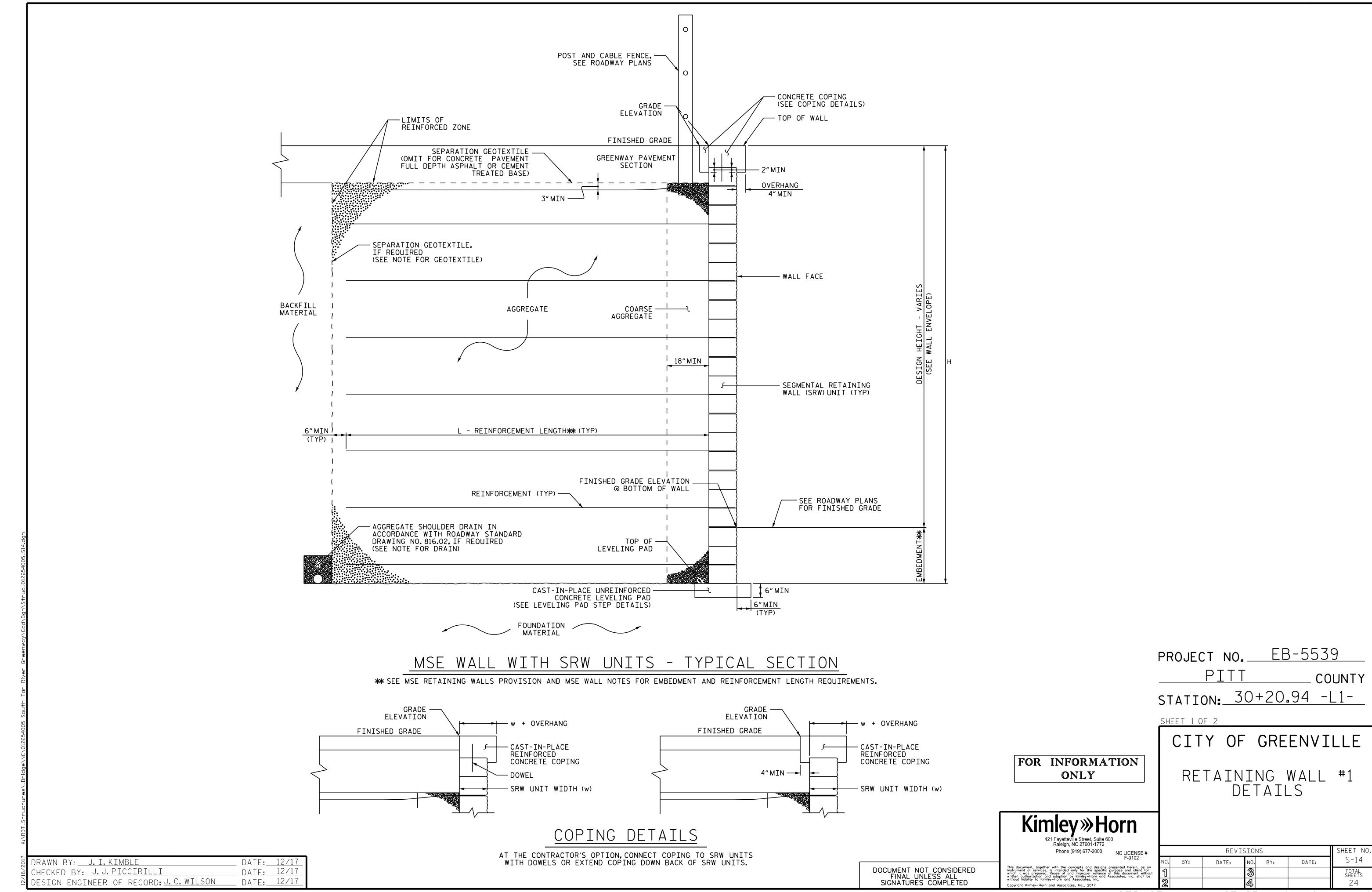
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SHEET NO REVISIONS S-12 DATE: DATE: NO. BY: TOTAL SHEETS

DRAWN BY: <u>J.I.KIMBL</u> DATE: 12/17 DATE: 12/17 CHECKED BY: J.J.PICCIRILLI DESIGN ENGINEER OF RECORD: J.C.WILSON

NOTES SEE SHEET 1 OF 2 FOR NOTES. 28 STA. 35+05.88 -L1-OFFSET 8.00 (LT) 24 — STA. 33+90.43 -L1-OFFSET 8.00'(LT) EL. 17.35′ TOP OF WALL 20 EL. 14.95′ ---EXISTING GROUND LINE - APPROXIMATE FINISHED GRADE @ BOTTOM OF WALL 115′-5³⁄8″ 47′-35/16″ 17′-35/₁₆″ MEASURED ALONG FACE OF WALL RETAINING WALL #1 ENVELOPE 28 — END RETAINING WALL #1 STA. 37+40.98 -L1-24 STA. 36+08.89 -L1-OFFSET 8.00'(LT) OFFSET 8.00 (LT) TOP OF WALL EL. 21.39' EL.16.39′ 20 - APPROXIMATE FINISHED GRADE @ BOTTOM OF WALL 56′-8⁵⁄₁₆″ 135′-1⁵⁄₁₆″ MEASURED ALONG FACE OF WALL PROJECT NO. EB-5539 RETAINING WALL #1 ENVELOPE COUNTY STATION: 30+20.94 -L1-CITY OF GREENVILLE FOR INFORMATION RETAINING WALL #1 ENVELOPE ONLY SHEET NO REVISIONS S-13 DRAWN BY: <u>J.I.KIMBL</u> DATE: 12/17 DATE: DATE: NO. BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED CHECKED BY: J.J.PICCIRILLI TOTAL SHEETS

DATE: 12/17 DATE: 12/17 DESIGN ENGINEER OF RECORD: J. C. WILSON



NOTES:

FOR MECHANICALLY STABILIZED EARTH (MSE) RETAINING WALLS, SEE MECHANICALLY STABILIZED EARTH RETAINING WALLS PROVISION.

USE AN MSE WALL SYSTEM WITH SEGMENTAL RETAINING WALL UNITS (SRW) UNITS THAT MEET ARTICLE 1040-4 OF THE STANDARD SPECIFICATIONS FOR RETAINING WALL NO.1.

- AT THE CONTRACTOR'S OPTION, USE FINE AGGREGATE IN THE REINFORCED ZONE OF RETAINING WALL NO.1.
- A SEPARATION GEOTEXTILE IS NOT REQUIRED AT THE BACK OF THE REINFORCED ZONE FOR RETAINING WALL NO.1.
- A DRAIN IS REQUIRED FOR RETAINING WALL NO.1.

BEFORE BEGINNING MSE WALL DESIGN FOR RETAINING WALL NO.1, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL NO. 1 FOR THE FOLLOWING:

- 1) H = DESIGN HEIGHT + EMBEDMENT
- 2) DESIGN LIFE = 75 YEARS

MATERIAL REQUIREMENTS.

- 3) MAXIMUM FACTORED VERTICAL PRESSURE ON FOUNDATION MATERIAL = 2,000 LB/SF
- 4) MINIMUM REINFORCEMENT LENGTH (L) = .7H OR 6 FT, WHICHEVER IS LONGER
- 5) MINIMUM REINFORCEMENT LENGTH (L) = 14 FT FOR FINAL LAYER OF REINFORCEMENT 6) MINIMUM EMBEDMENT ELEVATION = 10 FT OR DEPTH = 2 FT (WHICHEVER IS DEEPER)
- 7) REINFORCED ZONE AGGREGATE PARAMETERS:

AGGREGATE TYPE*	UNIT WEIGHT (Y) LB/CF	FRICTION ANGLE (Ø) DEGREES	COHESION (c) LB/SF			
COARSE	110	38	0			
FINE	115	34	0			
* SEE MSE RETAINING WALLS PROVISION FOR COARSE AND FINE AGGREGATE						

8) IN-SITU ASSUMED MATERIAL PARAMETERS:

MATERIAL TYPE	UNIT WEIGHT (Y) LB/CF	FRICTION ANGLE (Ø) Degrees	COHESION (c) LB/SF
BACKFILL	120	30	0
FOUNDATION	110	28	0

DESIGN RETAINING WALL NO.1 FOR A LIVE LOAD (TRAFFIC) SURCHARGE.

EXISTING OR FUTURE OBSTRUCTIONS SUCH AS FOUNDATIONS, GUARDRAIL, FENCE OR HANDRAIL POSTS, PAVEMENTS, PIPES, INLETS OR UTILITIES WILL INTERFERE WITH REINFORCEMENT FOR RETAINING WALL NO.1.

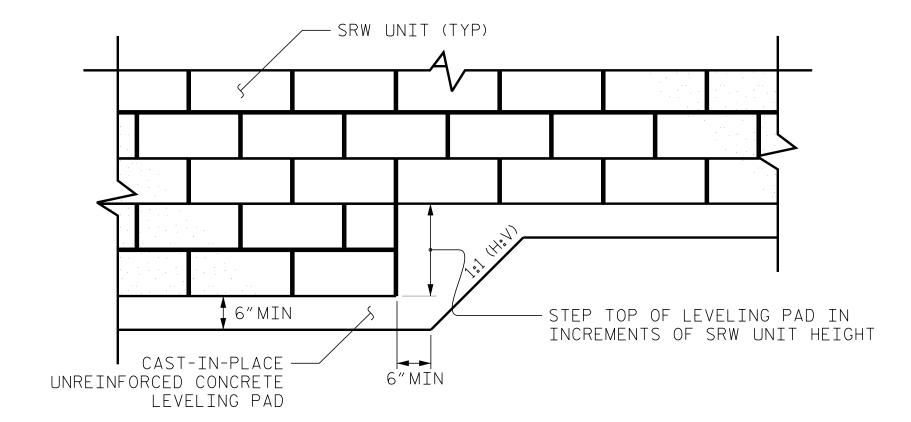
FOUNDATIONS FOR BOARDWALK SECTION 2 BENT 16 LOCATED AT STATION 30+20.94 MAY INTERFERE WITH REINFORCEMENT FOR RETAINING WALL NO.1. SEE "FOUNDATION LAYOUT" SHEET FOR FOUNDATION LOCATIONS.

DO NOT PLACE LEVELING PAD CONCRETE, AGGREGATE OR REINFORCEMENT FOR RETAINING WALL NO.1 UNTIL EXCAVATION DIMENSIONS AND FOUNDATION MATERIAL ARE APPROVED.

FACE OF RETAINING WALL SHALL BE TREATED WITH ANTI-GRAFFITI COATING AS SPECIFIED IN THE APPLICATION OF BRIDGE COATING SPECIAL PROVISION. PAYMENT WILL BE MADE UNDER THE RETAINING WALL #1 PAY ITEM AND NO SEPARATE PAYMENT WILL BE MADE.

FOR UNDERCUT AND DESIGN REQUIREMENTS, REFER TO THE FALCON ENGINEERING GEOTECHNICAL REPORT DATED MAY 12, 2015.

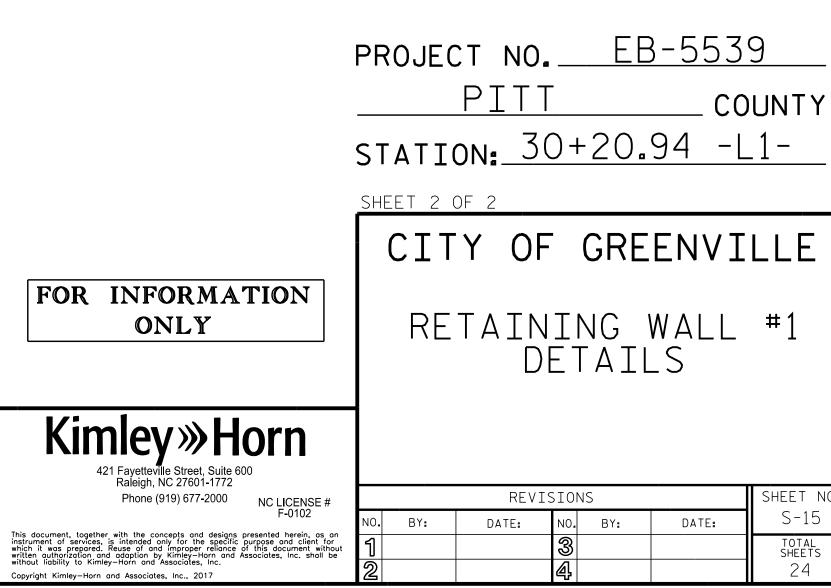
UNDERCUT WILL BE REQUIRED AT RETAINING WALL #1 AND WILL BE PAIED FOR AS "UNDERCUT EXCAVATION".



SEGMENTAL RETAINING WALL (SRW) UNITS

LEVELING PAD STEP DETAILS

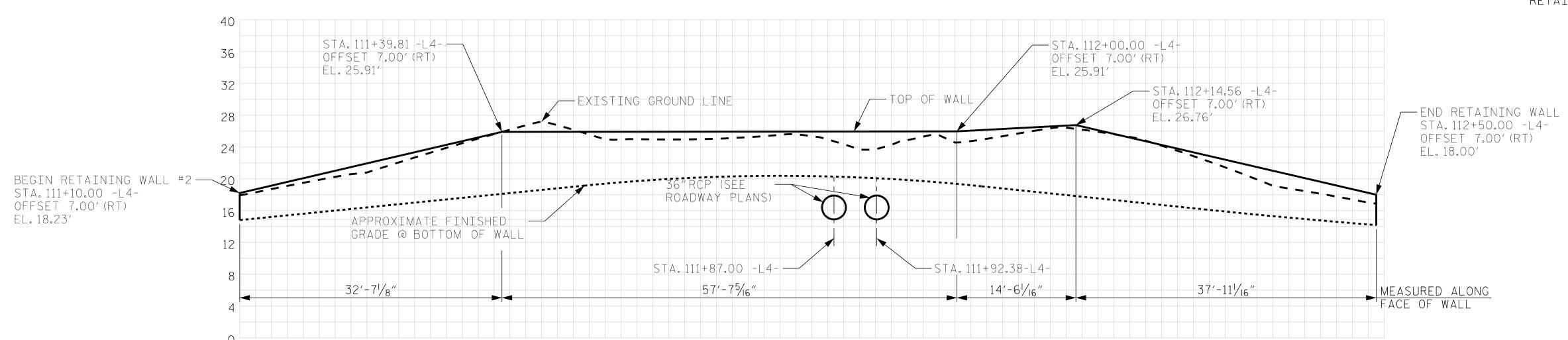
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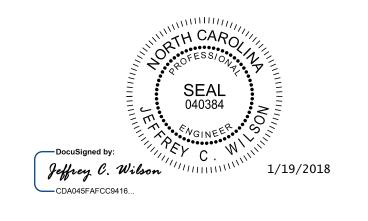


FOR PLAN VIEW OF RETAINING WALL, SEE ROADWAY PLANS. FOR RETAINING WALL TYPICAL SECTION AND NOTES, SEE "RETAINING WALL #2 DETAILS" SHEET.



RETAINING WALL #2 ENVELOPE

PROJECT NO. EB-5539 PITT ____ COUNTY STATION: 111+10.00 -L4-



CITY OF GREENVILLE

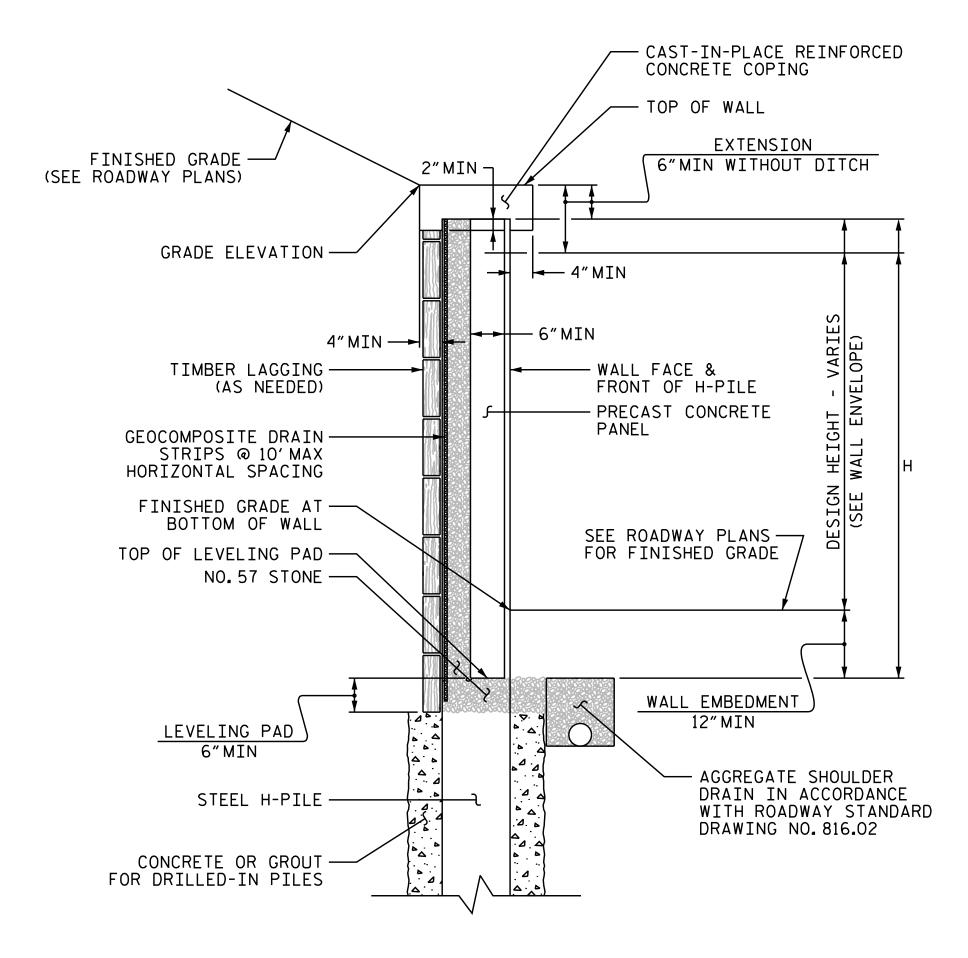
RETAINING WALL #2 ENVELOPE

Vimlay W LL

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421 Fayetteville Street, Suite 600 Raleigh, NC 27601-1772	L
Phone (919) 677-2000 NC LICENSE #	L
F-0102	Ν
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without liability to Kimley—Horn and Associates, Inc.	4

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		3			TOTAL SHEETS	
		4			24	

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CHECKED BY: <u>J.J.PICCIRILLI</u>
DESIGN ENGINEER OF RECORD: <u>J.C.WILSON</u> DATE: 12/17 DATE: 12/17



SOLDIER PILE WALL WITH PRECAST PANEL - TYPICAL SECTIONS

AT THE CONTRACTOR'S OPTION, CONNECT COPING TO PANELS WITH DOWELS OR EXTEND COPING DOWN BACK OF PANELS AND PILES.

NOTES:

FOR SOLDIER PILE RETAINING WALLS, SEE SOLDIER PILE RETAINING WALLS

DRILLED-IN H-PILES ARE REQUIRED FOR RETAINING WALL NO. 2.

BEFORE BEGINNING SOLDIER PILE WALL DESIGN FOR RETAINING WALL NO.2, SURVEY WALL LOCATION AND SUBMIT A REVISED WALL PROFILE VIEW (WALL ENVELOPE) FOR REVIEW. DO NOT START WALL DESIGN OR CONSTRUCTION UNTIL THE REVISED WALL ENVELOPE IS ACCEPTED.

DESIGN RETAINING WALL NO. 2 FOR THE FOLLOWING: 1) H = DESIGN HEIGHT + WALL EMBEDMENT 2) DESIGN LIFE = 75 YEARS

FRICTION ANGLE, ♦= 28 DEGREES

3) MINIMUM WALL EMBEDMENT DEPTH = 1 FT 4) IN-SITU ASSUMED MATERIAL PARAMETERS ABOVE ELEVATION 10 FT: UNIT WEIGHT, $\gamma = 105 \text{ LB/CF}$

COHESION, c = 0 LB/SF 5) IN-SITU ASSUMED MATERIAL PARAMETERS BELOW ELEVATION 10 FT: UNIT WEIGHT, γ = 115 LB/CF FRICTION ANGLE, ϕ = 30 DEGREES

COHESION, c = 0 LB/SF 6) IN-SITU ASSUMED MATERIAL PARAMETERS BELOW ELEVATION 3 FT: UNIT WEIGHT, γ = 125 LB/CF FRICTION ANGLE, ϕ = 34 DEGREES COHESION, c = 0 LB/SF

DESIGN RETAINING WALL NO. 2 FOR A PIPE EXTENDING UNDER OR THROUGH THE WALL AS SHOWN. VERIFY PIPE LOCATION AND ELEVATION BEFORE BEGINNING SOLDIER PILE WALL DESIGN OR CONSTRUCTION.

FACE OF RETAINING WALL SHALL BE TREATED WITH ANTI-GRAFFITI COATING AS SPECIFIED IN THE APPLICATION OF BRIDGE COATING SPECIAL PROVISION. PAYMENT WILL BE MADE UNDER THE RETAINING WALL #2 PAY ITEM AND NO SEPARATE PAYMENT WILL BE MADE.

PROJECT NO. EB-5539 COUNTY STATION: 111+10.00 -L4-



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Phone (919) 677-2000 NC LICENSE # F-0102 RETAINING WALL #2 DETAILS

CITY OF GREENVILLE

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STD CELL Wall_SoldierPile

DATE:

SHEET NO

S-17

TOTAL SHEETS

24

NOTES FOR SECTIONS B-B AND C-C SEE "PILE SUPPORTED SLAB

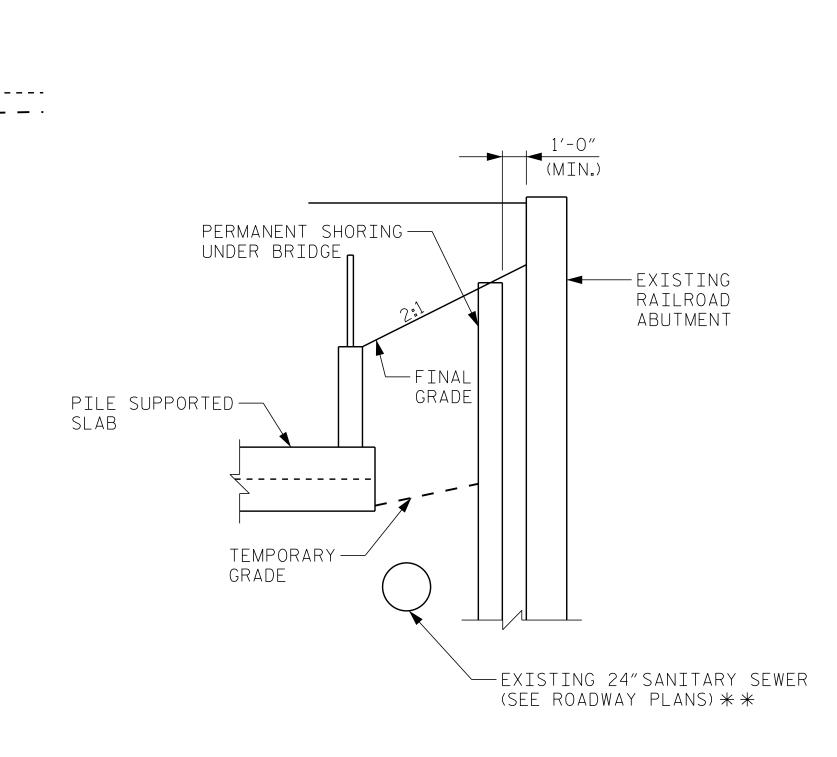
DETAILS" SHEET 1 OF 2.

FOR PILE SUPPORED SLAB NOTES, SEE "GENERAL NOTES" SHEETS.

NOTIFY THE ENGINEER IMMEDIATELY IF THE EXCAVATION FOR THE PILE SUPPORTED SLAB UNDERCUTS THE EXISTING GABION WALL ON THE SOUTHWEST SIDE OF THE RAILROAD BRIDGE.

* * FIELD VERIFY THE LOCATION OF THE EXISTING 24" SANITARY SEWER LINE BEFORE ANY PILES ARE DRIVEN. NOTIFY THE ENGINEER IMMEDIATELY IF SANITARY SEWER LINE IS FOUND TO VARY FROM PLANS.

EXISTING BRIDGE PLANS ATTACHED TO THIS PLAN SET ARE FOR INFORMATION ONLY.



SECTION A-A

VARIES

-EXIST RAILROAD BRIDGE

FENCE (SEE ROADWAY PLANS).

(BEYOND)

EXISTING CONCRETE —

WALL (NEAR SIDE)

HP12×53 STEEL PILE

DATE: 12/17

DATE: 12/17

(SEE PLAN VIEW)(TYP.)

EXISTING

PIER (BEYOND)

REMOVE EXISTING RIP-RAP —

AND SOIL DOWN TO BOTTOM OF PILE SUPPORTED SLAB.

REPLACE RIP-RAP TO THE

EDGE OF SLAB.

RAILROAD

RIP RAP AND ON

ON RIVER BANK

(FAR SIDE)

APPROXIMATE NATURAL —

GROUND LINE (FARSIDE)

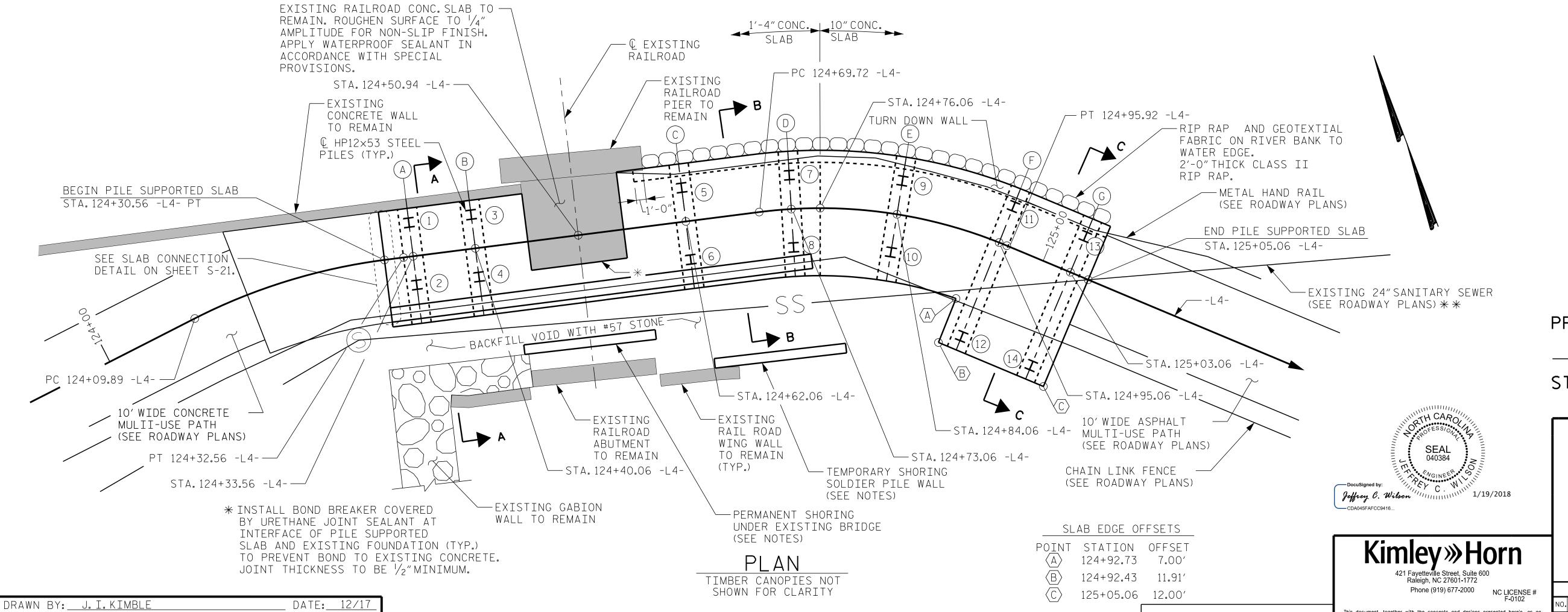
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GEOTEXTIAL FABRIC

PERMANENT SHORING UNDER BRIDGE DETAIL

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<u>⊢</u>EXISTING

— EXISTING GABION WALL

#57 STONE (FAR SIDE)

-EXISTING 24"SANITARY SEWER

BACKFILL WITH

(SEE ROADWAY PLANS) * *

TO REMAIN (NEAR SIDE)

PILE SUPPORTED SLAB

RAILROAD

ABUTMENT

(BEYOND)

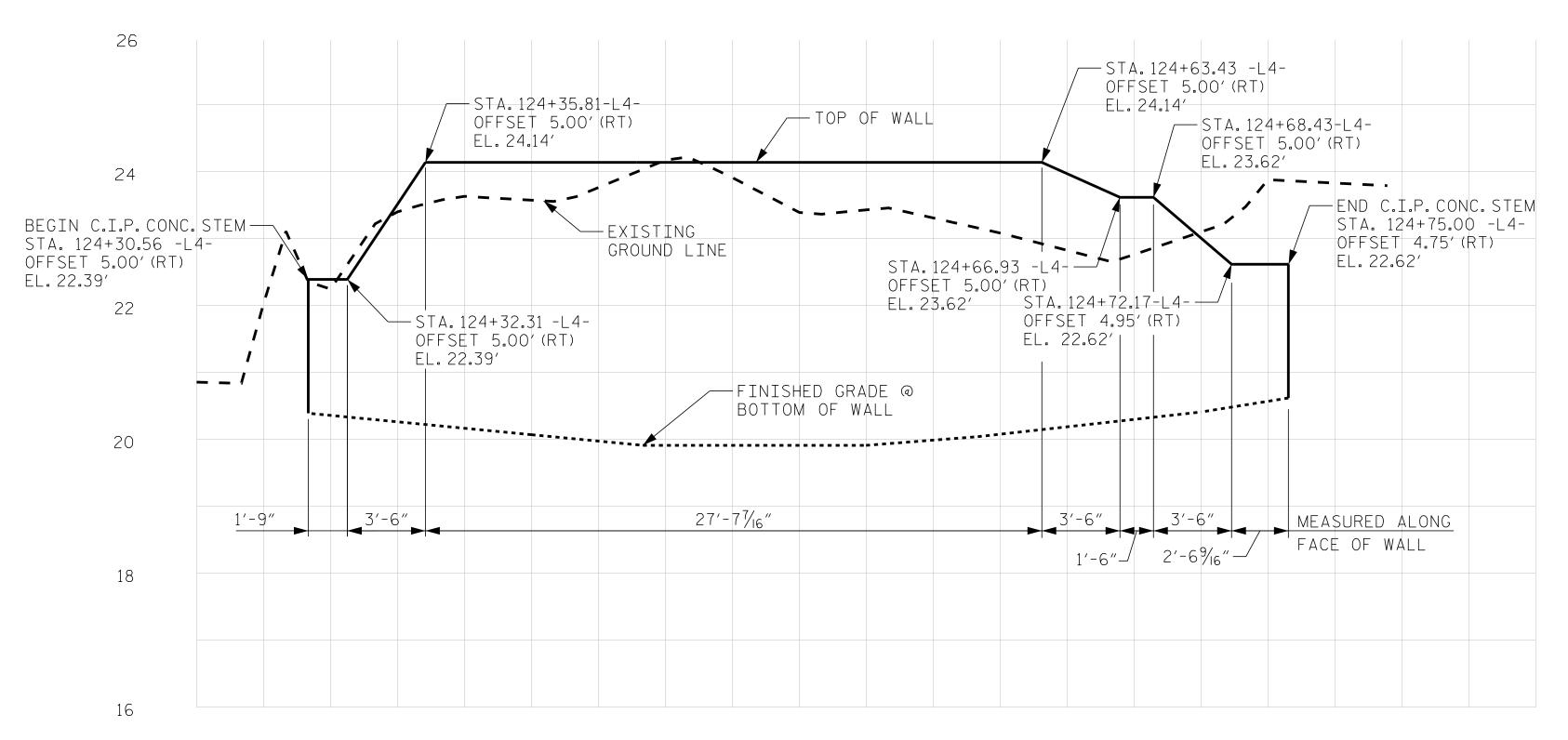
	ΤO	P OF	PILE
	EL	EVAT	IONS
	PILE	OFFSET	ELEVATION
	1	3′-6″ (LT)	18.81′
	2	3′-0″(RT)	18.85′
	3	3′-6″ (LT)	18.62′
	4	3′-0″(RT)	18.63′
	5	3′-6″(LT)	18.65′
	6	4'-0"(RT)	18.67′
	7	3′-6″(LT)	19.05′
	8	4'-0"(RT)	19.10′
	9	3′-6″(LT)	20.08′
	10	4'-0"(RT)	20.16′
	11	4'-0"(LT)	20.60′
	12	10′-6″ (RT)	20.83′
	13	4'-0"(LT)	20.99′
[14	10′-6″ (RT)	21.26′

CITY OF GREENVILLE

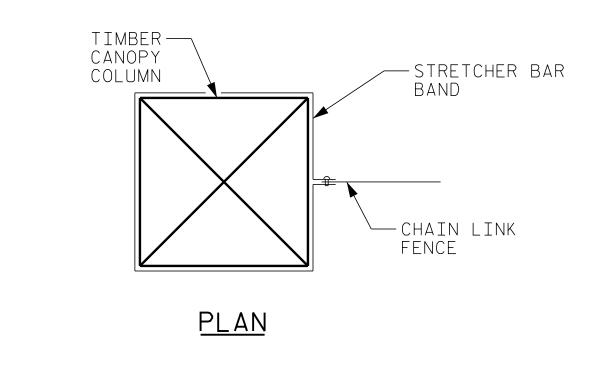
PILE SUPPORTED SLAB

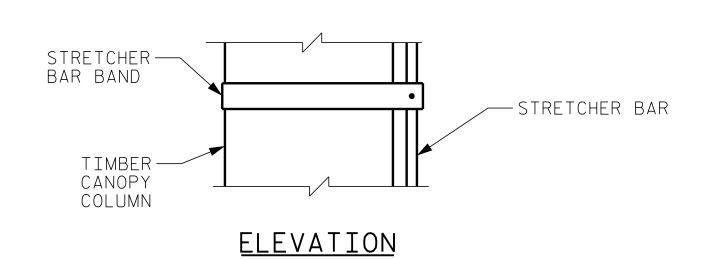
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CHAIN LINK FENCE MOUNTED ON TOP OF REMAINING WALL AND SHALL HAVE A MINIMUM HEIGHT OF 8'-0" ABOVE GREENWAY, SEE ROADWAY PLANS. CHAIN LINK FENCE NOT SHOWN FOR CLARITY.

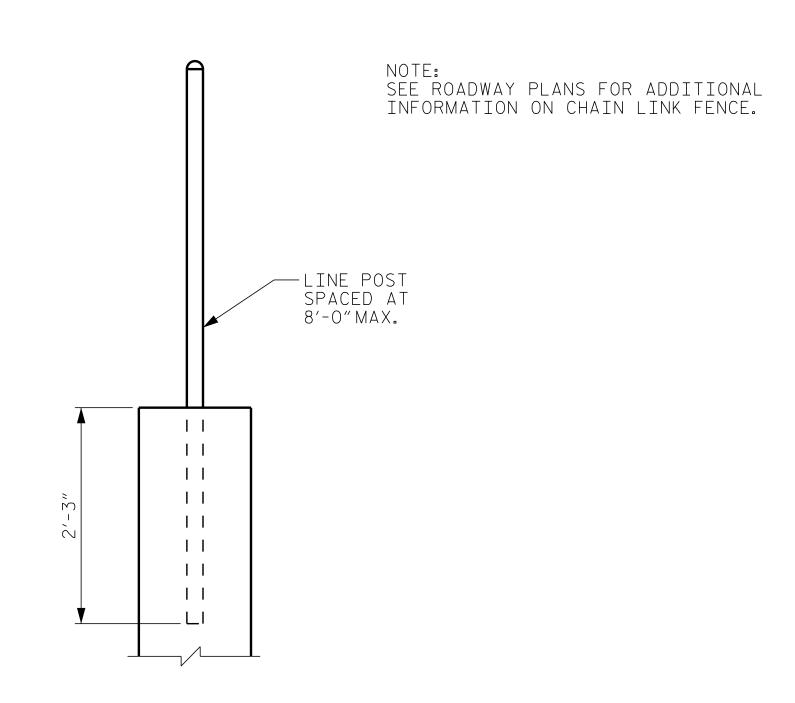


PILE SUPPORTED SLAB STEM





CHAIN LINK FENCE ATTACHMENT DETAIL



LINE POST CONNECTION DETAIL

PROJECT NO. EB-5539 COUNTY

STATION: 124+30.56 -L4-



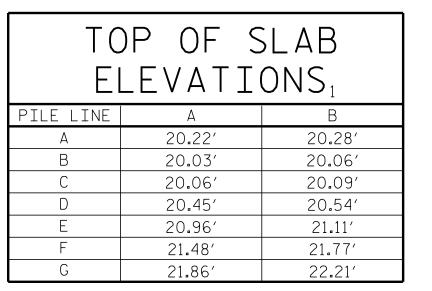
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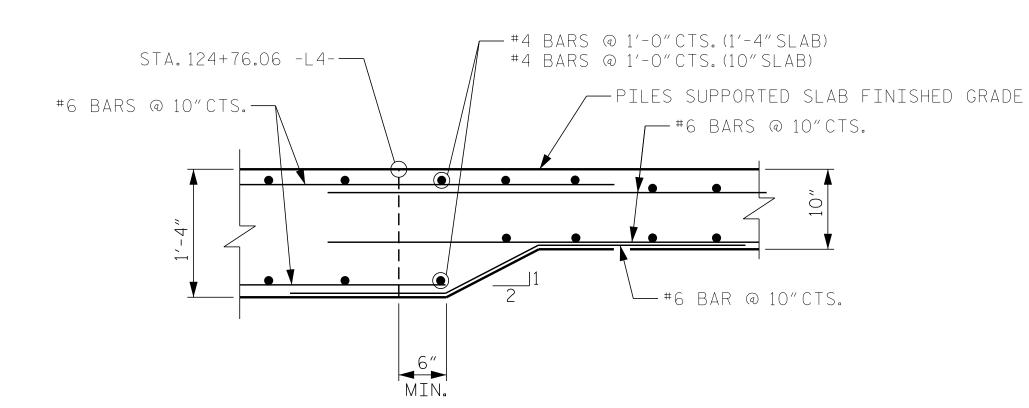
PILE SUPPORTED SLAB
STEM ENVELOPE
AND FENCE
ATTACHEMENT DETAILS

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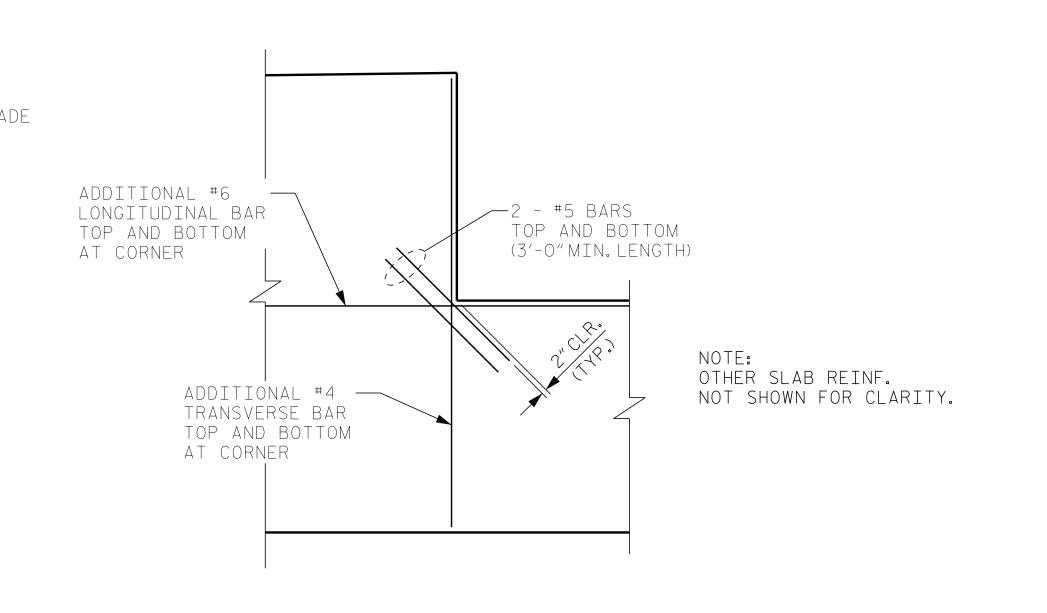
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1 CALCULATED AT EDGE OF SLAB ALONG CENTERLINE OF PILES, RADIAL TO -L4-.



SLAB TRANSITION DETAIL



GRADE-

-#4 BARS

@ 1'-0"CTS.

POINT

◄——-L4-

11'-9" OR 16'-9"

SECTION C-C

SLAB CORNER DETAIL

(SEE RDWY.PLANS)

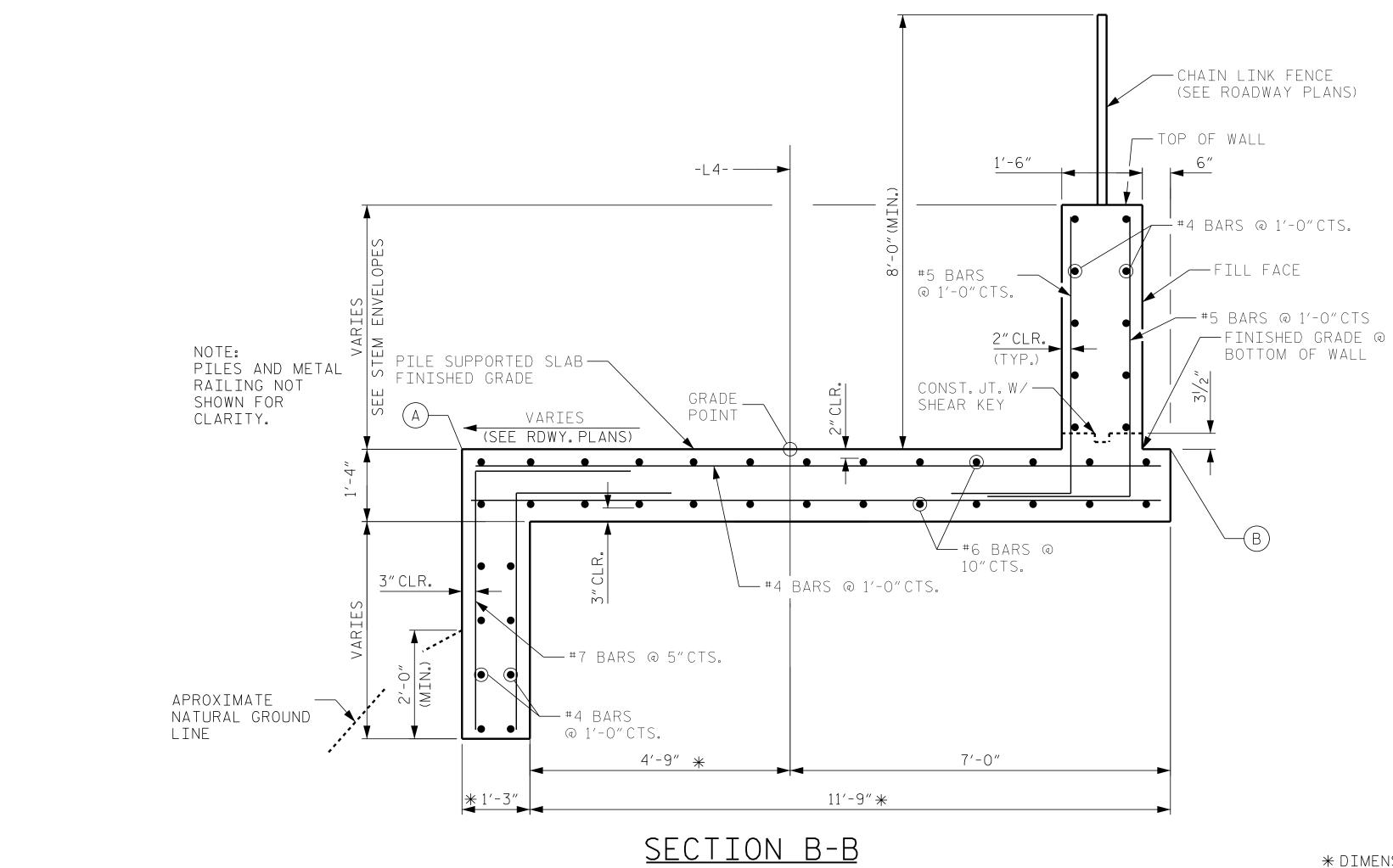
----#7 BARS

+4 BARS

@ 1'-0"CTS.

4'-9"

@ 5"CTS.



FOR PILE SUPPORTED SLAB
N EAST SIDE OF EXISTING RR

* DIMENSIONS SHOWN ARE FOR PILE SUPPORTED SLAB WITH RETAINING WALL ON EAST SIDE OF EXISTING RR BRIDGE. ON THE WEST SIDE OF THE EXISTING BRIDGE THERE IS NO TURN DOWN WALL AND THE EDGE OF SLAB OFFSET FROM -L4- IS 5'-O".

NOTE: Piles and metal

3"CLR.

2'-0" (MIN.)

RAILING NOT SHOWN FOR

CLARITY.

APROXIMATE -NATURAL GROUND

LINE

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CITY OF GREENVILLE

PILE SUPPORTED
SLAB DETAILS

STATION: 124+30.56 -L4-

EB-5539

COUNTY

---#6 BARS @ 10"CTS. .

7'-0" OR 12'-0"

PROJECT NO.___

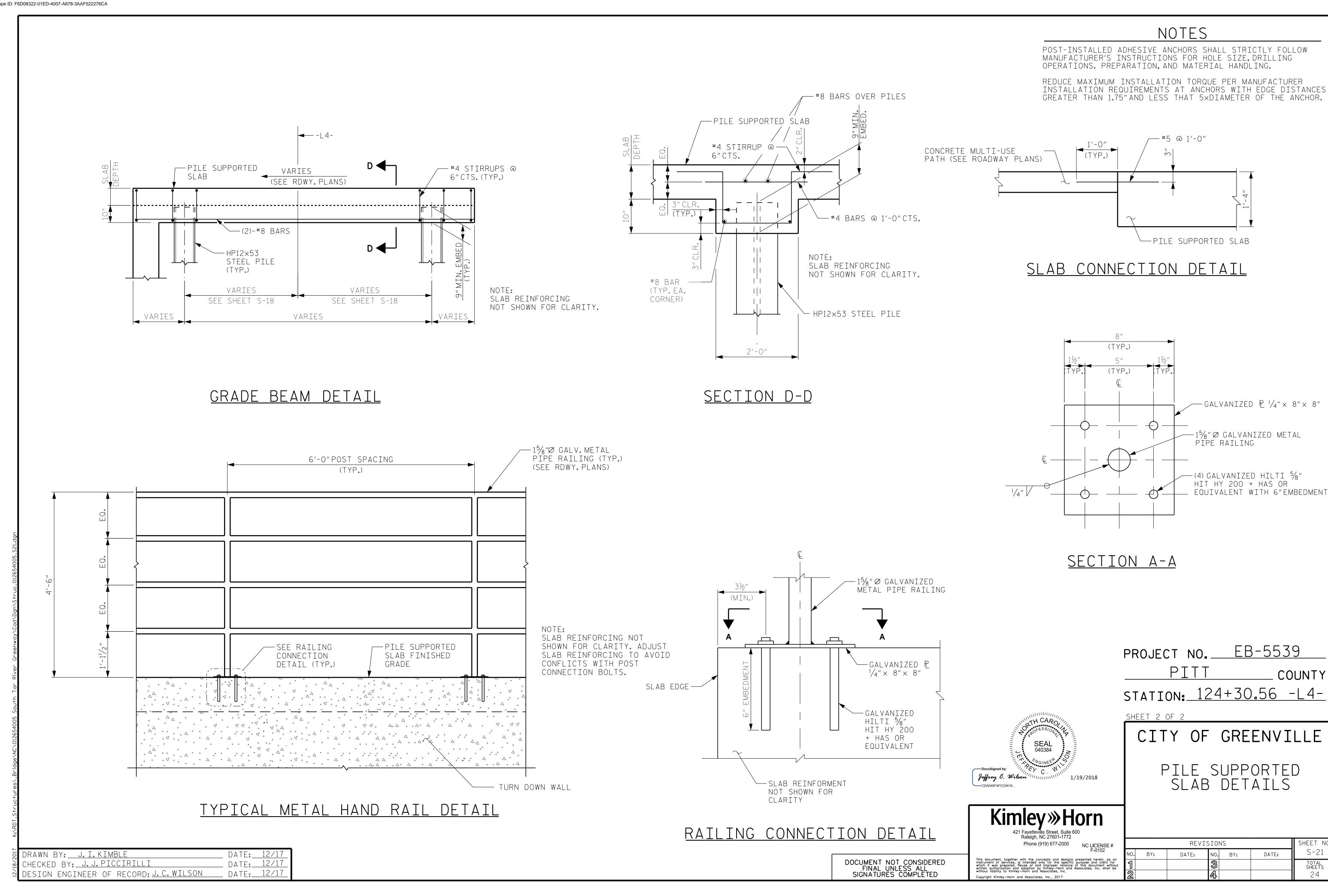
SLAB DETAIL

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DATE: 12/17
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DATE: 12/17



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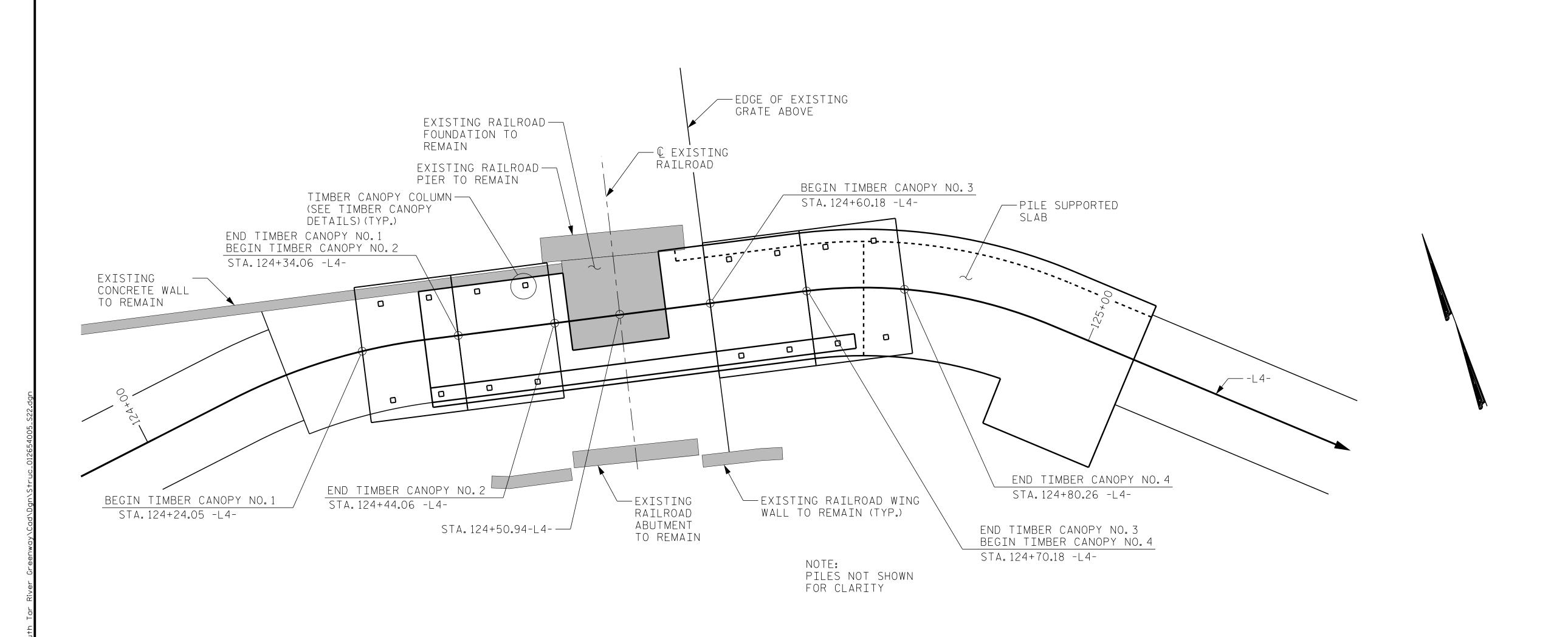
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DATE: 12/17

DATE: 12/17 DATE: 12/17

NOTES

FOR NOTES, SEE "GENERAL NOTES" SHEETS.



TIMBER CANOPY PLAN

PROJECT NO. EB-5539 COUNTY STATION: 124+30.56 -L4-

CITY OF GREENVILLE

TIMBER CANOPIES PLAN

Kimley » Horn

Jeffrey C. Wilson' CDA045FAFCC9416...

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		<u></u>			TOTAL SHEETS
		4			24

(2) LAYERS 3/4" PRESSURE TREATED —

H1 SIMPSON GALV. HURRICANE TIES —

EDGE (TYP.)

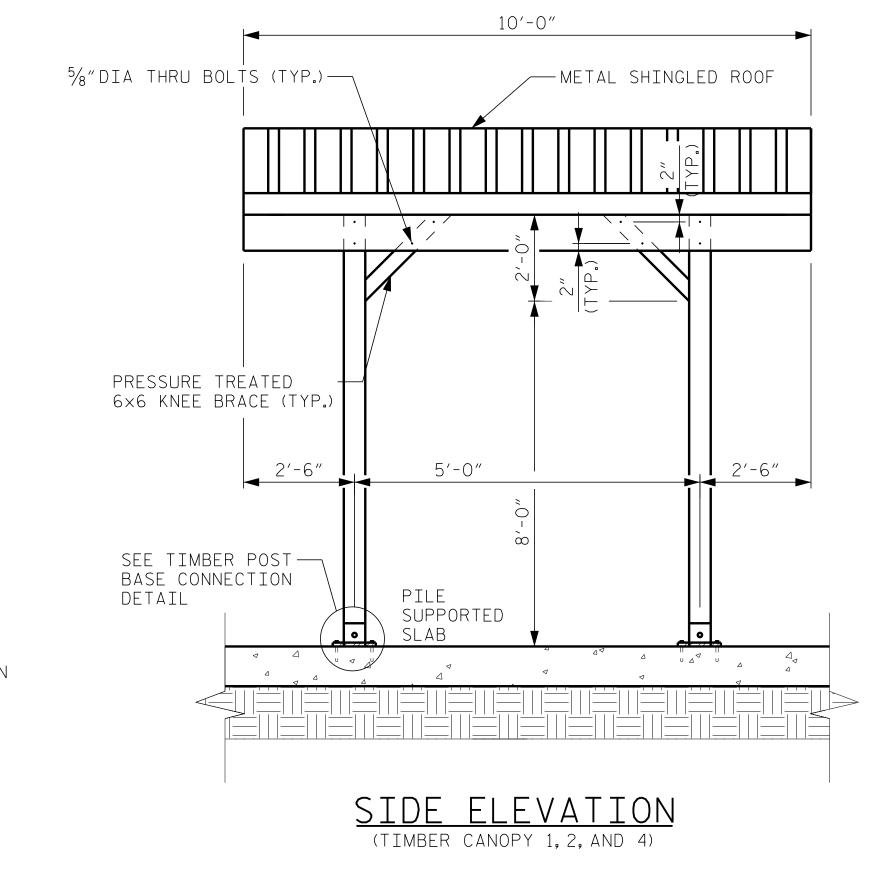
EXT. PLYWOOD (TYP.)

(OR APPROVED EQUAL) ON ALL

TRUSSES BOTH SIDES (TYP).

(TYP.)

CONNECT INTO BA TRUSS W/ (2) #6 SCREWS (TYP.)



NOTES

POST-INSTALLED ADHESIVE ANCHORS SHALL STRICTLY FOLLOW MANUFACTURER'S INSTRUCTIONS FOR HOLE SIZE, DRILLING OPERATIONS. PREPARATION, AND MATERIAL HANDLING.

REDUCE MAXIMUM INSTALLATION TORQUE PER MANUFACTURER INSTALLATION REQUIREMENTS AT ANCHORS WITH EDGE DISTANCES GREATER THAN 1.75" AND LESS THAN 5xDIAMETER OF THE ANCHOR.

TIMBER POST BASE CONNECTION ON RETAINING WALL STEM WILL BE THE SAME CONNECTION SHOWN ON THIS SHEET. THE CENTERLINE OF THE CONNECTION SHALL BE ALONG THE CENTERLINE OF THE RETAINING WALL STEM. THE TIMBER POST MOUNTED TO THE RETAINING WALL SHALL BE SHORTER THAN 10' SUCH THAT THE PREFABRICATED WOOD ROOF TRUSS IS LEVEL AND 10'-0" CLEARANCE IS MAINTAINED ALONG THE GREENWAY.

SEE SHEET 24 FOR ADDITIONAL INFORMATION ON TIMBER CANOPY

PROJECT NO. EB-5539 COUNTY STATION: 124+30.56 -L4-

CITY OF GREENVILLE

TIMBER CANOPY DETAILS

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TIMBER POST BASE CONNECTION

(CONNECTION TO SLAB SHOWN, SEE NOTES FOR CONNECTION TO WALL.)

 $-(1) \frac{1}{2}$ % Galvanized thru bolt

-GALVANIZED HILTI 5/8″HIT HY 200 + HAS OR EQUIVALENT

-SLAB REINFORCEMENT NOT

SHOWN FOR CLARITY

PRÉDRILL HOLE THROUGH

14'-0"

---6×6 TIMBER POST

TIMBER POST

<u>SECTION</u>

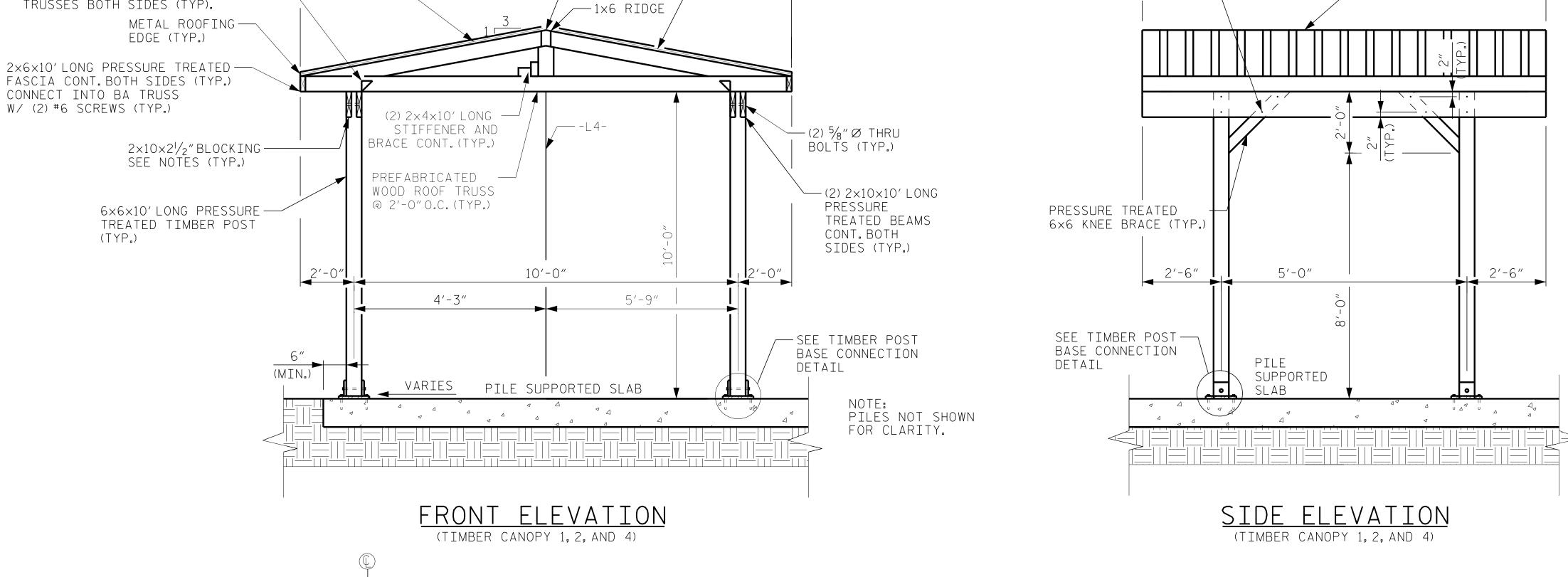
SHIM BASE PLATE TO BE LEVEL

— 4-8D NAILS

SHEET 1 OF 2

1/19/2018

REVISIONS SHEET N S-23 DATE: DATE: NO. BY: TOTAL SHEETS



(TYP.)

PLAN

2"(TYP.)

NOTES:

2"(TYP.)

TIMBER POST AND THRU BOLT

-(4) GALVANIZED HILTI 5/8"HIT HY 200 + HAS OR EQUIVALENT WITH 61/2"EMBEDMENT

NOT SHOWN FOR CLARITY.

-GALVANIZED

GALVANIZED

P 1/2"×14"×14"

 $HSS6\times6\times1/4$ " $\times6$ " LENGTH

-30"FELT WITH METAL SHINGLES

NOTES

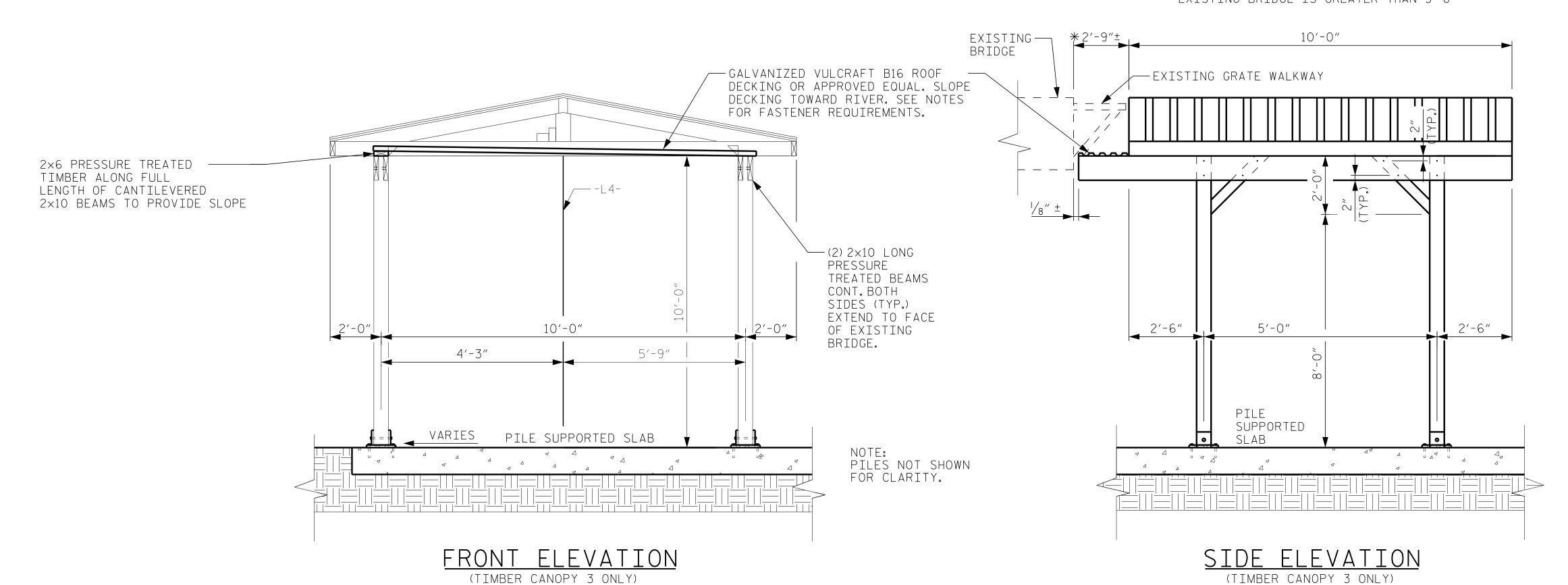
METAL DECKING TO BE FIELD CUT AROUND EXISTING BRIDGE KICKERS SUPPORTING EXISTING GRATING AS REQUIRED.

METAL DECKING SHALL BE GALVANIZED VULCRAFT B16 ROOF DECKING OR APPROVED EQUAL.

FASTENERS FOR METAL DECKING SHALL BE #10×1½"ROUND HEAD GALVANIZED WOOD SCREWS SPACED AT 6"ALONG CANTILEVERED

PROVIDE A RUBBER WASHER AT EACH FASTENER LOCATION. METAL DECKING ONLY REQUIRED FOR TIMBER CANOPY NO. 3 FOR MEMBERS AND DIMENSIONS NOT LISTED SEE SHEET 23.

* FIELD VERIFY. NOTIFY ENGINEER IMMEDIATELY IF DISTANCE FROM EDGE OF ROOF TO EDGE OF EXISTING BRIDGE IS GREATER THAN 3'-0"



PROJECT NO. EB-5539 COUNTY STATION: 124+30.56 -L4-

SHEET 2 OF 2

CITY OF GREENVILLE

TIMBER CANOPY DETAILS

S-24

TOTAL SHEETS

1/19/2018

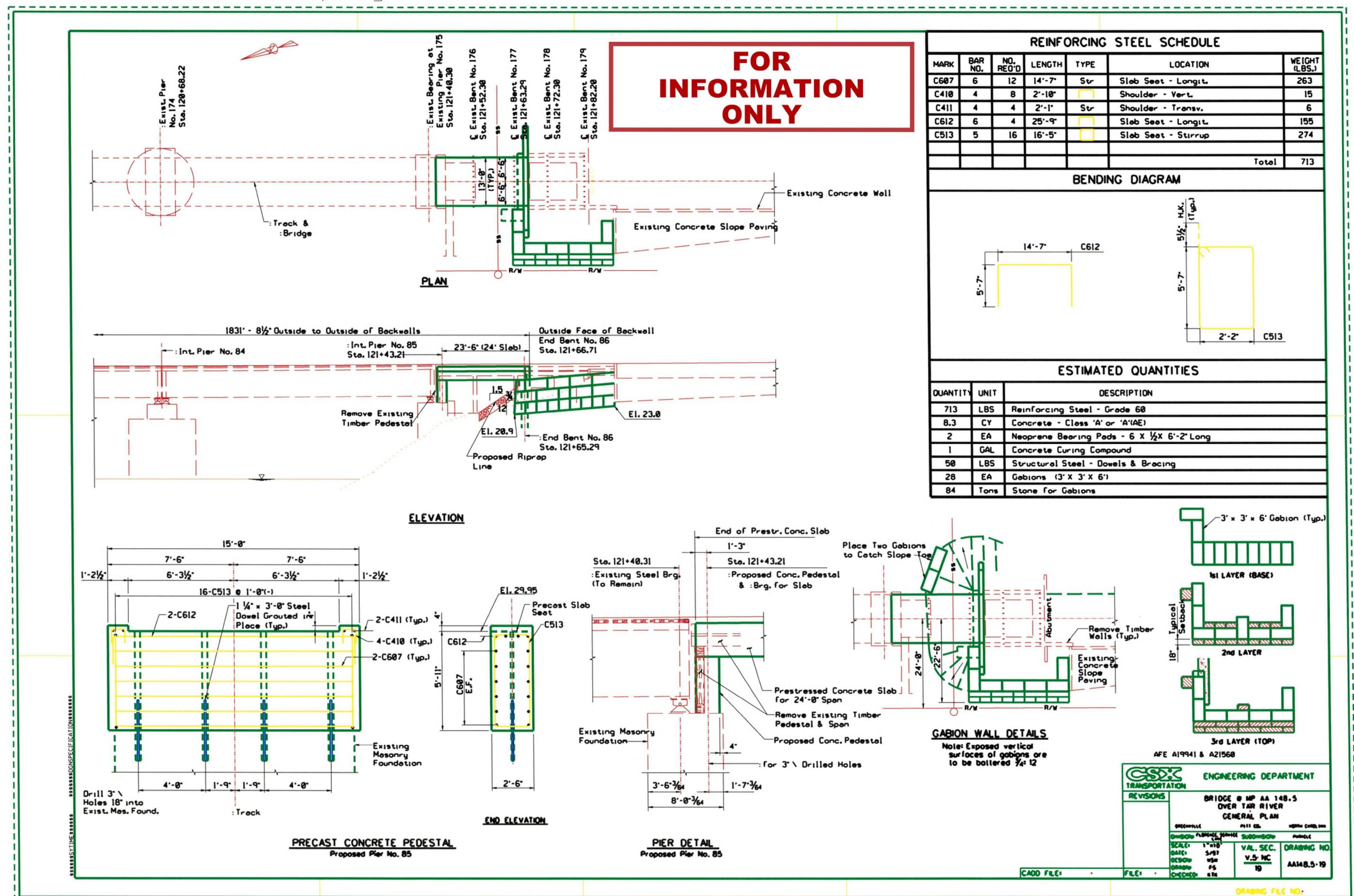
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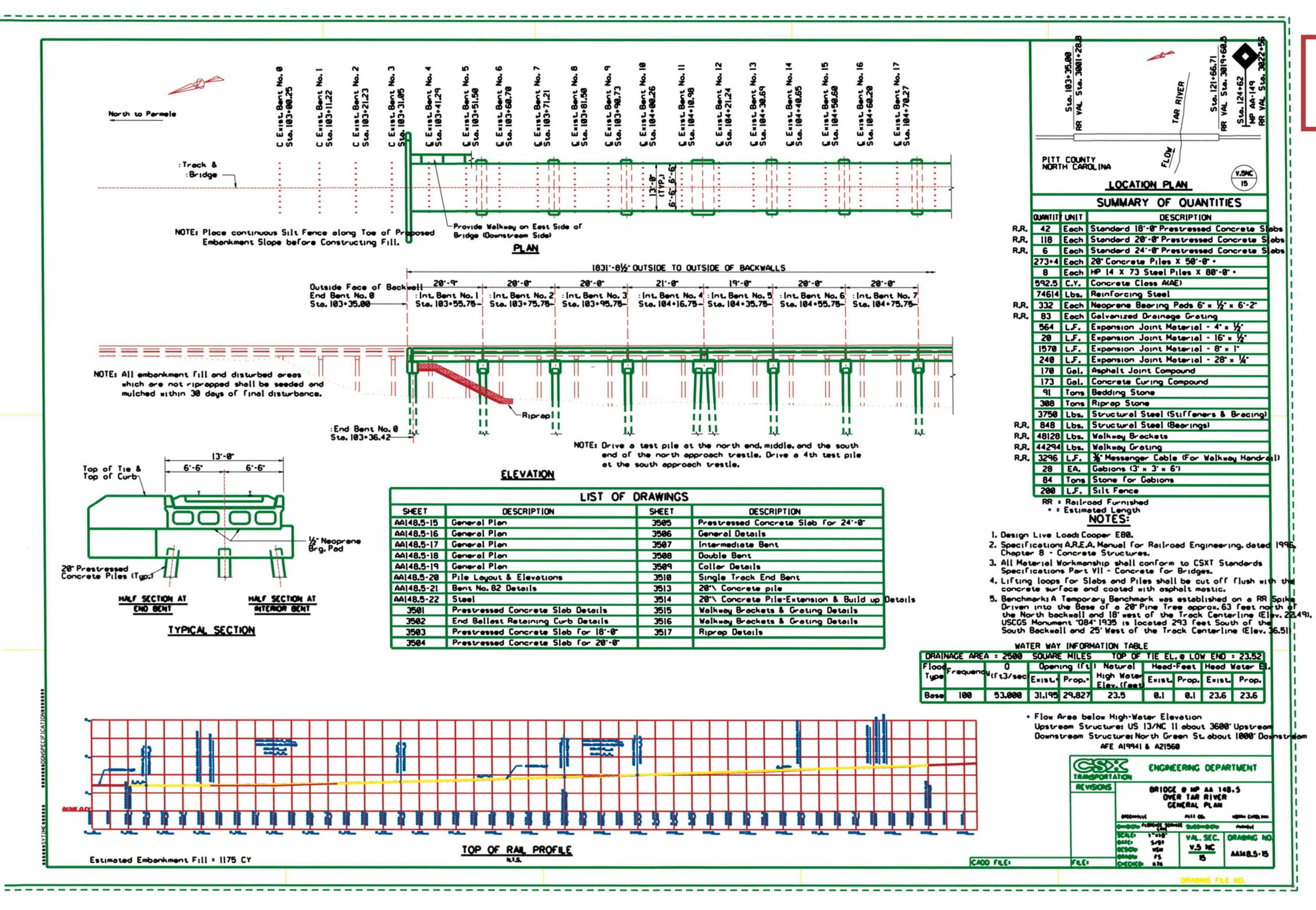
DRAWN BY: <u>J.I.KIMBL</u>E DATE: 12/17 DATE: 12/17 CHECKED BY: J. J. PICCIRILLI DATE: 12/17 DESIGN ENGINEER OF RECORD: J. C. WILSON

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

Pier Detail scaled up by 2.5



YPICAL SECTION IS SCALED UP BY 2.5



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